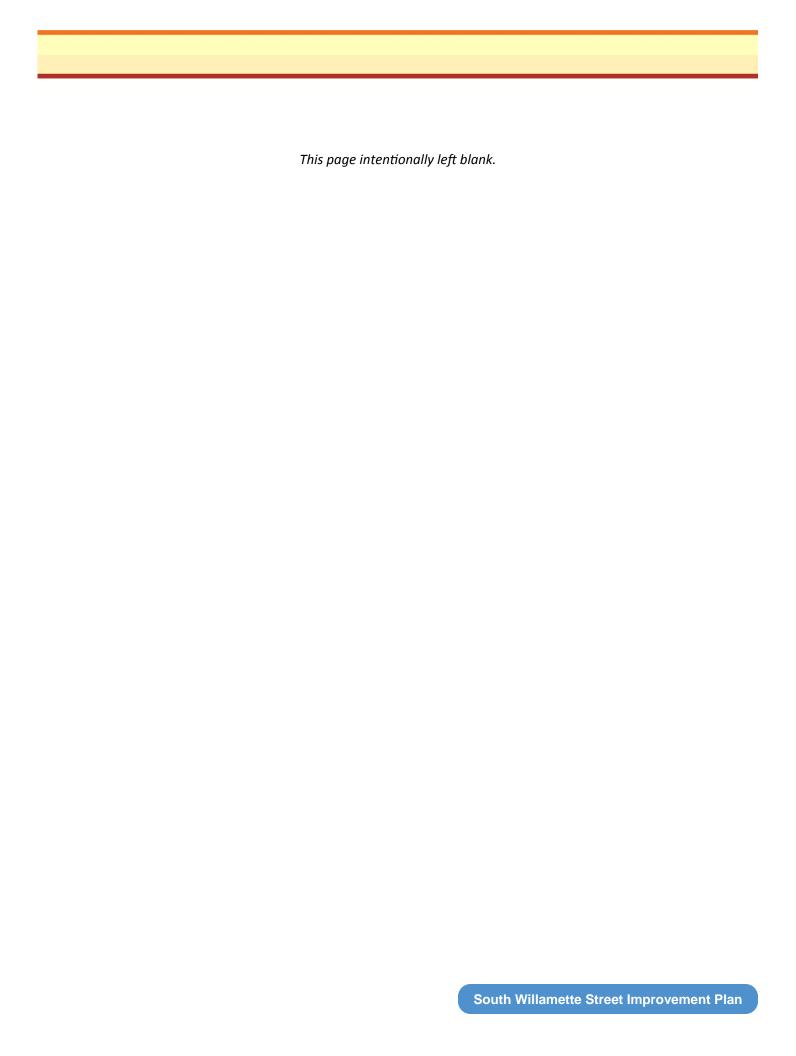
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OCTOBER 2013



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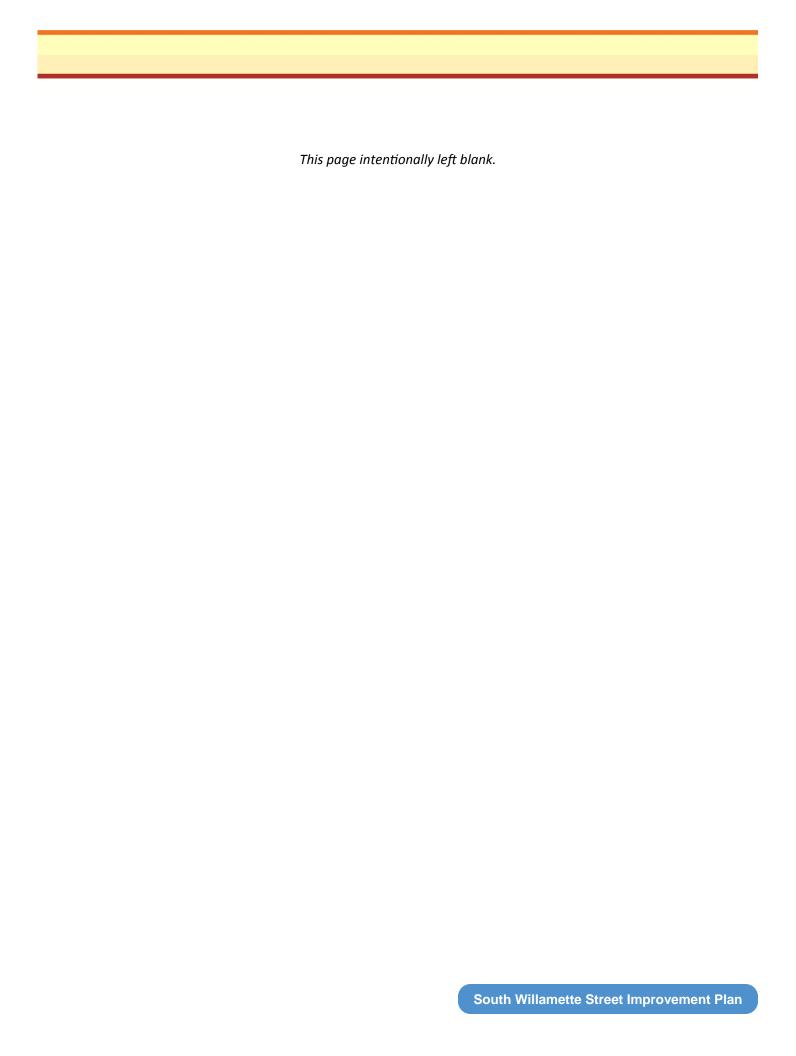
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Acknowledgements

The South Willamette Street Improvement Plan was a collaborative process among various public agencies, key stakeholders and the community. Input, assistance, and outreach by the following people helped make the Improvement Plan possible:

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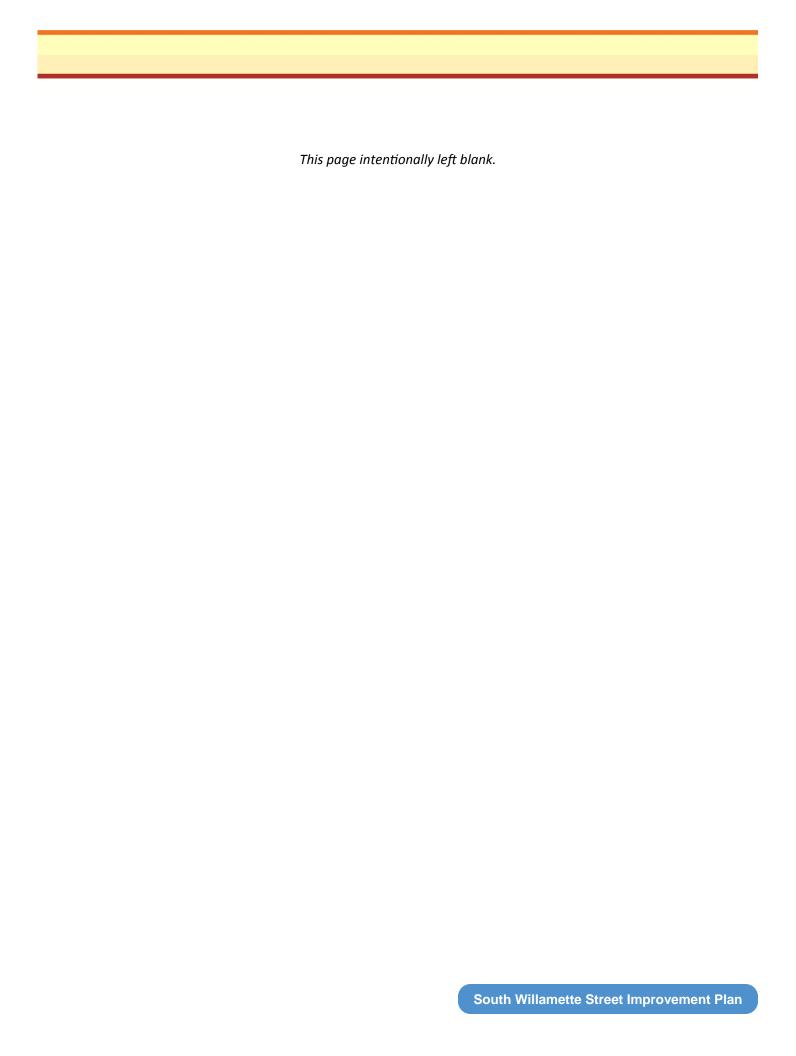
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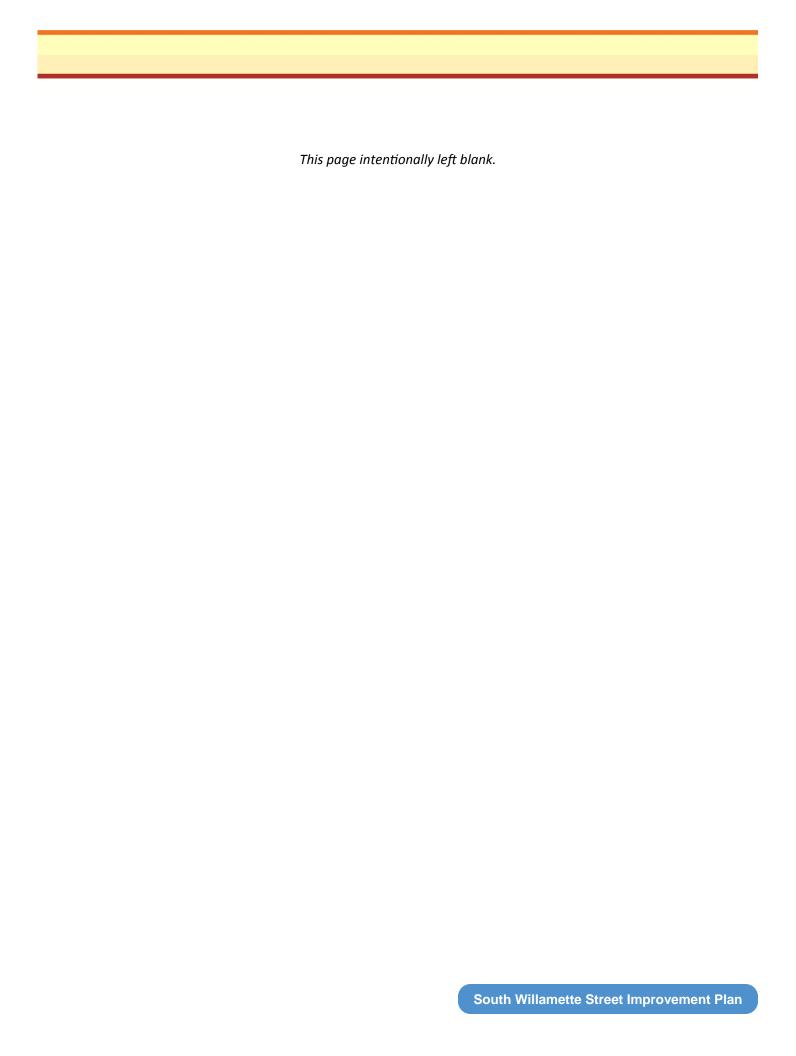
CITY OF SPRINGFIELD

Kristi Krueger

This project was partially funded by a grant from the Transportation Growth Management (TGM) Program, a joint program of the Oregon Department of Transportation and the Oregon Department of Land Conservation and Development. This TGM grant is financed, in part, by federal Safe, Accountable, Flexible, Efficient Transportation Equity Act: A Legacy for Users (SAFETEA-LU), local government, and State of Oregon funds. The contents of this document do not necessarily reflect views or policies of the State of Oregon.



SECTION A EVALUATION CRITERIA



TECHNICAL MEMORANDUM #1



117 Commercial Street NE Suite 310 Salem, OR 97301 503.391.8773 www.dksassociates.com

DATE: November 5, 2012

TO: Project Management Team

FROM: Scott Mansur, P.E., PTOE

Brad Coy, P.E. Derek Moore, E.I.T

SUBJECT: South Willamette Street Improvement Plan - Evaluation Criteria

P10086-012

This memorandum defines the evaluation criteria and scoring methodology that will be used to analyze alternatives developed for the South Willamette Street Improvement Plan. A point-based technical rating methodology will be used to rate how well proposed design alternatives meet measure of effectiveness criteria. By summing ratings (and weighting if desired), alternatives can be compared and prioritized. In this way, a consistent method will be used to evaluate and rank the alternatives based on how well they meet the identified goals and objectives. The ranking will be used to inform the Technical Advisory Committee, stakeholders and appointed and elected officials; however, the final recommended alternative will be based on feedback and direction from these parties.

Evaluation Criteria and Scoring Methodology

The City's Draft Transportation System Plan¹ (TSP) identifies numerous goals and objectives that guide future transportation projects and programs. These goals and objectives are based on a review of local and regional plans, the Sustainable Transportation Access Rating Systems (STARS) draft guidance document, and input from Eugene's Transportation Community Resource Group (TCRG). The goals provide broad statements that describe the desires of the Eugene community, and a list of 20 objectives is provided which are focused on achieving the goals. These objectives are separated into the following eight STARS goal categories:

- Access and Mobility (for all modes)
- Safety and Health
- Social Equity
- Economic Benefit
- Cost Effectiveness
- Climate and Energy
- Ecological Function
- Community Context

The goals and objectives in the Draft TSP provided a basis for the development of the evaluation criteria, which are intended to assess a project's potential to meet the transportation needs of the City. The evaluation criteria were then refined based on a review of planning documents more specific to the study

¹ Eugene Transportation System Plan: Existing Conditions and Deficiencies, March 2011



area, such as the South Willamette Concept Plan. The criteria are summarized in Table 1 according to the goal category they support.

Table 1: South Willamette Street Evaluation Criteria and Scoring

Criteria	Evaluation Score
Access and Mobility	
Reliability	+1. Improves trip reliability
Improves trip reliability, consistency, comfort and convenience for all modes (walk, bike, transit, cars).	0. No change
	-1. Reduces trip reliability
Neighborhood Connectivity	+1. Increases # of connected households
Increases the number of households that can safely walk, bike, or use transit services to meet basic (non-work) daily needs.	0. No change
	-1. Decreases # of connected household
Motor Vehicle Travel Time Reduces travel time between key origins and destinations for	+1. Decreases travel time for motor vehicles
motor vehicles.	0. No change
	-1. Increases travel time for motor vehicles
Alternative Mode Travel Time Reduces travel time between key origins and destinations for	+1. Decreases travel time for alternative modes
alternative modes.	0. No change
	-1. Increases travel time for alternative modes
Safety and Health	
Safety	+1. Improves safety for all modes
Improve safety and security for all users, especially for the most vulnerable; strive for	0. No change
zero fatalities.	-1. Reduces safety for all modes
<u>Security</u>	+1. Improves sense of security
Improve actual and perceived sense of security (i.e. Safe driving, getting to and riding transit, walking and biking).	0. No change
	-1. Decreases sense of security
Emergency Access	+1. Improves emergency response times
Improves or maintains emergency response times within and through the corridor.	0. No change
- -	-1. Reduces emergency response times



(Continued) Table 1: South Willamette Street Evaluation Criteria and Scoring

Criteria	Evaluation Score
Social Equity	
Equity Contributes to closing the transportation access gap between the general user and populations with limited choices, such as the elderly, low income, minority populations, and people with disabilities.	 +1. Specifically benefits populations with limited choices 0. No Change -1. Negatively impacts populations with limited choices
Economic Access Improves access from residences to employment and neighborhood centers within a 20-minute walk, bike, or transit trip.	+1. Improves employment access 0. No change -1. Decreases employment access
Economic Benefit	
Freight Mobility Provides safe, efficient, and continuous motor vehicle operation to allow timely freight movement along Willamette Street.	+1. Improves corridor's freight movement 0. No Change -1. Negative impact on freight movement
Walkable/Bikeable Business District Promotes a "Safe, Attractive Pedestrian Experience for Business, Shopping and Entertainment."	 +1. Improves business district pedestrian and bicycle experience 0. No change -1. Reduces business district pedestrian and bicycle experience
Business Vitality Supports access and visibility of businesses that rely on drive-by traffic by balancing congestion with economic vitality	+1. Supports economic vitality0. No change-1. Negative impact on economic vitality



(Continued) Table 1: South Willamette Street Evaluation Criteria and Scoring

Criteria	Evaluation Score
Cost Effectiveness	
<u>Fundability</u> Available funding sources exist to implement projects in a timely fashion.	+1. Funding sources are available0. Feasible costs, but no identified funding-1. High costs and no funding expected
Asset Management Favors the enhancement and maintenance of existing systems over system expansion.	 +1. Enhances existing transportation system 0. Minimal enhancement or expansion -1. Expands transportation system
Project Benefits Optimizes benefits relative to public, private and social costs over the life-cycle of the project	+1. Provides maximum benefits 0. Minimal benefits -1. Provides no benefits
Climate and Energy	
Reduce Vehicle Miles Traveled (VMT) Improves the corridor as an attractive area without having to drive. Increases mode share for walk, bike, and transit thus reducing greenhouse gases and fossil fuel consumption.	+1. Reduces VMT 0. No change -1. Increases VMT
Pedestrian Facilities Adds sidewalks and crosswalks that fill in system gaps, improve system connectivity, removes obstructions and are accessible to all users.	+1. Improves pedestrian facilities 0. No change -1. Negative impact on pedestrian facilities
Bicycle Facilities Adds bikeways that fill in system gaps, improve system connectivity, and are accessible to all users.	 +1. Improves bicycle facilities, including bike lanes 0. No change -1. Negative impact on bicycle facilities
<u>Transit Facilities</u> Improves transit facilities and accessibility to transit stops (for all users) along and near the corridor.	+1. Improves transit facilities 0. No change -1. Negative impact on transit facilities

Table Continued on next page.



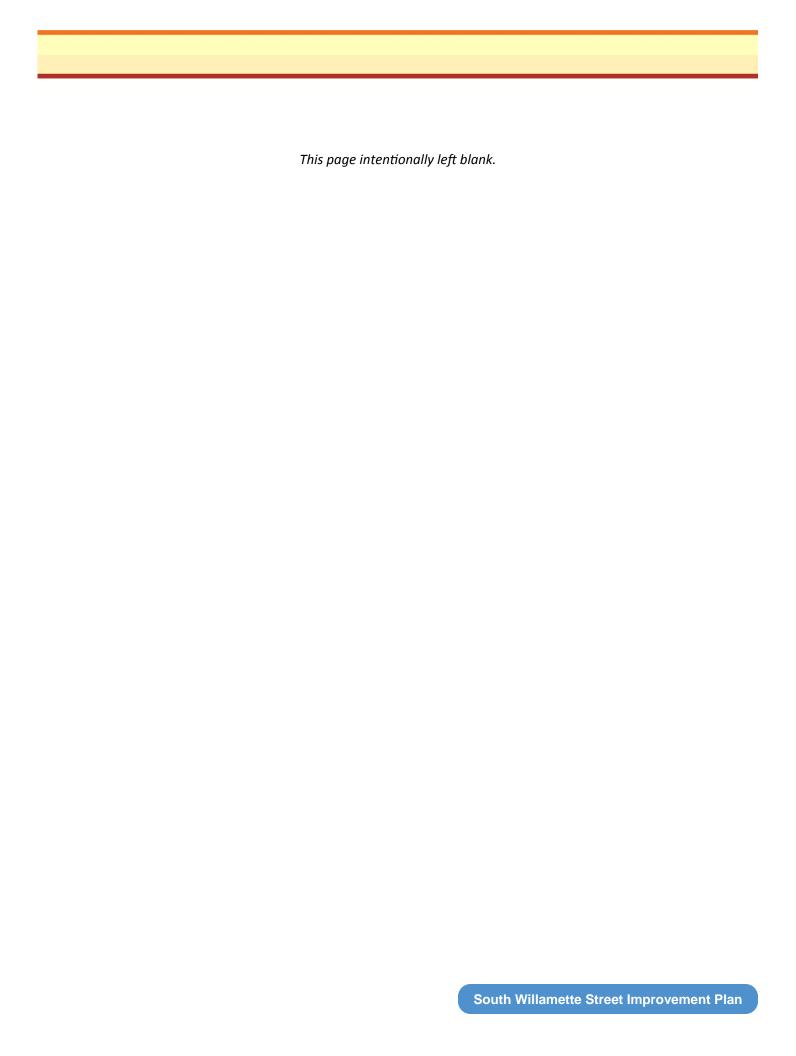
(Continued) Table 1: South Willamette Street Evaluation Criteria and Scoring

Criteria	Evaluation Score
Ecological Function	
Stormwater Design	+1. Minimizes storm water runoff
Transportation improvements lower the rate of storm water runoff and improve water quality.	0. No change
	-1. Increases storm water runoff
Landscape Design	+1. Reduces heat island
Reduces the urban heat island through landscape design, less pavement, and increased tree canopy.	0. No change
	-1. Increases heat island
Community Context	
Community Vision and Land Use	+1. Supports Envision Eugene
Supports implementation of Envision Eugene land use and growth management goals and A Community Climate and	0. No change
Energy Action Plan for Eugene.	-1. Conflicts with Envision Eugene
Transportation Planning Compatibility Compatible with City's transportation plans (TSP, Long Range	+1. Compatible with City transportation plans
Transit Plan, and Pedestrian and Bicycle Master Plan [PBMP])	O. Has little or no impact (or has offset impacts)
	-1. Not compatible with City transportation plans

The scoring methodology can be applied in one of the following three ways:

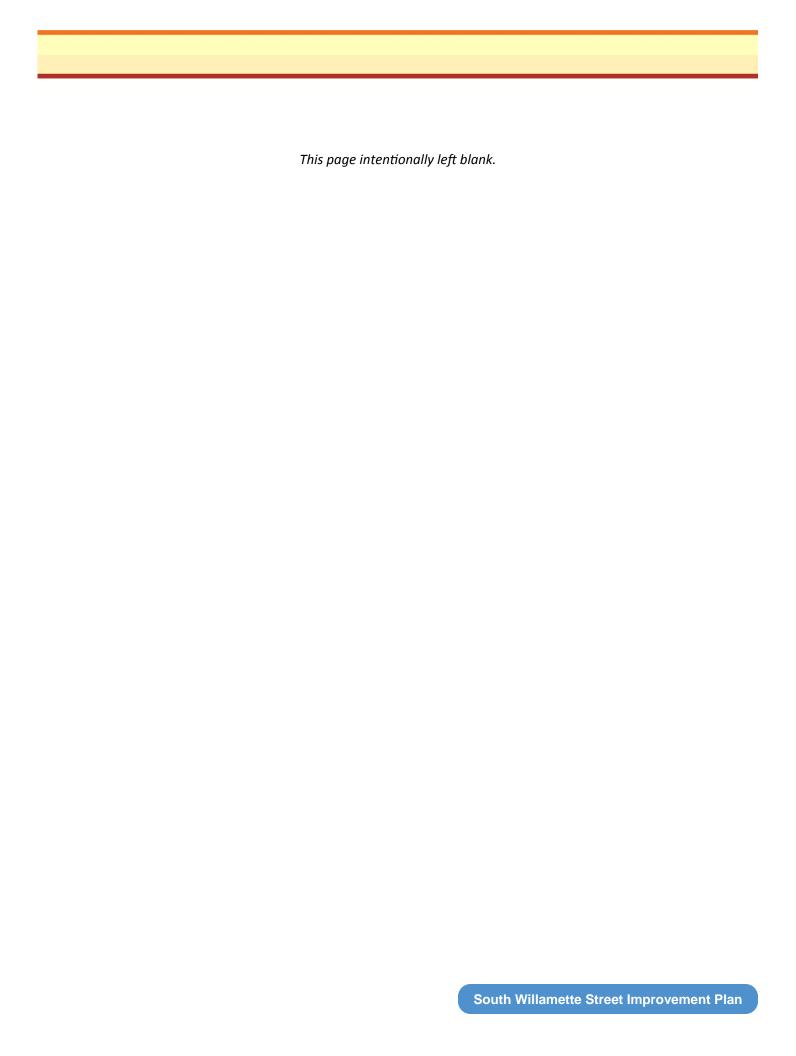
- 1. **Equal weight for each criteria** The evaluation scores for all criteria are summed to determine the overall evaluation score. This method allows a goal category with more supporting criteria to have a larger influence on the overall score.
- 2. **Equal weight for each goal category** Each of the eight categories receives an equal weight. In this method, evaluation scores for each criterion under a particular goal category would be averaged to determine one score for each goal category. They would then be summed to arrive at an overall evaluation score.
- Stakeholder feedback to determine weight
 — Feedback from stakeholders would be solicited to
 help determine the weight of each goal category. Criteria scores for a particular category would be
 averaged and the weight would then be applied.

Typically, scoring methods involve either 1 or 2 or a combination of both for verification. However, if stakeholder input is provided to allow for weighting of project goals, then method 3 could also be applied.



SECTION B

EXISTING CONDITIONS, FUTURE CONDITIONS, AND PLANNED IMPROVEMENTS







117 Commercial Street NE Suite 310 Salem, OR 97301 503.391.8773 www.dksassociates.com

DATE: February 12, 2013

TO: Project Management Team

FROM: Scott Mansur, P.E., PTOE

Brad Coy, P.E. Derek Moore, E.I.T

SUBJECT: Task 2.1-Existing Conditions, Forecast Conditions, and Planned Improvements

P10086-012

This memorandum summarizes the existing transportation conditions for the South Willamette Street Improvement Plan in Eugene, Oregon. Information contained in this document will be used to inform the development and analysis of alternatives as part of a project focused on revitalizing South Willamette Street.

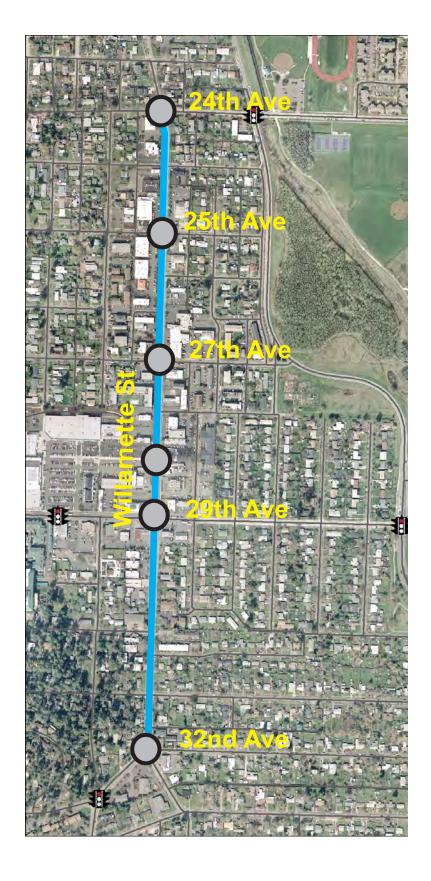
The purpose the South Willamette Street Improvement Plan is to explore options for people to easily and safely walk, bike, take the bus, or drive in an eight-block study area from 24th Avenue to 32nd Avenue. The goal of this study is to help South Willamette Street become a vibrant urban corridor accessible by bicycle, foot, car, and bus. This memorandum identifies the study corridor, provides a summary of the existing transportation facilities, and summarizes the existing travel conditions for all users. In addition, it documents improvements planned for the study corridor and will also include the forecasted traffic conditions when they become available.

Study Corridor

The study corridor is a 0.8 mile segment of Willamette Street between 24th Avenue and 32nd Avenue. This section of Willamette Street is a minor arterial that carries approximately 16,500 vehicles per day¹ and has five signalized and several unsignalized intersections. All five signalized intersections and one unsignalized intersection (as listed below) were analyzed in conjunction with this memorandum. These intersections are listed below and shown in Figure 1.

- Willamette Street/24th Avenue
- Willamette Street/25th Avenue
- Willamette Street/27th Avenue
- Willamette Street/Willamette Plaza Driveway (unsignalized)
- Willamette Street/29th Avenue
- Willamette Street/32nd Avenue

¹ Tube counts collected on 7/22/2010 south of the Willamette Street/27th Avenue intersection.







- Study Corridor



- Study Intersection





Figure





Existing Facilities

This section of the memorandum documents the existing roadway conditions, including key characteristics about the roadway network, bicycle and pedestrian facilities, transit facilities, and adjacent land uses.

Roadway Network

The transportation characteristics of the study corridor and key intersecting roadways are shown in Table 1 and include functional classification, approximate street width, number and direction of travel lanes, posted speeds, and the presence of sidewalks and/or bike lanes. The functional classification is a key characteristic because it specifies the purpose of the roadway and is a determining factor for applicable cross-section, access spacing, and intersection performance standards. At the north end of the study corridor, 24th Avenue provides an important connection to the east and attracts a high number of vehicles traveling north along Willamette Street. To the south, 29th Avenue is a minor arterial that carries approximately 12,000 to 15,700² vehicles per day, and the remaining cross streets primarily provide local access to businesses and residential areas.

Table 1: Roadway Characteristics

Roadway	Functional Classification	Street Width	Travel Lanes	Posted Speed	Sidewalk	Bike Lanes
Willamette Street (North of 29 th Avenue)	Minor Arterial	42 feet	4	25	Yes	No
Willamette Street (South of 29 th Avenue)	Minor Arterial	41 feet	41 feet 3 (2 Southbound, 1 Northbound)		Yes	Yes
24 th Avenue (East of Willamette Street)	Minor Arterial	36 feet	2	30	Yes	Yes
24 th Avenue (West of Willamette Street)	Local	32 feet	2	25	Yes	No
25 th Avenue	Local	33 feet	2	25	Yes	No
27 th Avenue	Major Collector	32-38 feet	2	25	Yes	No
29 th Avenue	Minor Arterial	40 feet	3 (TWLTL) ^a	30	Yes	Yes
Donald Street	Major Collector	34 feet	2	25	Yes	No

^a TWLTL= Two-Way Left Turn Lane

As shown in Table 1, there are three primary cross sections for Willamette Street. From 24th Avenue to 29th Avenue, Willamette Street consists of 4 lanes, 2 lanes in each direction, and no bike lanes (shown in Figure 2a). As it approaches 29th Avenue, the roadway widens to 5 lanes to accommodate left-turn lanes on both the northbound and southbound approaches (shown in Figure 2b). A southbound bike lane originates south of 29th Avenue and continues south through the study area. Roughly 500 feet south of 29th Avenue, the cross section

² City of Eugene 2007 Traffic Flow Map, downloaded from City website (www.eugene-or.gov)



transitions to include bike lanes in both directions, one northbound travel lane, and two southbound travel lanes (shown in Figure 2c).

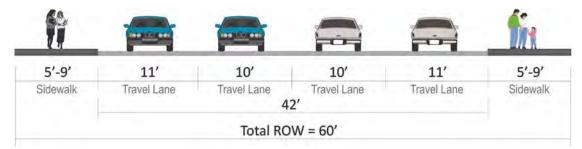


Figure 2a: 4-Lane Cross Section

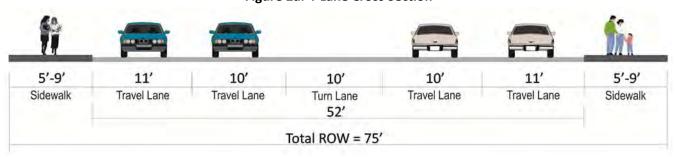


Figure 2b: 5-Lane Cross Section

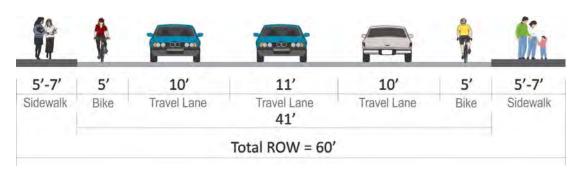


Figure 2c: 3-Lane Cross Section

Bicycle and Pedestrian Facilities

Sidewalks are present on both sides of Willamette Street for the full length of the study corridor and they vary in width ranging from 5 feet to 9 feet. Most of the study area has curbside sidewalks with the exception of small sections of landscaping near the north and south limits of the study area. Utility poles and other objects create obstacles and impact accessibility. There are marked pedestrian crossings at the five signalized intersections. No other marked crosswalks currently exist within the study area.

Bike lanes exist from approximately 250' south of 29^{th} Avenue and continue south through 32^{nd} Avenue. There are currently no bicycle facilities to the north of 29^{th} Avenue. Bike lanes are present on the cross streets of 24^{th}



Avenue and 29th Avenue; however the lack of bike lanes on Willamette Street hinders connectivity to these facilities. Portland Street (one block to the west) and Oak Street (one block to the east) provide potential alternate bike routes to Willamette Street but these roadways include connectivity gaps in the network.

Figure 3 below shows the location of existing bike lanes, sidewalks, and paths.



Figure 3: Existing Bicycle (Left) and Pedestrian (Right) Facility Inventory.

Driveways and Access Points

There are over 70 driveways on the 0.8 mile corridor of Willamette Street. The Arterial and Collector Street Plan (ACSP) indicates that for a typical minor arterial, emphasis should be given to mobility rather than accessibility and that access regulation is of high priority for roadways with this classification. However, the commercial nature of Willamette Street will require a balanced approach to maintaining access and supporting mobility.



Transit Facilities

Lane Transit District (LTD) provides public transit service to the Eugene-Springfield areas. The following two routes provide service to the study area.

- Route 24 (Donald) Route 24 runs both directions over the length of the study corridor. On weekdays, it operates from roughly 6:15 am to 11:00 pm with 30-minute headways. After 7:00 pm, it operates with one-hour headways. On Saturdays, this route operates very similar to weekdays, and on Sundays it operates on one-hour headways from 8:00 am to 8:00 pm.
- Route 73 (UO/Willamette) Route 73 runs both directions on Willamette Street from 29th Avenue to 40th Avenue. At 29th Street, the route head east to Hilyard Street. On weekdays, this route operates from about 7:00 am to 7:00 pm with headways ranging from 20 minutes to two hours, and there is no service on weekends.

Figure 4 shows the locations of marked bus stops located within the study area as well as the available transit routes through the study corridor.

Adjacent Land Uses

Figure 5 on the following page summarizes the land uses adjacent to the study corridor. From 24th Avenue to 29th Avenue, the adjacent land use is a combination of a few single family homes, apartment buildings, and retail stores. Woodfield Station is located between 28th Avenue and 29th Avenue on the west side of Willamette Street. Adjacent land use south of 29th Avenue consists mostly of apartment buildings and single family residential units.



Figure 4: Transit Stops and Routes



Travel Conditions

This section summarizes existing travel conditions, including traffic volume, speed, and classification along the Willamette Street corridor; turning movement volumes and operations for the six study intersections; multi-modal level of service for Willamette Street segments; and collision analysis results.

Volumes, Speed, and Classification

Table 2 presents data collected from 24-hour tube counts³ taken south of the Willamette Street/27th Avenue intersection. The data presented includes vehicular bi-directional volumes, 85th percentile speeds, and heavy vehicle percentages from intersection manual turn counts⁴. As shown, the daily traffic volume is approximately 16,500 along the study corridor and the daily directional split is fairly even. The



Figure 5: Adjacent Land Use

85th percentile speeds (meaning 85% of vehicles travel at this speed or slower) along Willamette Street are approximately 5 mph higher than the posted speed of 25 mph and the heavy vehicle percentages are around 2%.

Table 2: Willamette Street ADT, Speed, and Classification

	Northbound	Southbound	Total
Average Daily Traffic	7,610 (47%)	8,750 (53%)	16,360
85 th Percentile Speed	31.7 mph	29.8 mph	30.7 mph
Heavy Vehicle Percentage	2%	2%	2%

To further understand the use of this roadway over the course of a 24-hour period, Figure 6 shows vehicle movements throughout the day. This graph shows that the highest northbound traffic volume occurs during the

³ 24-hour bi-directional volume count taken on July 20, 2010 and 24-hour speed counts taken on October 2, 2012.

⁴ Turn movement counts taken on October 2nd and 3rd, 2012.



lunch hour and the highest southbound volumes occur during the p.m. peak hours. The northbound direction is used more heavily during the a.m. hours and the southbound direction tends to have higher volumes during the p.m. hours. This directionality split is a typical commuting scenario with the a.m. flow towards the downtown business district and the p.m. traffic moving away from the downtown core.

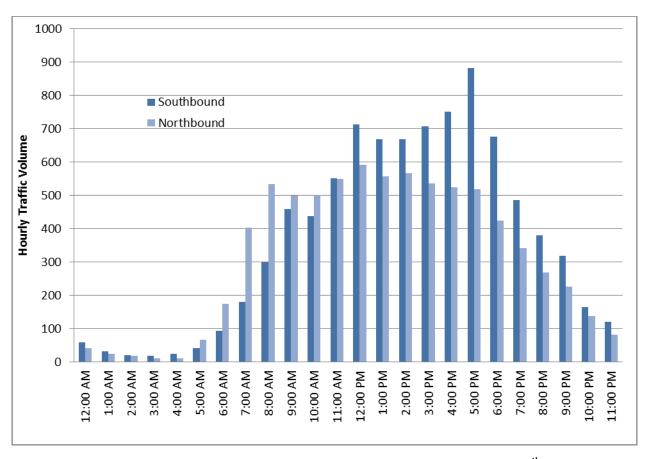
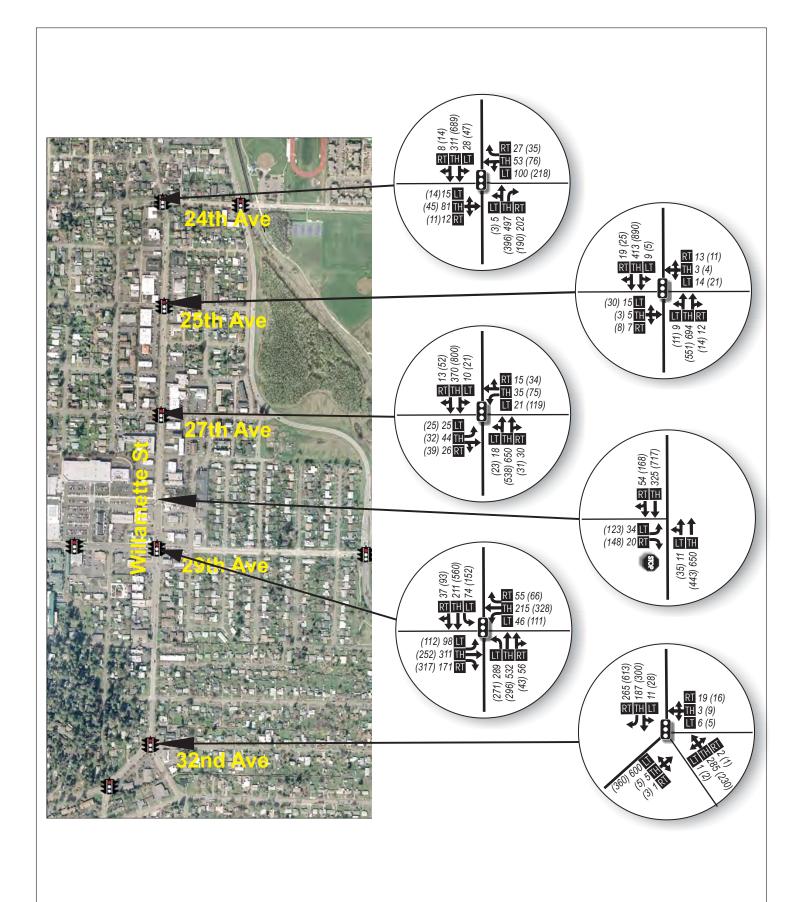


Figure 6: 24-Hour Bi-Directional Volume (Willamette Street south of 27th Avenue)

Intersection Turn Movement Volumes

Intersection turn movement volumes were collected at the six study intersections listed previously during the a.m. (7:00 a.m. to 10:00 a.m.) and the p.m. (4:00 p.m. to 7:00 p.m.) peak periods⁵. The a.m. and p.m. peak hour traffic volumes for the study intersection are shown in Figure 7 along with the associated lane configurations and traffic control. Figure 8 shows the a.m. and p.m. peak hour bicycle and pedestrian volumes at each intersection.

⁵ Turn movement counts taken on October 2nd and 3rd, 2012.





STOP - Stop Sign

- Traffic Signal



Lane Configuration

000 (000) - AM (PM) Peak Hour Traffic Volumes



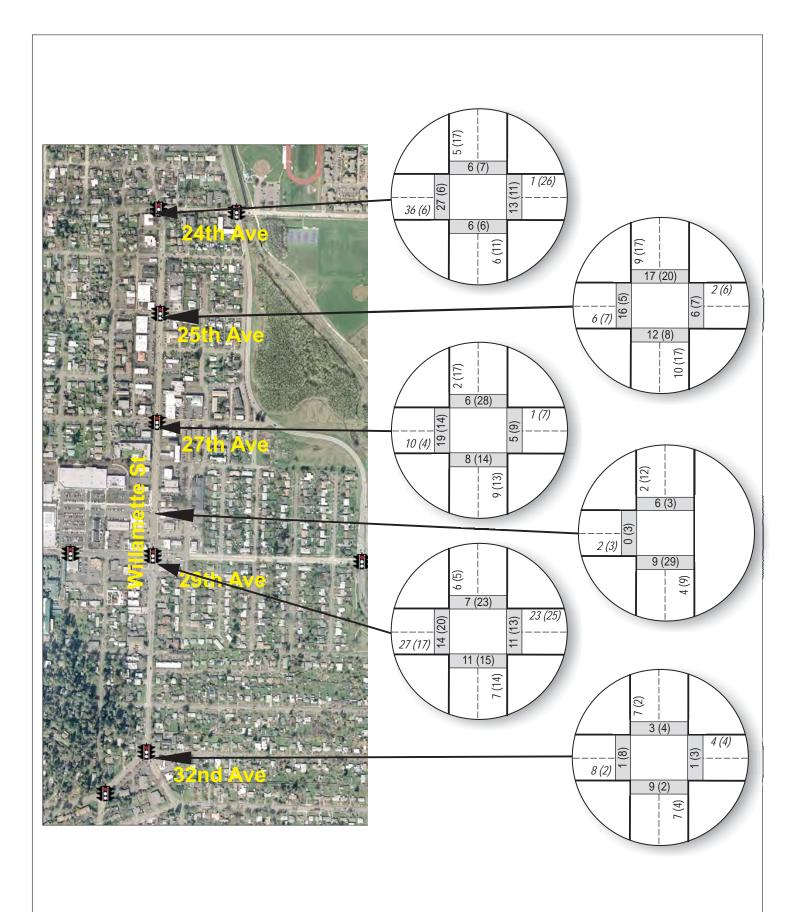
- Volume Turn Movement





Figure

EXISTING 2012 AM & PM PEAK HOUR TRAFFIC VOLUMES



LEGEND

000 (000) - AM (PM) Peak Hour Bicycle Volumes 000 (000) - AM (PM) Peak Hour Pedestrian Volumes





Figure

8

Existing Conditions, Forecast Conditions, and Planned Improvements February 12, 2013 Page 11 of 16



Mobility Standards

The City of Eugene bases intersection mobility standards on level of service (LOS) and currently specifies a minimum performance of LOS "D" at signalized and unsignalized intersections. 6

Intersection Operations

The existing traffic operations at the study intersections were determined for the a.m. and p.m. peak hours based on *2000 Highway Capacity Manual* methodology⁷. The estimated average delay, level of service (LOS), and volume to capacity (v/c) ratio of each study intersection are shown in Table 3. As shown, all of the study intersections currently meet operating standards. The Willamette Street/29th Avenue intersection experiences the greatest delay.

Table 3: Existing Intersection Operations

Tubic 3: Existing intersection	o per a trons						
Intonoction	Operating	Existing	a.M. Pe	eak Hour	Existing P.M. Peak Hour		
Intersection	Standard	Delay	LOS	V/C	Delay	LOS	V/C
Signalized							
Willamette Street/24 th Avenue	LOS D	9.5	Α	0.52 (0.53)	13.9	В	0.61 (0.74)
Willamette Street/25 th Avenue	LOS D	4.0	Α	0.34 (0.36)	9.3	Α	0.39 (0.49)
Willamette Street/27 th Avenue	LOS D	7.7	Α	0.34 (0.39)	8.4	Α	0.45 (0.46)
Willamette Street/29 th Avenue	LOS D	29.9	С	0.82 (0.82)	41.3	D	0.83 (0.85)
Willamette Street/32 nd Avenue	LOS D	26.4	С	0.97 (0.97)	10.5	В	0.67 (0.73)
Unsignalized							
Willamette Street/Willamette Plaza Driveway	N/A	0.7	A/B	0.29	3.4	A/C	0.44
Signalized Intersections: LOS = Level of Service of Intersection V/C = Volume-to-Capacity Ratio of Intersection (Critical Movement)			LOS =	lized Intersection Level of Servion Volume-to-Cap	ce of Major S		

Field Observations

Field observations were performed during the p.m. peak conditions at the study intersections. Extensive queuing was observed on the southbound approach to the Willamette Street/29th Avenue intersection which resulted in multiple cycle failures. It was also observed that the northbound left-turn movement experienced long queues that did not clear during each cycle.

⁶ The one exception to the City's LOS D mobility standard is that within the Central Area Transportation Study Area (primarily downtown and near the University of Oregon), the City allows LOS "E" for signalized intersection operations. However, this does not apply to the study corridor.

⁷ 2000 Highway Capacity Manual, Transportation Research Board, Washington DC, 2000.

Existing Conditions, Forecast Conditions, and Planned Improvements February 12, 2013 Page 12 of 16



Multimodal LOS

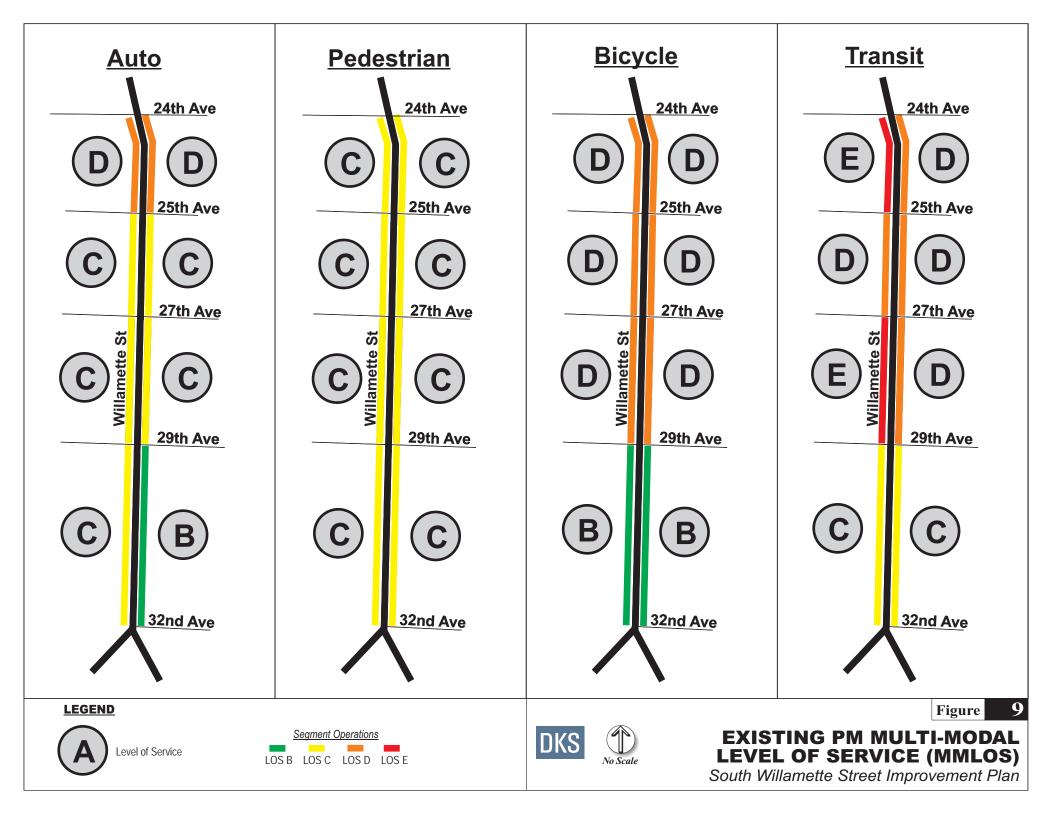
Auto, pedestrian, bicycle and transit operations along Willamette Street were evaluated using the multi-modal level of service (MMLOS) methodologies outlined in the *Highway Capacity Manual 2010 (HCM2010)*. This evaluation is performed for roadway segments (not accounting for intersection operations) and focuses on the users' perceived comfort level as they travel along the corridor. Using signalized intersections as break points, Willamette Street was divided into four segments for analysis. Analysis was performed based on p.m. peak hour conditions when the higher traffic volumes would result in the worst case level of service for each mode of transportation.

This analysis was performed using the *LOS+* software that is a hybrid tool that utilizes two different MMLOS methodologies⁸. The auto LOS component of the analysis is based on NCHRP Project 3-70, while the pedestrian, bicycle, and transit components are based on the HCM2010. While NCHRP 3-70 provided the basis for the MMLOS methodology described in the HCM2010, there were some significant differences. One of the main differences is that the LOS methodology for autos presented in the NCHRP 3-70 report requires less input data and is less intensive computationally.

Pedestrian LOS is influenced by traffic volumes, vehicle speeds, sidewalk width, and presence of a buffer. Bicycle LOS in influenced by bike lane width, pavement quality, on-street parking, and heavy vehicle percentage. Transit LOS is influenced by service frequency, bus reliability, average passenger load, and transit stop amenities. The LOS+ software assesses how well a facility meets the needs of the traveling community by reporting a LOS grade (A-F) for each mode of transportation.

The existing MMLOS operations for Willamette Street are shown in Figure 9. The auto, pedestrian, and bicycle LOS range from "B" to "D". The LOS for transit ranges from "C" to "E" based on the current bus service frequency. One transit route currently serves the Willamette Street segment from 24th Avenue to 29th Avenue which results in LOS "D/E". Two transit routes serve the corridor from 29th Avenue to 32nd Avenue, which is reflected in the LOS "C" operations for that segment. It should be noted that the existing LOS "D" MMLOS operations for the existing bicycle facilities was a better level of service than was expected. Based on stakeholder interviews, most bicycle users are not comfortable biking on Willamette Street without bike lanes. Therefore, it is clear that the comfort level of motor vehicles driving on a roadway with LOS "D" conditions is not a suitable comparison to cyclists travelling on a facility with LOS "D" conditions.

⁸ LOS+ Software developed by Fehr & Peers.



Existing Conditions, Forecast Conditions, and Planned Improvements February 12, 2013 Page 14 of 16



Collision Analysis

Collision analysis was performed for the study corridor as well as the study intersections to identify collision trends and potentially hazardous locations in need of safety improvements. The most recent three years of available collision data (2008-2010) were obtained from the ODOT Crash and Analysis Reporting Unit and verified against collision data provided by the City of Eugene.

In total, the Willamette Street corridor between 24th Avenue and 32nd Avenue experienced 74 collisions during the three years evaluated. Of the 74 reported collisions, 26 (35%) were related to movements into or out of an alley or driveway. The study corridor was divided into three segments with the northernmost segment including collisions occurring at the 24th Avenue intersection as well as those occurring up to and at the 27th Avenue intersection. The middle segment includes collisions occurring south of 27th Avenue up to and at 29th Avenue, and the southern segment includes collision occurring south of 29th Avenue through 32nd Avenue. Table 4 provides a summary of the collisions occurring along each segment.

The yearly collision rate for this segment of Willamette Street was calculated based on the total number of collisions occurring over the length of the study corridor. The resulting collision rate is 5.2 collisions per million vehicle-miles traveled (VMT). This is nearly double the statewide average of 2.91 collisions per million VMT for urban city minor arterial roadways for the same years (i.e., 2008-2010). ⁹

Table 4: 2008-2010 Segment Collision Summary

Segment (Distance)	Severity		Туре				Total	Collision
Segment (Distance)	Injury	PDO ^a	Turn	Rear-End	Angle	Other	Iotai	Rate ^b
24 th Ave thru 27 th Ave (0.30 mi.)	14	10	7	10	6	1	24	-
27 th Ave thru 29 th Ave (0.20 mi.)	15	18	22	8	1	2	33	-
29 th Ave thru 32 nd Ave (0.28 mi.)	11	6	6	10	0	1	17	-
Entire Study Corridor (0.78 mi.)	40	34	35	28	7	4	74	5.2
% of Total	54%	46%	47%	38%	10%	5%	100%	-

^a PDO = Property Damage Only

Collision analysis was also performed at the individual study intersections to pinpoint high collision locations. Table 5 lists the number of collisions at each study intersection and categorizes them by severity, type, and collision rate. Collisions occurring along this corridor are associated with the nearest intersection, although in many cases they are not specifically related to intersection operations. All collisions occurring within 100 feet of an intersection were included in the totals shown in Table 5. Individual vehicle movements were examined to determine if a collision should be included in the total for each study intersection when coded as occurring more than 100 feet from the intersection. For the years evaluated, there were 5 bike collisions, no pedestrian collisions, and no collisions resulting in a fatality. As shown, roughly half of the collisions resulted in an injury and over half of the collisions were related to turning movements.

^b Rate Calculation = Collision per year / (Average Daily Traffic x 365 days / 1 million vehicle-miles traveled)

⁹ 2011 State Highway Crash Rate Tables, ODOT Crash Analysis and Reporting Unit, August 2011; Table II, pg. 7.



Development to an update of the Eugene TSP¹⁰ identified a potential safety concern at the Willamette Street/29th Avenue intersection due to a collision rate of 1.40; however it also acknowledges that many of the collisions were related to driveways or alleys. The collision rate in this analysis for the Willamette Street/29th Avenue intersection was found to be significantly lower (0.76). As discussed above, this reduction is attributed to removing driveway-related collisions that were outside the influence of the intersection.

Table 5: 2008-2010 Intersection Collision Summary

Intersection	Seve	Severity		Туре				Collision
mersection	Injury	PDO ^a	Turn	Rear-End	Angle	Other	Total	Rate ^b
Willamette Street/24 th Avenue	2	2	0	1	3	0	4	0.21
Willamette Street/25 th Avenue	5	1	2	3	1	0	6	0.34
Willamette Street/27 th Avenue	5	4	4	2	2	1	9	0.44
Willamette Street/Willamette Plaza Driveway	3	5	8	0	0	0	8	0.45
Willamette Street/29 th Avenue	8	14	12	7	2	1	22	0.76
Willamette Street/32 nd Avenue	3	1	2	2	0	0	4	0.23
Total	26	27	28	15	8	2	53	-
% of Total	49%	51%	53%	28%	15%	4%	100%	-

^a PDO = Property Damage Only

PLANNED IMPROVEMENTS

To accurately predict future traffic volumes and operations, it is critical to identify planned improvements to the transportation system that would alter the existing conditions. This section summarizes the identified planned improvements that would impact the study corridor.

The Eugene TSP identifies potential projects to address recognized needs and deficiencies throughout the City; however it does not recommend specific projects at this time. One of the identified projects was a bike and pedestrian transformation between 24th Avenue and 32nd Avenue (the subject of this study).

The Eugene Pedestrian and Bicycle Master Plan (PBMP) identifies existing conditions and needed improvements to the bicycle and pedestrian facilities. In developing this plan, three corridors were selected for a more detailed level of feasibility analysis, one of which was Willamette Street from 18th Avenue to 32nd Avenue. As a companion document to the PBMP, a technical memorandum¹¹ documents the recommended improvements as well as several alternatives. Some of these alternatives maintain existing curb-to-curb widths, and some require utilization of additional available ROW to widen the road. Additional details regarding the identified alternative can be found in the referenced document.

^b Collisions per 1 million entering vehicles

 $^{^{}m 10}$ Eugene Transportation System Plan: Existing Conditions and Deficiencies, March 2011

¹¹ Eugene Pedestrian and Bicycle Master Plan Road Reconfiguration Assessment, May 2011



A rehabilitation of Willamette Street from 19th Avenue to 24th Avenue is planned to occur in 2013. This project will include replacing deteriorating and failing sections of pavement, as well as reconstructing sidewalk access ramps to meet accessibility standards. This project is currently in the public involvement process and the project website¹² has information about the public feedback received so far. Among the many concerns identified, roughly 96% of respondents to an online survey indicated that they would like to see bike lanes added to this segment of Willamette Street. If bike lanes are included in the redesign, it will further increase the benefit of providing connecting bike facilities from 24th Avenue to 32nd Avenue.

FORECAST CONDITIONS

This section provides a summary of future year (2035) motor vehicle traffic operations for the p.m. peak hour. Traffic operations analysis is based on *2000 Highway Capacity Manual* methodology¹³. The estimated average delay, level of service (LOS), and volume to capacity (v/c) ratio of each study intersection are shown in Table 6. As shown, all of the study intersections are anticipated to meet the minimum performance standard of LOS "D" operations, with the exception of the Willamette Street/29th Avenue intersection. Future year travel volume forecasts were developed using the regional travel demand model developed by the Lane Council of Governments (LCOG). Future year 2035 motor vehicle volumes and intersection operations are documented in the appendix.

Table 6: Future Intersection Operations

Intercetion	Operating	Existi	ng P.M. P	eak Hour	203	5 P.M. Pea	ak Hour
Intersection	Standard	Delay	LOS	V/C	Delay	LOS	V/C
Signalized							
Willamette Street/24 th Avenue	LOS D	13.9	В	0.61 (0.74)	16.0	В	0.72 (0.80)
Willamette Street/25 th Avenue	LOS D	9.3	Α	0.39 (0.49)	11.3	В	0.45 (0.56)
Willamette Street/27 th Avenue	LOS D	8.4	Α	0.45 (0.46)	11.6	В	0.57 (0.60)
Willamette Street/29 th Avenue	LOS D	41.3	D	0.83 (0.85)	70.8	E	>1.0 (>1.0)
Willamette Street/32 nd Avenue	LOS D	10.5	В	0.67 (0.73)	18.6	В	0.81 (0.95)
Unsignalized							
Willamette Street/Willamette Plaza Driveway	N/A	3.4	A/C	0.44	3.7	A/D	0.53
Signalized Intersections: LOS = Level of Service of Intersection V/C = Volume-to-Capacity Ratio of Intersection (Critical			LOS =	ized Intersection Level of Servic Volume-to-Capa	e of Major S		

Movement)

¹² http://www.eugene-or.gov/index.aspx?NID=2195

¹³ 2000 Highway Capacity Manual, Transportation Research Board, Washington DC, 2000.



Appendix

Level of Service Descriptions

HCM Analysis – Existing

HCM Analysis – Future



Level of Service Descriptions

TRAFFIC LEVELS OF SERVICE

Analysis of traffic volumes is useful in understanding the general nature of traffic in an area, but by itself indicates neither the ability of the street network to carry additional traffic nor the quality of service afforded by the street facilities. For this, the concept of *level of service* has been developed to subjectively describe traffic performance. Level of service can be measured at intersections and along key roadway segments.

Level of service categories are similar to report card ratings for traffic performance. Intersections are typically the controlling bottlenecks of traffic flow and the ability of a roadway system to carry traffic efficiently is generally diminished in their vicinities. Levels of Service A, B and C indicate conditions where traffic moves without significant delays over periods of peak travel demand. Level of service D and E are progressively worse peak hour operating conditions and F conditions represent where demand exceeds the capacity of an intersection. Most urban communities set level of service D as the minimum acceptable level of service for peak hour operation and plan for level of service C or better for all other times of the day. The *Highway Capacity Manual* provides level of service calculation methodology for both intersections and arterials. The following two sections provide interpretations of the analysis approaches.

¹ 2000 Highway Capacity Manual, Transportation Research Board, Washington D.C., 2000, Chapters 16 and 17.

UNSIGNALIZED INTERSECTIONS (Two-Way Stop Controlled)

Unsignalized intersection level of service is reported for the major street and minor street (generally, left turn movements). The method assesses available and critical gaps in the traffic stream which make it possible for side street traffic to enter the main street flow. The 2000 Highway Capacity Manual describes the detailed methodology. It is not unusual for an intersection to experience level of service E or F conditions for the minor street left turn movement. It should be understood that, often, a poor level of service is experienced by only a few vehicles and the intersection as a whole operates acceptably.

Unsignalized intersection levels of service are described in the following table.

Expected Delay	(Sec/Veh)	
Little or no delay	0-10.0	
Short traffic delay	>10.1-15.0	
Average traffic delays	>15.1-25.0	
Long traffic delays	>25.1-35.0	
Very long traffic delays	>35.1-50.0	
Extreme delays potentially affecting other traffic movements in the intersection	> 50	
way Capacity Manual, Transportation Research Board Washington, D.C.		
	Little or no delay Short traffic delay Average traffic delays Long traffic delays Very long traffic delays Extreme delays potentially affecting other traffic movements in the intersection	Little or no delay O-10.0 Short traffic delay Average traffic delays State of the state of t

SIGNALIZED INTERSECTIONS

For signalized intersections, level of service is evaluated based upon average vehicle delay experienced by vehicles entering an intersection. Control delay (or signal delay) includes initial deceleration delay, queue move-up time, stopped delay, and final acceleration delay. In previous versions of this chapter of the HCM (1994 and earlier), delay included only stopped delay. As delay increases, the level of service decreases. Calculations for signalized and unsignalized intersections are different due to the variation in traffic control. The 2000 Highway Capacity Manual provides the basis for these calculations.

(secs.) <10.00	Tree Flow/Insignificant Delays: No approach phase is fully utilized by traffic and no vehicle wai longer than one red indication. Most vehicles do not stop at all. Progression is extremely favorable ar
≤10.00	
	most vehicles arrive during the green phase.
10.1-20.0	Stable Operation/Minimal Delays: An occasional approach phase is fully utilized. Many drivers beg to feel somewhat restricted within platoons of vehicles. This level generally occurs with good progressic short cycle lengths, or both.
20.1-35.0	Stable Operation/Acceptable Delays: Major approach phases fully utilized. Most drivers feel somewherestricted. Higher delays may result from fair progression, longer cycle lengths, or both. Individual cycle failures may begin to appear at this level, and the number of vehicles stopping is significant.
35.1-55.0	Approaching Unstable/Tolerable Delays: The influence of congestion becomes more noticeable Drivers may have to wait through more than one red signal indication. Longer delays may result fro some combination of unfavorable progression, long cycle lengths, or high v/c ratios. The proportion vehicles not stopping declines, and individual cycle failures are noticeable.
55.1-80.0	Unstable Operation/Significant Delays: Volumes at or near capacity. Vehicles may wait though sever signal cycles. Long queues form upstream from intersection. These high delay values generally indicate poor progression, long cycle lengths, and high v/c ratios. Individual cycle failures are a freque occurrence.
≥80.0	Forced Flow/Excessive Delays: Represents jammed conditions. Queues may block upstreat intersections. This level occurs when arrival flow rates exceed intersection capacity, and is considered be unacceptable to most drivers. Poor progression, long cycle lengths, and v/c ratios approaching 1.0 m contribute to these high delay levels.
Source: 2000 F	Highway Capacity Manual, Transportation Research Board, Washington D.C.
	35.1-55.0 55.1-80.0 ≥80.0



HCM Analysis – Existing AM Peak

c Critical Lane Group

Movement Lane Configurations 414 Volume (vph) 15 81 53 27 497 202 311 Ideal Flow (vphpl) 1750 1750 1750 1750 1750 1750 1750 1750 1750 1750 1750 1750 Total Lost time (s) 4.5 4.5 4.5 4.5 Lane Util. Factor 1.00 1.00 1.00 1.00 1.00 0.95 Frpb, ped/bikes 1.00 1.00 0.92 1.00 0.93 1.00 Flpb, ped/bikes 1.00 1.00 1.00 1.00 1.00 1.00 0.99 1.00 0.85 0.85 1.00 1.00 Flt Protected 0.99 0.97 1.00 1.00 1.00 1.00 Satd. Flow (prot) 1694 1677 1367 1715 1363 3079 Flt Permitted 0.95 0.76 1.00 1.00 1.00 0.90 Satd. Flow (perm) 1626 1318 1367 1711 1363 2770 0.91 0.91 Peak-hour factor, PHF 0.91 0.91 0.91 0.91 0.91 0.91 0.91 0.91 0.91 0.91 Adj. Flow (vph) 89 13 110 58 30 546 222 31 342 16 5 9 RTOR Reduction (vph) 0 0 0 0 22 0 0 87 0 0 Lane Group Flow (vph) 551 380 0 168 0 135 0 Confl. Peds. (#/hr) 13 27 27 13 6 Confl. Bikes (#/hr) Heavy Vehicles (%) 0% 1% 0% 0% 0% 0% 2% 1% 4% 2% 7% 12% Turn Type NA NA NA NA Perm Perm Protected Phases 2 8 4 Permitted Phases 18.5 Actuated Green, G (s) 18.5 18.5 42.5 42.5 42.5 42.5 Effective Green, g (s) 18.5 18.5 18.5 42.5 42.5 Actuated g/C Ratio 0.26 0.26 0.26 0.61 0.61 0.61 Clearance Time (s) 4.5 4.5 4.5 4.5 4.5 4.5 Vehicle Extension (s) 3.0 3.0 3.0 2.0 2.0 2.0 429 Lane Grp Cap (vph) 348 361 1038 827 1681 v/s Ratio Prot v/s Ratio Perm 0.07 c0.13 0.01 c0.32 0.10 0.14 v/c Ratio 0.26 0.48 0.02 0.53 0.16 0.23 Uniform Delay, d1 20.3 21.7 19.1 8.0 6.0 6.3 Progression Factor 1.00 1.00 1.00 0.61 0.68 1.00 Incremental Delay, d2 0.3 1.1 0.0 0.4 1.9 0.3 Delay (s) 20.6 22.8 19.1 6.7 4.5 6.6 Level of Service С С В Α Approach Delay (s) 20.6 22.2 6.1 6.6 Approach LOS С С Intersection Summary HCM 2000 Control Delay 9.5 HCM 2000 Level of Service HCM 2000 Volume to Capacity ratio 0.52 Actuated Cycle Length (s) 70.0 Sum of lost time (s) Intersection Capacity Utilization 70.0% ICU Level of Service С 15 Analysis Period (min)

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			414			414	
Volume (vph)	15	5	7	14	3	13	9	694	12	9	413	19
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Total Lost time (s)		4.5			4.5			4.5			4.5	
Lane Util. Factor		1.00			1.00			0.95			0.95	
Frpb, ped/bikes		0.99			0.98			1.00			1.00	
Flpb, ped/bikes		0.99			0.99			1.00			1.00	
Frt		0.97			0.94			1.00			0.99	
Flt Protected		0.97			0.98			1.00			1.00	
Satd. Flow (prot)		1615			1570			3276			3204	
Flt Permitted		0.81			0.84			0.95			0.94	
Satd. Flow (perm)		1342			1352			3109			3010	
Peak-hour factor, PHF	0.83	0.83	0.83	0.83	0.83	0.83	0.83	0.83	0.83	0.83	0.83	0.83
Adj. Flow (vph)	18	6	8	17	4	16	11	836	14	11	498	23
RTOR Reduction (vph)	0	7	0	0	14	0	0	1	0	0	2	0
Lane Group Flow (vph)	0	25	0	0	23	0	0	860	0	0	530	0
Confl. Peds. (#/hr)	17	20	12	12	20	17	6	000	16	16	000	6
Confl. Bikes (#/hr)	.,		2	12		6			9	10		10
Heavy Vehicles (%)	0%	0%	0%	0%	0%	0%	0%	1%	8%	0%	3%	0%
Turn Type	Perm	NA		Perm	NA		Perm	NA		Perm	NA	
Protected Phases		2			6			4			8	
Permitted Phases	2			6			4			8		
Actuated Green, G (s)		7.2			7.2			53.8			53.8	
Effective Green, q (s)		7.2			7.2			53.8			53.8	
Actuated g/C Ratio		0.10			0.10			0.77			0.77	
Clearance Time (s)		4.5			4.5			4.5			4.5	
Vehicle Extension (s)		3.0			3.0			2.0			2.0	
Lane Grp Cap (vph)		138			139			2389			2313	
v/s Ratio Prot												
v/s Ratio Perm		c0.02			0.02			c0.28			0.18	
v/c Ratio		0.18			0.16			0.36			0.23	
Uniform Delay, d1		28.7			28.7			2.6			2.3	
Progression Factor		1.00			1.00			0.59			1.71	
Incremental Delay, d2		0.6			0.6			0.4			0.2	
Delay (s)		29.3			29.2			1.9			4.1	
Level of Service		27.5 C			C			A			A	
Approach Delay (s)		29.3			29.2			1.9			4.1	
Approach LOS		C			C			Α			A	
Intersection Summary	_	_	_	_	-	_	_		_	_		
HCM 2000 Control Delay			4.0	Li	CM 2000	Lovelof	Convice		A			
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Intersection Capacity Utiliza	tion		50.7%		um of lost U Level o				9.0 A			
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Analysis Period (min)			15									
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1.00	1 00		4.5	4.5			4.5			4.5	
	1.00		1.00	1.00			0.95			0.95	
0.00	0.99		1.00	0.99			1.00			1.00	
0.99	1.00		0.99	1.00			1.00			1.00	
1.00	0.94		1.00	0.96			0.99			1.00	
0.95	1.00		0.95	1.00			1.00			1.00	
1588	1592		1649	1657			3261			3143	
0.72	1.00		0.70	1.00			0.94			0.94	
1203	1592		1222	1657			3067			2948	
0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87
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Perm	NA		Perm	NA		Perm	NA		Perm	NA	
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2			6			4			8		
	14.4			14.4			46.6			46.6	
14.4	14.4		14.4	14.4			46.6			46.6	
0.21	0.21		0.21	0.21			0.67			0.67	
4.5	4.5		4.5	4.5			4.5			4.5	
3.0	3.0		3.0	3.0			2.0			2.0	
247	327		251	340			2041			1962	
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	۶	\rightarrow	4	†	↓	4	
Movement	EBL	EBR	NBL	NBT	SBT	SBR	
Lane Configurations		7		414	↑ ↑		
Volume (veh/h)	34	20	11	650	325	54	
Sign Control	Stop			Free	Free		
Grade	0%			0%	0%		
Peak Hour Factor	0.87	0.87	0.87	0.87	0.87	0.87	
Hourly flow rate (vph)	39	23	13	747	374	62	
Pedestrians				9	6		
Lane Width (ft)				12.0	12.0		
Walking Speed (ft/s)				4.0	4.0		
Percent Blockage				1	1		
Right turn flare (veh)							
Median type				None	None		
Median storage veh)							
Upstream signal (ft)				341	673		
pX, platoon unblocked	0.86						
vC, conflicting volume	809	227	436				
vC1, stage 1 conf vol							
vC2, stage 2 conf vol							
vCu, unblocked vol	445	227	436				
tC, single (s)	6.9	6.9	4.1				
tC, 2 stage (s)							
tF (s)	3.5	3.3	2.2				
p0 queue free %	91	97	99				
cM capacity (veh/h)	455	776	1135				
Direction, Lane #	EB 1	EB 2	NB 1	NB 2	SB 1	SB 2	
Volume Total	39	23	262	498	249	187	
Volume Left	39	0	13	0	0	0	
Volume Right	0	23	0	0	0	62	
cSH	455	776	1135	1700	1700	1700	
Volume to Capacity	0.09	0.03	0.01	0.29	0.15	0.11	
Queue Length 95th (ft)	7	2	1	0	0	0	
Control Delay (s)	13.7	9.8	0.5	0.0	0.0	0.0	
Lane LOS	В	Α	Α				
Approach Delay (s)	12.2		0.2		0.0		
Approach LOS	В						
Intersection Summary							
Average Delay			0.7				
Intersection Capacity Utiliza	ition		40.6%	IC	CU Level o	of Service	A
Analysis Period (min)			15				

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ኻ	*	7	ሻ	†	7	ች	∱ î>		*	† 1>	
Volume (vph)	98	311	171	46	215	55	289	532	56	74	211	37
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Lane Width	12	10	10	10	10	11	10	10	12	12	12	12
Total Lost time (s)	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5		4.5	4.5	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95		1.00	0.95	
Frpb, ped/bikes	1.00	1.00	0.98	1.00	1.00	0.95	1.00	1.00		1.00	0.99	
Flpb, ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00		1.00	1.00	
Frt	1.00	1.00	0.85	1.00	1.00	0.85	1.00	0.99		1.00	0.98	
Flt Protected	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00		0.95	1.00	
Satd. Flow (prot)	1630	1601	1264	1349	1586	1373	1492	2986		1539	3021	
Flt Permitted	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00		0.95	1.00	
Satd. Flow (perm)	1630	1601	1264	1349	1586	1373	1492	2986		1539	3021	
Peak-hour factor, PHF	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88
Adj. Flow (vph)	111	353	194	52	244	62	328	605	64	84	240	42
RTOR Reduction (vph)	0	0	90	0	0	48	0	9	0	0	17	0
Lane Group Flow (vph)	111	353	104	52	244	14	328	660	0	84	265	0
Confl. Peds. (#/hr)	7		11	11		7	11		14	14		11
Confl. Bikes (#/hr)			23			27			6			7
Heavy Vehicles (%)	2%	2%	8%	15%	3%	0%	4%	1%	11%	8%	6%	11%
Turn Type	Prot	NA	pm+ov	Prot	NA	Perm	Prot	NA		Prot	NA	
Protected Phases	5	2	3	1	6		3	8		7	4	
Permitted Phases			2			6						
Actuated Green, G (s)	6.7	20.3	40.5	3.6	17.2	17.2	20.2	26.8		6.5	13.1	
Effective Green, q (s)	6.7	20.3	40.5	3.6	17.2	17.2	20.2	26.8		6.5	13.1	
Actuated g/C Ratio	0.09	0.27	0.54	0.05	0.23	0.23	0.27	0.36		0.09	0.17	
Clearance Time (s)	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5		4.5	4.5	
Vehicle Extension (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0		2.0	2.0	
Lane Grp Cap (vph)	145	432	756	64	362	314	400	1064		133	526	
v/s Ratio Prot	c0.07	c0.22	0.04	0.04	0.15		c0.22	c0.22		0.05	0.09	
v/s Ratio Perm			0.05			0.01						
v/c Ratio	0.77	0.82	0.14	0.81	0.67	0.05	0.82	0.62		0.63	0.50	
Uniform Delay, d1	33.5	25.7	8.6	35.5	26.4	22.6	25.8	20.0		33.2	28.1	
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00		1.00	1.00	
Incremental Delay, d2	19.2	10.8	0.0	50.3	3.9	0.0	11.8	0.8		7.0	0.3	
Delay (s)	52.7	36.5	8.7	85.7	30.3	22.6	37.6	20.8		40.2	28.4	
Level of Service	D	D	Α	F	С	С	D	С		D	С	
Approach Delay (s)		31.0			37.0			26.3			31.1	
Approach LOS		С			D			С			С	
Intersection Summary												
HCM 2000 Control Delay			30.0	Н	CM 2000	Level of	Service		С			
HCM 2000 Volume to Capacit	ty ratio		0.82									
Actuated Cycle Length (s)			75.2	S	um of lost	t time (s)			18.0			
Intersection Capacity Utilization	on		64.3%	IC	CU Level	of Service	9		С			
Analysis Period (min)			15									
c Critical Lane Group												

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			ર્ન	7
Volume (vph)	600	5	1	6	3	19	1	285	2	11	187	265
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Total Lost time (s)		5.0			4.0			4.0			4.0	5.0
Lane Util. Factor		1.00			1.00			1.00			1.00	1.00
Frpb, ped/bikes		1.00			0.98			1.00			1.00	0.98
Flpb, ped/bikes		1.00			1.00			1.00			1.00	1.00
Frt		1.00			0.91			1.00			1.00	0.85
Flt Protected		0.95			0.99			1.00			1.00	1.00
Satd. Flow (prot)		1629			1545			1681			1666	1403
Flt Permitted		0.67			0.91			1.00			0.97	1.00
Satd. Flow (perm)		1138			1423			1680			1617	1403
Peak-hour factor, PHF	0.84	0.84	0.84	0.84	0.84	0.84	0.84	0.84	0.84	0.84	0.84	0.84
Adj. Flow (vph)	714	6	1	7	4	23	1	339	2	13	223	315
RTOR Reduction (vph)	0	0	0	0	10	0	0	0	0	0	0	0
Lane Group Flow (vph)	0	721	0	0	24	0	0	342	0	0	236	315
Confl. Peds. (#/hr)	2		3	3		2	4		4	4		4
Confl. Bikes (#/hr)			4			8			7			7
Heavy Vehicles (%)	2%	0%	0%	0%	0%	0%	0%	4%	0%	0%	5%	4%
Turn Type	custom	NA		Perm	NA		Perm	NA		Perm	NA	custom
Protected Phases					8			2			6	
Permitted Phases	3	3		8			2			6		36
Actuated Green, G (s)		51.0			45.7			18.4			18.4	78.4
Effective Green, g (s)		51.0			45.7			18.4			18.4	74.4
Actuated g/C Ratio		0.65			0.58			0.23			0.23	0.95
Clearance Time (s)		5.0			4.0			4.0			4.0	
Vehicle Extension (s)		2.0			2.5			2.5			2.5	
Lane Grp Cap (vph)		740			829			394			379	1331
v/s Ratio Prot												
v/s Ratio Perm		c0.63			0.02			c0.20			0.15	0.22
v/c Ratio		0.97			0.03			0.87			0.62	0.24
Uniform Delay, d1		13.1			6.9			28.8			26.9	0.1
Progression Factor		1.00			1.00			1.00			1.00	1.00
Incremental Delay, d2		26.5			0.0			17.8			2.7	0.0
Delay (s)		39.6			6.9			46.6			29.6	0.2
Level of Service		D			Α			D			С	Α
Approach Delay (s)		39.6			6.9			46.6			12.8	
Approach LOS		D			Α			D			В	
Intersection Summary												
HCM 2000 Control Delay			31.4	Н	CM 2000	Level of	Service		С			
HCM 2000 Volume to Capa	acity ratio		0.97									
Actuated Cycle Length (s)	,		78.4	Sı	um of lost	time (s)			11.0			
Intersection Capacity Utiliza	ation		70.2%			of Service			С			
Analysis Period (min)			15									
c Critical Lane Group												

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HCM Analysis – Existing PM Peak

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			र्स	7		4	7		414	
Volume (vph)	14	45	11	218	76	35	3	396	190	47	689	14
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Total Lost time (s)		4.5			4.5	4.5		4.5	4.5		4.5	
Lane Util. Factor		1.00			1.00	1.00		1.00	1.00		0.95	
Frpb, ped/bikes		0.99			1.00	0.96		1.00	0.96		1.00	
Flpb, ped/bikes		1.00			1.00	1.00		1.00	1.00		1.00	
Frt		0.98			1.00	0.85		1.00	0.85		1.00	
Flt Protected		0.99			0.96	1.00		1.00	1.00		1.00	
Satd. Flow (prot)		1684			1675	1427		1732	1408		3271	
Flt Permitted		0.92			0.73	1.00		1.00	1.00		0.90	
Satd. Flow (perm)		1565			1273	1427		1726	1408		2958	
Peak-hour factor, PHF	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Adj. Flow (vph)	15	49	12	240	84	38	3	435	209	52	757	15
RTOR Reduction (vph)	0	8	0	0	0	25	0	0	97	0	1	0
Lane Group Flow (vph)	0	68	0	0	324	13	0	438	112	0	823	0
Confl. Peds. (#/hr)	7		6	6		7	11		6	6		11
Confl. Bikes (#/hr)			26			6			17			11
Heavy Vehicles (%)	0%	0%	0%	0%	1%	0%	0%	1%	1%	0%	1%	0%
Turn Type	Perm	NA		Perm	NA	Perm	Perm	NA	Perm	Perm	NA	
Protected Phases		2			6			8			4	
Permitted Phases	2			6		6	8		8	4		
Actuated Green, G (s)		25.8			25.8	25.8		40.2	40.2		40.2	
Effective Green, g (s)		25.8			25.8	25.8		40.2	40.2		40.2	
Actuated g/C Ratio		0.34			0.34	0.34		0.54	0.54		0.54	
Clearance Time (s)		4.5			4.5	4.5		4.5	4.5		4.5	
Vehicle Extension (s)		3.0			3.0	3.0		2.0	2.0		2.0	
Lane Grp Cap (vph)		538			437	490		925	754		1585	
v/s Ratio Prot												
v/s Ratio Perm		0.04			c0.25	0.01		0.25	0.08		c0.28	
v/c Ratio		0.13			0.74	0.03		0.47	0.15		0.52	
Uniform Delay, d1		16.9			21.7	16.3		10.8	8.8		11.2	
Progression Factor		1.00			1.00	1.00		0.64	0.78		1.00	
Incremental Delay, d2		0.1			6.7	0.0		1.7	0.4		1.2	
Delay (s)		17.0			28.3	16.3		8.6	7.2		12.4	
Level of Service		В			С	В		Α	Α		В	
Approach Delay (s)		17.0			27.1			8.2			12.4	
Approach LOS		В			С			Α			В	
Intersection Summary												
HCM 2000 Control Delay			13.9	Н	CM 2000	Level of	Service		В			
HCM 2000 Volume to Capac	city ratio		0.61									
Actuated Cycle Length (s)			75.0	Si	um of lost	time (s)			9.0			
Intersection Capacity Utilizat	tion		80.8%	IC	U Level	of Service)		D			
Analysis Period (min)			15									
c Critical Lane Group												

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EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBF
	4			4			414			414	
30	3	8	21	4	11		551	14	5	890	25
1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
	4.5			4.5			4.5			4.5	
	1.00			1.00			0.95			0.95	
	0.99			0.99			1.00			1.00	
	0.98			0.99			1.00			1.00	
	0.97			0.96			1.00			1.00	
	0.96			0.97			1.00			1.00	
	1570			1599			3275			3275	
	0.82			0.86			0.93			0.95	
	1340			1419			3063			3121	
0.93						0.93	0.93	0.93			0.93
32		9	23	4	12	12	592	15	5	957	27
0	7	0	0	9	0	0	2	0	0	2	0
0	37	0	0	30	0	0	617	0	0	987	0
20		8	8		20	7		5	5		7
		6			7			17			17
3%	0%	0%	0%	0%	0%	0%	1%	0%	0%	1%	0%
Perm	NA		Perm	NA		Perm	NA		Perm	NA	
	2			6			4			8	
2			6			4			8		
	18.0			18.0			48.0			48.0	
	18.0			18.0			48.0			48.0	
	0.24			0.24			0.64			0.64	
	4.5			4.5			4.5			4.5	
	3.0			3.0			2.0			2.0	
	321			340			1960			1997	
	c0.03			0.02			0.20			c0.32	
	0.12			0.09			0.31			0.49	
	22.3			22.1			6.1			7.1	
	1.00			1.00			1.76			0.88	
	0.2			0.1			0.4			0.8	
	22.4			22.2			11.1			7.0	
	С			С			В			Α	
	22.4			22.2			11.1			7.0	
	С			С			В			Α	
		9.3	H	CM 2000	Level of	Service		А			
ratio		0.39									
		75.0	Sı	um of lost	time (s)			9.0			
n		53.9%				9		Α			
	0.93 32 0 0 20 3%	EBL EBT 30 3 31750 1750 4.5 1.00 0.99 0.98 0.97 0.96 1570 0.82 1340 0.93 32 3 0 7 0 37 20 3% 0% Perm NA 22 2 18.0 18.0 0.24 4.5 3.0 321 c0.03 0.12 22.3 1.00 0.2 22.4 C C	EBL EBT EBR 30 3 8 1750 1750 1750 4.5 1.00 0.99 0.98 0.97 0.96 1570 0.82 1340 0.93 0.93 0.93 32 3 9 0 7 0 0 37 0 20 8 6 3% 0% 0% Perm NA 2 2 2 18.0 18.0 18.0 0.24 4.5 3.0 18.0 0.24 4.5 3.0 18.0 0.24 4.5 3.0 18.0 0.24 4.5 3.0 18.0 0.24 4.5 3.0 0.24 4.5 3.0 0.24 4.5 3.0 0.24 4.5 3.0 0.24 4.5 3.0 0.24 4.5 3.0 0.24 4.5 3.0 0.24 4.5 3.0 0.24 4.5 3.0 0.24 0.24 0.24 0.25 0.24 0.26 0.27 0.27 0.38 0.39 0.39 0.39 0.39 0.39 0.39 0.39 0.39	30 3 8 21 1750 1750 1750 1750 4.5 1.00 0.999 0.98 0.97 0.96 1570 0.82 1340 0.93 0.93 0.93 0.93 32 3 9 23 30 7 0 0 0 0 37 0 0 0 0 37 0 0 0 0 37 0 0 0 0 37 0 0 0 0 18 8 8 8 6 3% 0% 0% 0% 0% Perm NA Perm 2 2 2 6 18.0 18.0 0.24 4.5 3.0 321 c0.03 0.12 22.3 1.00 0.2 22.4 C C 22.4 C C 22.4 C C 9.3 HH 7 ratio 0.39 7 5.0 Sign 150	EBL EBT EBR WBL WBT	BBL BBT BBR WBL WBT WBR	EBL EBT EBR WBL WBT WBR NBL	Fig. Fig.	FBL	FBL FBR FBR	EBL EBT EBR WBL WBT WBR NBL NBT NBR SBL SBT 30 3 8 21 4 11 11 555 14 5 890 1750 1750 1750 1750 1750 1750 1750 1750

 DKS Associates
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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	1>		٦	1>			414			€Î}	
Volume (vph)	25	32	39	119	75	34	23	538	31	21	800	52
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Total Lost time (s)	4.5	4.5		4.5	4.5			4.5			4.5	
Lane Util. Factor	1.00	1.00		1.00	1.00			0.95			0.95	
Frpb, ped/bikes	1.00	0.98		1.00	0.98			1.00			1.00	
Flpb, ped/bikes	0.97	1.00		0.98	1.00			1.00			1.00	
Frt	1.00	0.92		1.00	0.95			0.99			0.99	
Flt Protected	0.95	1.00		0.95	1.00			1.00			1.00	
Satd. Flow (prot)	1613	1554		1620	1609			3218			3247	
Flt Permitted	0.68	1.00		0.71	1.00			0.91			0.93	
Satd. Flow (perm)	1163	1554		1210	1609			2921			3038	
Peak-hour factor, PHF	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Adj. Flow (vph)	26	33	40	123	77	35	24	555	32	22	825	54
RTOR Reduction (vph)	0	30	0	0	27	0	0	4	0	0	4	0
Lane Group Flow (vph)	26	43	0	123	85	0	0	607	0	0	897	0
Confl. Peds. (#/hr)	28	13	14	14	00	28	9	007	14	14	077	9
Confl. Bikes (#/hr)	20		7			4			15			13
Heavy Vehicles (%)	0%	3%	0%	1%	3%	0%	0%	2%	3%	5%	1%	0%
Turn Type	Perm	NA		Perm	NA		Perm	NA		Perm	NA	
Protected Phases		2			6			4			8	
Permitted Phases	2			6			4			8		
Actuated Green, G (s)	18.2	18.2		18.2	18.2			47.8		-	47.8	
Effective Green, g (s)	18.2	18.2		18.2	18.2			47.8			47.8	
Actuated g/C Ratio	0.24	0.24		0.24	0.24			0.64			0.64	
Clearance Time (s)	4.5	4.5		4.5	4.5			4.5			4.5	
Vehicle Extension (s)	3.0	3.0		3.0	3.0			2.0			2.0	
Lane Grp Cap (vph)	282	377		293	390			1861			1936	
v/s Ratio Prot		0.03			0.05							
v/s Ratio Perm	0.02			c0.10				0.21			c0.30	
v/c Ratio	0.09	0.11		0.42	0.22			0.33			0.46	
Uniform Delay, d1	22.0	22.1		23.9	22.7			6.2			7.0	
Progression Factor	1.00	1.00		1.00	1.00			1.00			0.45	
Incremental Delay, d2	0.1	0.1		1.0	0.3			0.5			0.7	
Delay (s)	22.1	22.3		24.9	23.0			6.7			3.9	
Level of Service	C	C		C	C			Α.			Α.,	
Approach Delay (s)	C	22.2		C	24.0			6.7			3.9	
Approach LOS		C			C C			Α.			Α.	
Intersection Summary												
HCM 2000 Control Delay			8.4	Ц	CM 2000	Lovol of	Sorvico		A			
HCM 2000 Control Delay	city ratio		0.45	П	CIVI 2000	FGACI OI	OCI VICE					
Actuated Cycle Length (s)	icity ratio		75.0	C	um of lost	time (c)			9.0			
Intersection Capacity Utiliza	ation		64.9%		um or iosi CU Level o		`		9.0 C			
Analysis Period (min)	JUOH		15	IC	O LEVEL	JI JEI VILE			C			
c Critical Lane Group			13									
Contical Latte Group												

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Movement	EBL	EBR	NBL	NBT	SBT	SBR	
Lane Configurations		7		414	† 1>		
Volume (veh/h)	123	148	35	443	717	168	
Sign Control	Stop			Free	Free		
Grade	0%			0%	0%		
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97	
Hourly flow rate (vph)	127	153	36	457	739	173	
Pedestrians				29	3		
Lane Width (ft)				12.0	12.0		
Walking Speed (ft/s)				4.0	4.0		
Percent Blockage				2	0		
Right turn flare (veh)							
Median type				None	None		
Median storage veh)							
Upstream signal (ft)				341	673		
pX, platoon unblocked	0.97	0.94	0.94				
vC, conflicting volume	1129	485	912				
vC1, stage 1 conf vol							
vC2, stage 2 conf vol							
vCu, unblocked vol	821	327	781				
tC, single (s)	6.8	6.9	4.2				
tC, 2 stage (s)							
tF (s)	3.5	3.3	2.2				
p0 queue free %	56	75	95				
cM capacity (veh/h)	289	617	777				
Direction, Lane #	EB 1	EB 2	NB 1	NB 2	SB 1	SB 2	
Volume Total	127	153	188	304	493	420	
Volume Left	127	0	36	0	0	0	
Volume Right	0	153	0	0	0	173	
cSH	289	617	777	1700	1700	1700	
Volume to Capacity	0.44	0.25	0.05	0.18	0.29	0.25	
Queue Length 95th (ft)	53	24	4	0	0	0	
Control Delay (s)	26.8	12.7	2.3	0.0	0.0	0.0	
Lane LOS	D	В	Α				
Approach Delay (s)	19.1		0.9		0.0		
Approach LOS	С						
Intersection Summary							
Average Delay			3.4				
Intersection Capacity Utiliza	ition		60.0%	IC	CU Level o	of Service	В
Analysis Period (min)			15				

 DKS Associates
 Synchro 8 - Report

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	*	*	7	ሻ	↑	7	ሻ	↑ ↑		*	↑ ↑	
Volume (vph)	112	252	317	111	328	66	271	298	43	152	560	93
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Lane Width	12	10	10	10	10	11	10	10	12	12	12	12
Total Lost time (s)	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5		4.5	4.5	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95		1.00	0.95	
Frpb, ped/bikes	1.00	1.00	0.97	1.00	1.00	0.96	1.00	0.99		1.00	0.99	
Flpb, ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00		1.00	1.00	
Ert	1.00	1.00	0.85	1.00	1.00	0.85	1.00	0.98		1.00	0.98	
Flt Protected	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00		0.95	1.00	
Satd. Flow (prot)	1662	1601	1339	1492	1617	1356	1536	2931		1646	3202	
Flt Permitted	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00		0.95	1.00	
Satd. Flow (perm)	1662	1601	1339	1492	1617	1356	1536	2931		1646	3202	
Peak-hour factor, PHF	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Adj. Flow (vph)	118	265	334	117	345	69	285	314	45	160	589	98
RTOR Reduction (vph)	0	0	56	0	0	52	0	9	0	0	11	0
Lane Group Flow (vph)	118	265	278	117	345	17	285	350	0	160	676	0
Confl. Peds. (#/hr)	8	200	18	18	0.10	8	5	000	10	10	0.0	5
Confl. Bikes (#/hr)			25			17	Ū		5			14
Heavy Vehicles (%)	0%	2%	1%	4%	1%	2%	1%	3%	5%	1%	1%	0%
Turn Type	Prot	NA	pm+ov	Prot	NA	Perm	Prot	NA		Prot	NA	
Protected Phases	5	2	3	1	6	T CITII	3	8		7	4	
Permitted Phases	3	_	2		U	6	3	U		,		
Actuated Green, G (s)	10.4	25.3	48.4	11.2	26.1	26.1	23.1	35.1		14.4	26.4	
Effective Green, g (s)	10.4	25.3	48.4	11.2	26.1	26.1	23.1	35.1		14.4	26.4	
Actuated g/C Ratio	0.10	0.24	0.47	0.11	0.25	0.25	0.22	0.34		0.14	0.25	
Clearance Time (s)	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5		4.5	4.5	
Vehicle Extension (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0		2.0	2.0	
Lane Grp Cap (vph)	166	389	681	160	405	340	341	989		227	812	
v/s Ratio Prot	0.07	0.17	0.09	c0.08	c0.21	340	c0.19	0.12		0.10	c0.21	
v/s Ratio Perm	0.07	0.17	0.07	CO.00	CU.Z I	0.01	CO. 17	0.12		0.10	CU.Z I	
v/c Ratio	0.71	0.68	0.12	0.73	0.85	0.05	0.84	0.35		0.70	0.83	
Uniform Delay, d1	45.3	35.7	18.3	44.9	37.1	29.6	38.6	25.9		42.8	36.7	
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00		1.00	1.00	
Incremental Delay, d2	11.3	3.9	0.1	13.7	15.2	0.0	15.4	0.1		7.9	7.0	
Delay (s)	56.6	39.6	18.5	58.7	52.3	29.6	54.1	26.0		50.6	43.7	
Level of Service	50.0 E	37.0 D	10.3 B	50.7 E	52.5 D	27.0 C	D D	20.0 C		50.0 D	73.7 D	
Approach Delay (s)		32.6	ь		50.8			38.4			45.0	
Approach LOS		32.0 C			D			D			73.0 D	
Intersection Summary							0 1					
HCM 2000 Control Delay			41.3	Н	ICM 2000	Level of	Service		D			
HCM 2000 Volume to Capac	any rano		0.83	_		(\			10.0			
Actuated Cycle Length (s)			104.0		um of los				18.0			
Intersection Capacity Utilizat	แบก		76.9%	10	CU Level	DI SELVICE	9		D			
Analysis Period (min)			15									
c Critical Lane Group												

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	7
Volume (vph)	360	5	3	5	9	16	2	230	1	28	300	613
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Total Lost time (s)		5.0			4.0			4.0			4.0	5.0
Lane Util. Factor		1.00			1.00			1.00			1.00	1.00
Frpb, ped/bikes		1.00			0.99			1.00			1.00	0.98
Flpb, ped/bikes		1.00			1.00			1.00			1.00	1.00
Frt		1.00			0.93			1.00			1.00	0.85
Flt Protected		0.95			0.99			1.00			1.00	1.00
Satd. Flow (prot)		1648			1537			1714			1726	1451
Flt Permitted		0.65			0.96			1.00			0.96	1.00
Satd. Flow (perm)		1123			1481			1710			1666	1451
Peak-hour factor, PHF	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Adj. Flow (vph)	379	5	3	5	9	17	2	242	1	29	316	645
RTOR Reduction (vph)	0	0	0	0	9	0	0	0	0	0	0	0
Lane Group Flow (vph)	0	387	0	0	22	0	0	245	0	0	345	645
Confl. Peds. (#/hr)	1		-	-		1	2		8	8		2
Confl. Bikes (#/hr)			4			2			2			4
Heavy Vehicles (%)	1%	0%	0%	0%	0%	6%	0%	2%	0%	0%	1%	1%
Turn Type	custom	NA		Perm	NA		Perm	NA		Perm	NA	custom
Protected Phases					8			2			6	
Permitted Phases	3	3		8			2			6		36
Actuated Green, G (s)		34.2			29.0			17.2			17.2	60.4
Effective Green, q (s)		34.2			29.0			17.2			17.2	56.4
Actuated g/C Ratio		0.57			0.48			0.28			0.28	0.93
Clearance Time (s)		5.0			4.0			4.0			4.0	
Vehicle Extension (s)		2.0			2.5			2.5			2.5	
Lane Grp Cap (vph)		635			711			486			474	1354
v/s Ratio Prot												
v/s Ratio Perm		c0.34			0.01			0.14			c0.21	0.44
v/c Ratio		0.61			0.03			0.50			0.73	0.48
Uniform Delay, d1		8.7			8.3			18.0			19.5	0.2
Progression Factor		1.00			1.00			1.00			1.00	1.00
Incremental Delay, d2		1.1			0.0			0.6			5.2	0.1
Delay (s)		9.8			8.3			18.6			24.7	0.3
Level of Service		A			Α			В			С	A
Approach Delay (s)		9.8			8.3			18.6			8.8	
Approach LOS		Α			Α			В			Α	
Intersection Summary												
HCM 2000 Control Delay			10.5	Н	CM 2000	Level of	Service		В			
HCM 2000 Volume to Cap	acity ratio		0.67		_ 000	_3.0.31	23,1100					
Actuated Cycle Length (s)	aony rano		60.4	ς	um of lost	time (s)			11.0			
Intersection Capacity Utiliz	ation		71.8%		U Level		,		C			
Analysis Period (min)			15	- 10	. C LOVOI (J. JOI VICE						
c Critical Lane Group			13									
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HCM Analysis – Future PM Peak

2: Willamette Street & 25th Avenue

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4	7		र्स	7		414	
Volume (vph)	15	50	15	280	85	50	5	480	215	50	735	20
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Total Lost time (s)		4.5			4.5	4.5		4.5	4.5		4.5	
Lane Util. Factor		1.00			1.00	1.00		1.00	1.00		0.95	
Frpb, ped/bikes		0.99			1.00	0.96		1.00	0.95		1.00	
Flpb, ped/bikes		1.00			0.99	1.00		1.00	1.00		1.00	
Frt		0.98			1.00	0.85		1.00	0.85		1.00	
Flt Protected		0.99			0.96	1.00		1.00	1.00		1.00	
Satd. Flow (prot)		1679			1673	1428		1732	1406		3268	
Flt Permitted		0.92			0.72	1.00		0.99	1.00		0.86	
Satd. Flow (perm)		1553			1257	1428		1720	1406		2817	
Peak-hour factor, PHF	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Adj. Flow (vph)	16	55	16	308	93	55	5	527	236	55	808	22
RTOR Reduction (vph)	0	10	0	0	0	33	0	0	122	0	2	0
Lane Group Flow (vph)	0	77	0	0	401	22	0	532	114	0	883	0
Confl. Peds. (#/hr)	7		6	6		7	11		6	6		11
Confl. Bikes (#/hr)			26			6			17			11
Heavy Vehicles (%)	0%	0%	0%	0%	1%	0%	0%	1%	1%	0%	1%	0%
Turn Type	Perm	NA		Perm	NA	Perm	Perm	NA	Perm	Perm	NA	
Protected Phases		2			6			8			4	
Permitted Phases	2			6		6	8		8	4		
Actuated Green, G (s)		29.8			29.8	29.8		36.2	36.2		36.2	
Effective Green, g (s)		29.8			29.8	29.8		36.2	36.2		36.2	
Actuated g/C Ratio		0.40			0.40	0.40		0.48	0.48		0.48	
Clearance Time (s)		4.5			4.5	4.5		4.5	4.5		4.5	
Vehicle Extension (s)		3.0			3.0	3.0		2.0	2.0		2.0	
Lane Grp Cap (vph)		617			499	567		830	678		1359	
v/s Ratio Prot												
v/s Ratio Perm		0.05			c0.32	0.02		0.31	0.08		c0.31	
v/c Ratio		0.13			0.80	0.04		0.64	0.17		0.65	
Uniform Delay, d1		14.3			20.0	13.8		14.5	10.9		14.6	
Progression Factor		1.00			1.00	1.00		0.51	0.13		1.00	
Incremental Delay, d2		0.1			9.1	0.0		3.6	0.5		2.4	
Delay (s)		14.4			29.1	13.9		11.0	2.0		17.0	
Level of Service		В			С	В		В	Α		В	
Approach Delay (s)		14.4			27.3			8.2			17.0	
Approach LOS		В			С			Α			В	
Intersection Summary												
HCM 2000 Control Delay			16.0	Н	CM 2000	Level of	Service		В			
HCM 2000 Volume to Capac	city ratio		0.72									
Actuated Cycle Length (s)			75.0		um of lost				9.0			
Intersection Capacity Utilizat	tion		91.7%	IC	U Level o	of Service			F			
Analysis Period (min)			15									
c Critical Lane Group												

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBF
Lane Configurations		4			4			414			414	
Volume (vph)	35	10	10	30	10	20	15	645	25	10	990	30
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Total Lost time (s)		4.5			4.5			4.5			4.5	
Lane Util. Factor		1.00			1.00			0.95			0.95	
Frpb, ped/bikes		1.00			0.99			1.00			1.00	
Flpb, ped/bikes		0.99			1.00			1.00			1.00	
Frt		0.98			0.95			0.99			1.00	
Flt Protected		0.97			0.98			1.00			1.00	
Satd. Flow (prot)		1592			1599			3267			3273	
Flt Permitted		0.82			0.86			0.92			0.95	
Satd. Flow (perm)		1349			1413			3023			3103	
Peak-hour factor, PHF	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Adj. Flow (vph)	38	11	11	32	11	22	16	694	27	11	1065	3
RTOR Reduction (vph)	0	8	0	0	17	0	0	3	0	0	2	(
Lane Group Flow (vph)	0	52	0	0	48	0	0	734	0	0	1106	(
Confl. Peds. (#/hr)	20		8	8		20	7		5	5		
Confl. Bikes (#/hr)			6			7			17			17
Heavy Vehicles (%)	3%	0%	0%	0%	0%	0%	0%	1%	0%	0%	1%	0%
Turn Type	Perm	NA	070	Perm	NA	0.0	Perm	NA	070	Perm	NA	07.
Protected Phases	1 Cilli	2		i ciiii	6		i ciiii	4		1 Cilli	8	
Permitted Phases	2			6	· ·		4			8	Ü	
Actuated Green, G (s)	_	18.0		U	18.0			48.0		U	48.0	
Effective Green, g (s)		18.0			18.0			48.0			48.0	
Actuated g/C Ratio		0.24			0.24			0.64			0.64	
Clearance Time (s)		4.5			4.5			4.5			4.5	
Vehicle Extension (s)		3.0			3.0			2.0			2.0	
Lane Grp Cap (vph)		323			339			1934			1985	
v/s Ratio Prot		323			337			1734			1703	
v/s Ratio Perm		c0.04			0.03			0.24			c0.36	
v/c Ratio		0.16			0.03			0.24			0.56	
Uniform Delay, d1		22.5			22.4			6.4			7.6	
Progression Factor		1.00			1.00			1.93			1.05	
Incremental Delay, d2		0.2			0.2			0.5			0.9	
Delay (s)		22.8			22.6			13.0			8.8	
Level of Service		22.0 C			22.0 C			13.0 B			0.0 A	
Approach Delay (s)		22.8			22.6			13.0			8.8	
		22.0 C			22.0 C			13.0 B			0.0 A	
Approach LOS		C			C			В			А	
Intersection Summary			44.0		014.0000		0 1		-			
HCM 2000 Control Delay			11.3	Н	CM 2000	Level of :	Service		В			
HCM 2000 Volume to Capa	city ratio		0.45	_		P / \			0.0			
Actuated Cycle Length (s)			75.0		um of lost				9.0			
Intersection Capacity Utiliza	tion		60.9%	IC	:U Level	of Service			В			
Analysis Period (min)			15									
c Critical Lane Group												

c Critical Lane Group

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	*	4		ň	4			4 1 >			ብጉ	
Volume (vph)	50	60	90	170	155	40	45	595	40	25	895	90
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Total Lost time (s)	4.5	4.5		4.5	4.5			4.5			4.5	
Lane Util. Factor	1.00	1.00		1.00	1.00			0.95			0.95	
Frpb, ped/bikes	1.00	0.98		1.00	0.99			1.00			1.00	
Flpb, ped/bikes	0.98	1.00		0.99	1.00			1.00			1.00	
Frt	1.00	0.91		1.00	0.97			0.99			0.99	
Flt Protected	0.95	1.00		0.95	1.00			1.00			1.00	
Satd. Flow (prot)	1621	1541		1623	1640			3211			3229	
Flt Permitted	0.56	1.00		0.64	1.00			0.84			0.93	
Satd. Flow (perm)	949	1541		1088	1640			2692			3002	
Peak-hour factor, PHF	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Adj. Flow (vph)	52	62	93	175	160	41	46	613	41	26	923	93
RTOR Reduction (vph)	0	65	0	0	15	0	0	5	0	0	8	0
Lane Group Flow (vph)	52	90	0	175	186	0	0	695	0	0	1034	0
Confl. Peds. (#/hr)	28		14	14		28	9		14	14		9
Confl. Bikes (#/hr)			7			4			15			13
Heavy Vehicles (%)	0%	3%	0%	1%	3%	0%	0%	2%	3%	5%	1%	0%
Turn Type	Perm	NA		Perm	NA		Perm	NA		Perm	NA	
Protected Phases		2			6			4			8	
Permitted Phases	2			6			4			8		
Actuated Green, G (s)	20.1	20.1		20.1	20.1			45.9			45.9	
Effective Green, g (s)	20.1	20.1		20.1	20.1			45.9			45.9	
Actuated g/C Ratio	0.27	0.27		0.27	0.27			0.61			0.61	
Clearance Time (s)	4.5	4.5		4.5	4.5			4.5			4.5	
Vehicle Extension (s)	3.0	3.0		3.0	3.0			2.0			2.0	
Lane Grp Cap (vph)	254	412		291	439			1647			1837	
v/s Ratio Prot		0.06			0.11							
v/s Ratio Perm	0.05			c0.16				0.26			c0.34	
v/c Ratio	0.20	0.22		0.60	0.42			0.42			0.56	
Uniform Delay, d1	21.3	21.3		24.0	22.7			7.6			8.6	
Progression Factor	1.00	1.00		1.00	1.00			1.00			0.66	
Incremental Delay, d2	0.4	0.3		3.5	0.7			0.8			1.1	
Delay (s)	21.7	21.6		27.4	23.3			8.4			6.8	
Level of Service	С	С		С	С			Α			Α	
Approach Delay (s)		21.6			25.2			8.4			6.8	
Approach LOS		С			С			Α			Α	
Intersection Summary												
HCM 2000 Control Delay			11.6	Н	CM 2000	Level of :	Service		В			
HCM 2000 Volume to Capa	city ratio		0.57									
Actuated Cycle Length (s)			75.0		um of lost				9.0			
Intersection Capacity Utiliza	ation		96.6%	IC	U Level o	of Service	;		F			
Analysis Period (min)			15									
c Critical Lane Group												

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Movement	EBL	EBR	NBL	NBT	SBT	SBR	
Lane Configurations	**	7		44	† 1>		
Volume (veh/h)	130	170	40	550	905	185	
Sign Control	Stop			Free	Free		
Grade	0%			0%	0%		
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97	
Hourly flow rate (vph)	134	175	41	567	933	191	
Pedestrians				29	3		
Lane Width (ft)				12.0	12.0		
Walking Speed (ft/s)				4.0	4.0		
Percent Blockage				2	0		
Right turn flare (veh)							
Median type				None	None		
Median storage veh)							
Upstream signal (ft)				341	673		
pX, platoon unblocked	0.93	0.89	0.89				
vC, conflicting volume	1397	591	1124				
vC1, stage 1 conf vol							
vC2, stage 2 conf vol							
vCu, unblocked vol	876	292	891				
tC, single (s)	6.8	6.9	4.2				
tC, 2 stage (s)							
tF (s)	3.5	3.3	2.2				
p0 queue free %	47	71	94				
cM capacity (veh/h)	253	614	667				
Direction, Lane #	EB 1	EB 2	NB 1	NB 2	SB 1	SB 2	
Volume Total	134	175	230	378	622	502	
Volume Left	134	0	41	0	0	0	
Volume Right	0	175	0	0	0	191	
cSH	253	614	667	1700	1700	1700	
Volume to Capacity	0.53	0.29	0.06	0.22	0.37	0.30	
Queue Length 95th (ft)	71	29	5	0	0	0	
Control Delay (s)	34.2	13.2	2.5	0.0	0.0	0.0	
Lane LOS	D	В	Α				
Approach Delay (s)	22.3		1.0		0.0		
Approach LOS	С						
Intersection Summary							
Average Delay			3.7				
Intersection Capacity Utilizati	ion		67.3%	IC	U Level o	of Service	
Analysis Period (min)			15				

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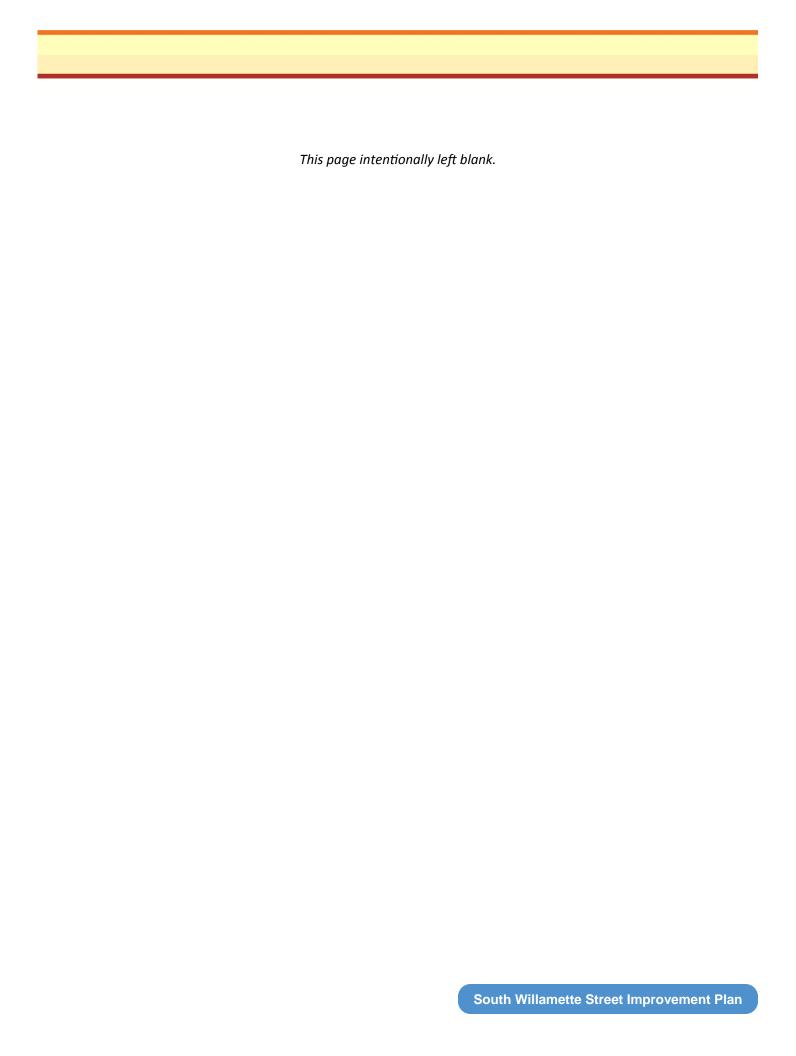
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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	*	^	7	*	↑	7	*	↑ ↑		*	∱ 1>	
Volume (vph)	140	350	330	140	480	100	290	350	55	210	720	135
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Lane Width	12	10	10	10	10	11	10	10	12	12	12	12
Total Lost time (s)	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5		4.5	4.5	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95		1.00	0.95	
Frpb, ped/bikes	1.00	1.00	0.97	1.00	1.00	0.96	1.00	0.99		1.00	0.99	
Flpb, ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00		1.00	1.00	
Frt	1.00	1.00	0.85	1.00	1.00	0.85	1.00	0.98		1.00	0.98	
Flt Protected	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00		0.95	1.00	
Satd. Flow (prot)	1662	1601	1331	1492	1617	1356	1536	2922		1646	3193	
Flt Permitted	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00		0.95	1.00	
Satd. Flow (perm)	1662	1601	1331	1492	1617	1356	1536	2922		1646	3193	
Peak-hour factor, PHF	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Adj. Flow (vph)	147	368	347	147	505	105	305	368	58	221	758	142
RTOR Reduction (vph)	0	0	347	0	0	67	0	10	0	0	13	0
Lane Group Flow (vph)	147	368	313	147	505	38	305	416	0	221	887	0
Confl. Peds. (#/hr)	8	300	18	18	303	8	5	410	10	10	007	5
. ,	0		25	10		17	Э		5	10		14
Confl. Bikes (#/hr)	00/	20/	25 1%	40/	10/		10/	20/		1%	1%	
Heavy Vehicles (%)	0%	2%		4%	1%	2%	1%	3%	5%			0%
Turn Type	Prot	NA	pm+ov	Prot	NA	Perm	Prot	NA		Prot	NA	
Protected Phases	5	2	3	1	6		3	8		7	4	
Permitted Phases	40.5	00 (2	40.5	05 (6	00.5	010		100	00.4	
Actuated Green, G (s)	10.5	33.6	56.1	12.5	35.6	35.6	22.5	36.0		19.9	33.4	
Effective Green, g (s)	10.5	33.6	56.1	12.5	35.6	35.6	22.5	36.0		19.9	33.4	
Actuated g/C Ratio	0.09	0.28	0.47	0.10	0.30	0.30	0.19	0.30		0.17	0.28	
Clearance Time (s)	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5		4.5	4.5	
Vehicle Extension (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0		2.0	2.0	
Lane Grp Cap (vph)	145	448	672	155	479	402	288	876		272	888	
v/s Ratio Prot	0.09	0.23	0.09	c0.10	c0.31		c0.20	c0.14		0.13	c0.28	
v/s Ratio Perm			0.15			0.03						
v/c Ratio	1.01	0.82	0.47	0.95	1.05	0.09	1.06	0.48		0.81	1.00	
Uniform Delay, d1	54.8	40.4	21.8	53.4	42.2	30.5	48.8	34.3		48.3	43.3	
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00		1.00	1.00	
Incremental Delay, d2	78.4	11.0	0.2	55.9	56.2	0.0	69.4	0.1		15.9	29.7	
Delay (s)	133.2	51.4	21.9	109.3	98.4	30.6	118.2	34.4		64.1	72.9	
Level of Service	F	D	С	F	F	С	F	С		E	E	
Approach Delay (s)		53.5			91.1			69.4			71.2	
Approach LOS		D			F			Ε			Е	
Intersection Summary												
HCM 2000 Control Delay			70.8	Н	CM 2000	Level of	Service		E			
HCM 2000 Volume to Capa	city ratio		1.04									
Actuated Cycle Length (s)			120.0	S	um of lost	time (s)			18.0			
Intersection Capacity Utiliza	tion		94.7%	IC	CU Level	of Service	;		F			
Analysis Period (min)			15									
c Critical Lane Group												

	٠	→	•	•	-	•	•	†	<i>></i>	\	↓	4
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	7
Volume (vph)	390	10	5	10	10	20	5	285	5	30	440	635
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Total Lost time (s)		5.0			4.0			4.0			4.0	5.0
Lane Util. Factor		1.00			1.00			1.00			1.00	1.00
Frpb, ped/bikes		1.00			0.99			1.00			1.00	0.98
Flpb, ped/bikes		1.00			1.00			1.00			1.00	1.00
Frt		1.00			0.93			1.00			1.00	0.85
Flt Protected		0.95			0.99			1.00			1.00	1.00
Satd. Flow (prot)		1648			1551			1711			1727	1450
Flt Permitted		0.65			0.91			0.98			0.97	1.00
Satd. Flow (perm)		1127			1429			1686			1679	1450
Peak-hour factor, PHF	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Adj. Flow (vph)	411	11	5	11	11	21	5	300	5	32	463	668
RTOR Reduction (vph)	0	0	0	0	11	0	0	1	0	0	0	0
Lane Group Flow (vph)	0	427	0	0	32	0	0	309	0	0	495	668
Confl. Peds. (#/hr)	1					1	2		8	8		2
Confl. Bikes (#/hr)			4			2			2			4
Heavy Vehicles (%)	1%	0%	0%	0%	0%	6%	0%	2%	0%	0%	1%	1%
Turn Type	custom	NA		Perm	NA		Perm	NA		Perm	NA	custom
Protected Phases					8			2			6	
Permitted Phases	3	3		8			2			6		36
Actuated Green, G (s)		35.1			29.7			20.0			20.0	64.1
Effective Green, g (s)		35.1			29.7			20.0			20.0	60.1
Actuated g/C Ratio		0.55			0.46			0.31			0.31	0.94
Clearance Time (s)		5.0			4.0			4.0			4.0	
Vehicle Extension (s)		2.0			2.5			2.5			2.5	
Lane Grp Cap (vph)		617			662			526			523	1359
v/s Ratio Prot												
v/s Ratio Perm		c0.38			0.02			0.18			c0.29	0.46
v/c Ratio		0.69			0.05			0.59			0.95	0.49
Uniform Delay, d1		10.6			9.4			18.6			21.5	0.2
Progression Factor		1.00			1.00			1.00			1.00	1.00
Incremental Delay, d2		2.7			0.0			1.4			26.3	0.1
Delay (s)		13.3			9.5			20.0			47.8	0.3
Level of Service		В			Α			В			D	Α
Approach Delay (s)		13.3			9.5			20.0			20.5	
Approach LOS		В			Α			В			С	
Intersection Summary												
HCM 2000 Control Delay			18.6	Н	CM 2000	Level of	Service		В			
HCM 2000 Volume to Capa	acity ratio		0.81									
Actuated Cycle Length (s)			64.1		um of lost				11.0			
Intersection Capacity Utiliza	ation		81.8%	IC	CU Level	of Service)		D			
Analysis Period (min)			15									

c Critical Lane Group

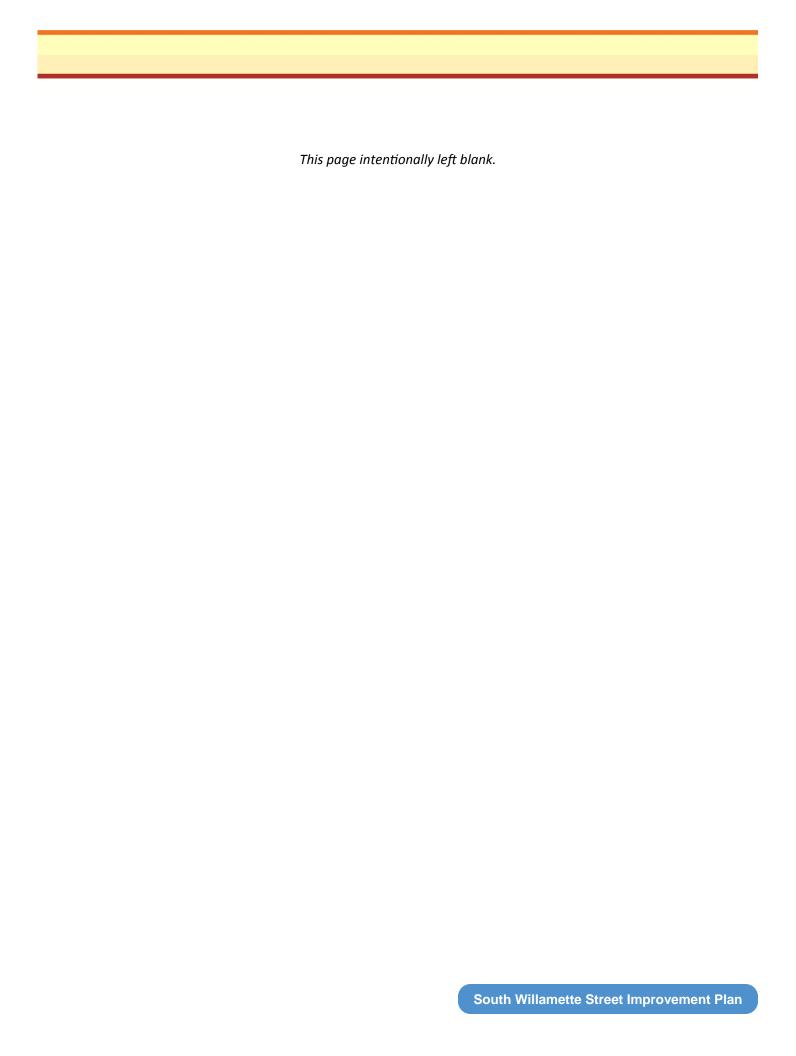
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SECTION C

ASSESMENT OF PREVIOUS TRANSPORTATION PLANNING EFFORTS



TECHNICAL MEMORANDUM #3



117 Commercial Street NE Suite 310 Salem, OR 97301 503.391.8773 www.dksassociates.com

DATE: November 1, 2012

TO: Project Management Team

FROM: Scott Mansur, P.E., PTOE

Brad Coy, P.E. Derek Moore, E.I.T

SUBJECT: Assessment of Previous Planning Efforts

P10086-012

This memorandum summarizes key elements from previous planning efforts related to the configuration of Willamette Street in the project area. This memorandum will be used as a resource in the preparation of the Willamette Street Improvement Plan to help ensure that the plan builds off of past effort, addresses outstanding issues, and fits into the larger regional context. The following documents have been reviewed and included in the summary:

- South Willamette Area Draft Concept Plan
- Draft City of Eugene Transportation System Plan
- Eugene Pedestrian and Bicycle Master Plan
- Walkable Community Workshops Summary Report
- Willamette Street Traffic Analysis Report

This memorandum begins with a description of how Willamette Street fits into the regional context, followed by summaries of applicable standards and previously identified design alternatives.

South Willamette Area Draft Concept Plan

The South Willamette Draft Concept Plan provides high-level guidance and vision on how development in the area should progress. The concept plan concentrates on residential and shopping areas surrounding Willamette Street that are bound by 24th Street to the north, 32nd Street to the south, Portland Street to the west, and Amazon Parkway to the east. It focuses on promoting business success in an urban district while supporting walking, biking, and driving.

A key concept identified in the plan is developing the "Heart of the Walkable Business District," which is characterized by a "Safe, Attractive Pedestrian Experience for Business, Shopping and Entertainment." The portion of Willamette Street extending from 24th Place to 27th Avenue is identified as part of this district. Additional segments (Oak Street between 28th and 29th Avenues, 24th Avenue to 24th Place, 27th- 29th Avenue and the alley between Oak and Willamette, from 27th to 28th Avenues) are identified as needing enhancements to the existing pedestrian experience. The Concept Plan identifies the potential for a pedestrian walkway across Willamette Street located between 27th Avenue and 29th Avenue. It also identifies gateways into the district located at the Willamette Street/23rd Avenue and Willamette Street/31st Avenue intersections.



The plan also recommends the establishment of shared parking facilities to support the commercial district.

Eugene Arterial and Collector Street Plan (ACSP)

The primary purpose of the Eugene ACSP (adopted 1999) is to provide an updated street classification map and the appropriate street design standards and guidelines. The ACSP recognizes that streets of similar classification may have very different characteristics, therefore rather than providing specific thresholds, a more general priority level is provided to help guide decision making related to street improvements. Table 1 provides a summary of the priorities for improvement or regulation relevant to Willamette Street (minor arterial).

Table 1: Priority of Improvement or Regulation for Minor Arterials

Improvement Type	Priority
Regulate Access	High
<u> </u>	•
Traffic Calming	Medium
Adding Sidewalks	High
Adding Bike Lanes	High
Upgrade Urban Standards	High
Major Corridor Improvements	Medium
New Street Mileage	Low

As shown, the highest priorities are identified to be regulating access, adding sidewalks and bike lanes, and upgrading urban standards. Regarding access management, the ACSP goes on to say "attempts should be made, wherever possible, to consolidate multiple driveways on arterial streets into a single access point." The City has also adopted access management standards within the City of Eugene Code (7.408) that are intended to:

- A. Balance the need for a safe and efficient roadway system against the need to provide ingress and egress to developed land adjacent to the street.
- B. Reduce conflict points in the transportation system by managing the number, spacing, location and design of access connections.
- C. Preserve intersection influence areas to allow drivers to focus on traffic operational tasks, weaving, speed changes, traffic signal indications, etc.
- D. Reduce interference with through movement, caused by slower vehicles exiting, entering or turning across the roadway, by providing turning lanes or tapers and restricting certain movements.

The City Code also provides direction on access spacing standards that are dependent upon the roadway classification and influence to adjacent intersections.

Draft City of Eugene Transportation System Plan (TSP)

The City's Draft TSP¹ identifies this portion of Willamette Street as a minor arterial, which should comply with the following standards (obtained from the Eugene ACSP):

- Right-of-way (ROW) widths from 65' to 100'
- Minimum 11' travel lanes

¹ Eugene Transportation System Plan: Existing Conditions and Deficiencies, March 2011

Assesment of Previous Planning Efforts November 1, 2012 Page 3 of 5



- Continuous sidewalks on both sides of street and set back from curb.
- Minimum sidewalk widths of 10' for curbside sidewalks, and 5' for setback sidewalks
- Bicycle lanes should be striped 6' (standard) or 5' (in constrained situations) and free from drainage grates and utility covers

The TSP specifies a minimum performance of Level of Service (LOS) "D" for signalized intersections. Within the Central Area Transportation Study Area Boundary (primarily downtown and near the University), the city allows LOS "E". The intersection of Willamette Street and 29th Avenue was evaluated when the TSP was developed, and was found to be operating at a LOS C. A crash rate of 1.40 crashes/million entering vehicles was reported, however only 24% of the crashes occurred within the intersection, meaning that a high number of crashes could be associated with nearby driveways. Additionally, the TSP identifies an overrepresentation of turning crashes, which represented 53% of all crashes at this intersection.

The TSP identifies potential projects to address recognized needs and deficiencies; however it does not recommend specific projects at this time. It identified as a potential project, a bike and pedestrian transformation between 24th Avenue and 32nd Avenue. As findings from this study are made available, they will be used to help inform the TSP update.

Eugene Pedestrian and Bicycle Master Plan

The Eugene Pedestrian and Bicycle Master Plan (PBMP) identifies existing conditions and needed improvements to the bicycle and pedestrian facilities. In developing this plan, three corridors were selected for a more detailed level of feasibility analysis, one of which was Willamette Street from 18th Avenue to 32nd Avenue. As a companion document to the PBMP, a technical memorandum², documents the recommended improvements as well as several alternatives.

The current roadway configuration does not include bike lanes, and the desired improvement along this corridor is to provide wider sidewalks and 5-6' bike lanes (6' is preferred). Several potential solutions are presented, some of which maintain existing curb-to-curb widths, and some require utilization of additional available ROW to widen the road. The potential reconfigurations specific to Willamette Street between 24th Avenue and 32nd Avenue are as follows:

Option 1: Meet Design Standards (recommended)

- From 32nd Avenue to approaching the 29th Avenue intersection the width would be 65' including three 11' lanes (1 northbound, 2 southbound), two 6' bike lanes, and 10' sidewalks on each side.
- Approaching 29th Avenue from the south and leaving 29th Avenue north the roadway would be 87' including five 11' lanes (1 center turn lane each direction), 6' bike lanes, and 10' sidewalks.
- Leaving 29th Avenue to 24th Avenue the width would be 76' including four 11' lanes, 6' bike lanes, and 10' sidewalks. The fourth lane is dropped in the northbound direction.

This option provides standard width pedestrian/bicycle facilities, but requires significant road widening, potential impacts to properties and structures, and is anticipated to have a high cost.

² Eugene Pedestrian and Bicycle Master Plan Road Reconfiguration Assessment, May 2011



Option 2: Two Travel Lanes and Center Turn Lane

• This option reduces the sidewalks widths from 10' to 6' allowing for maximum curb-to-curb width without widening outside of the existing 60' ROW. The cross section would include three 11' lanes (1 northbound, 1 southbound, and 1 center turn lane), two 6' bike lanes, and 6' curbside sidewalks. The section would be reconfigured to a left turn lane, through lane, and right turn lane on both the northbound and southbound approaches to 29th Avenue. This option provides standard width bicycle facilities and adds a center turn lane without requiring additional ROW. However, it reduces vehicular capacity by eliminating travel lanes.

Option 3: Three Travel Lanes, No Widening

• This option reduces the cross section from four lanes to three so that bike lanes can be additional curb-to-curb width. It would include three 10' lanes (1 northbound, 2 southbound), 5' bike lanes, and would maintain existing curbs and sidewalks. The cross section would be widened near 29th Avenue to allow for the addition of left turn lanes. This option provides bike facilities without widening the roadway, however vehicle capacity is reduced and there are no improvements to pedestrian facilities.

Option 4: Two Travel Lanes, No Center Turn Lane

This option would reduce the cross section to two 14' lanes (one in each direction), 6' bike lanes, and 10 sidewalks. Approaching 29th Avenue, the configuration would include a left turn lane, through lane, and bike lane in each direction. This option provides standard bicycle facilities and wider sidewalks without requiring additional ROW. However, it also has the highest reduction in vehicle capacity.

Walkable Community Workshops

In 2004, a series of interactive workshops were held with community members to identify and propose solutions to concerns about walkability. ³ One workshop focused on Willamette Street between 24th Avenue and 29th Avenue and the surrounding neighborhood. Four small groups discussed potential solutions after walking around the area. Many ideas were documented and a few identified by multiple groups are reiterated here (not a comprehensive list):

- Convert Willamette Street from its exiting four-lane configuration to a three-lane configuration with a center turn lane, bike lanes, and pedestrian refuge medians.
- Create bus pullouts at all stops to prevent buses from blocking traffic.
- Reduce the number of curb cuts and driveways wherever possible.
- Make pedestrian crossing of Willamette Street easier with refuge medians at key locations.
- Add landscaped medians for improved aesthetics.
- Move utilities underground or to alleyways for improved aesthetics and pedestrian circulation.

The summary report contains many additional ideas generated by the small groups. It also identified improved access management and a comprehensive look at traffic circulation in a broader area around Willamette Street as necessary steps to be taken before enhancements can be implemented.

³ Walkable Community Workshop Summary Report, May 2004

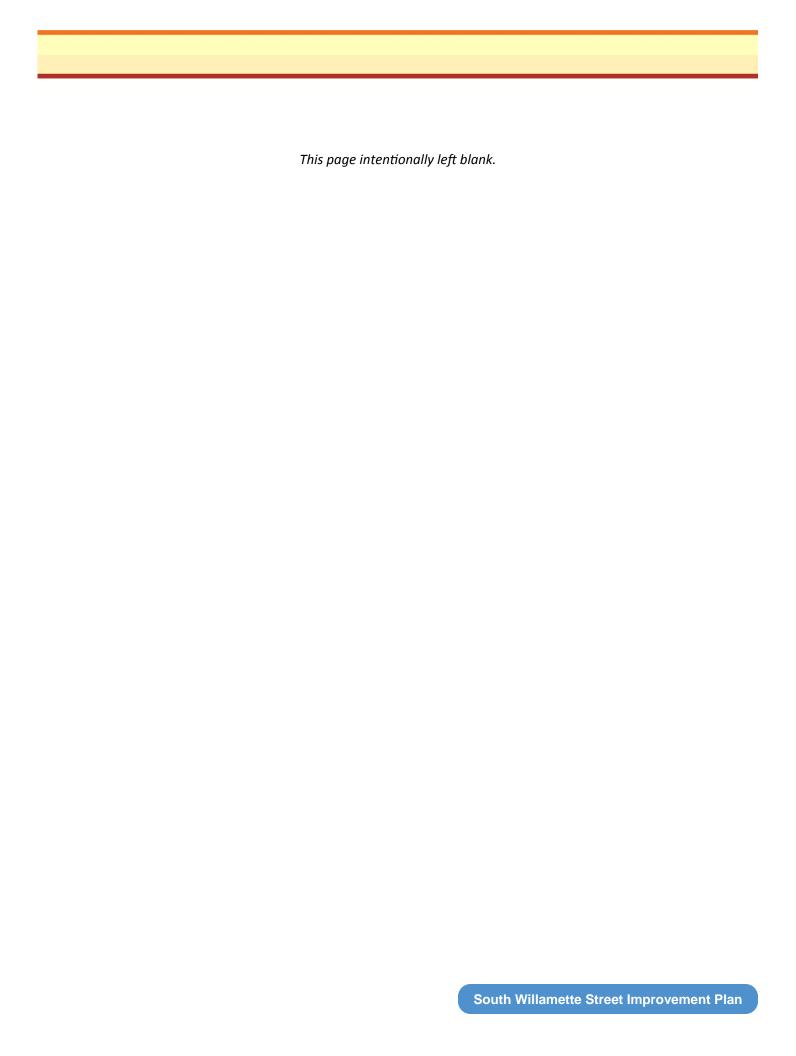
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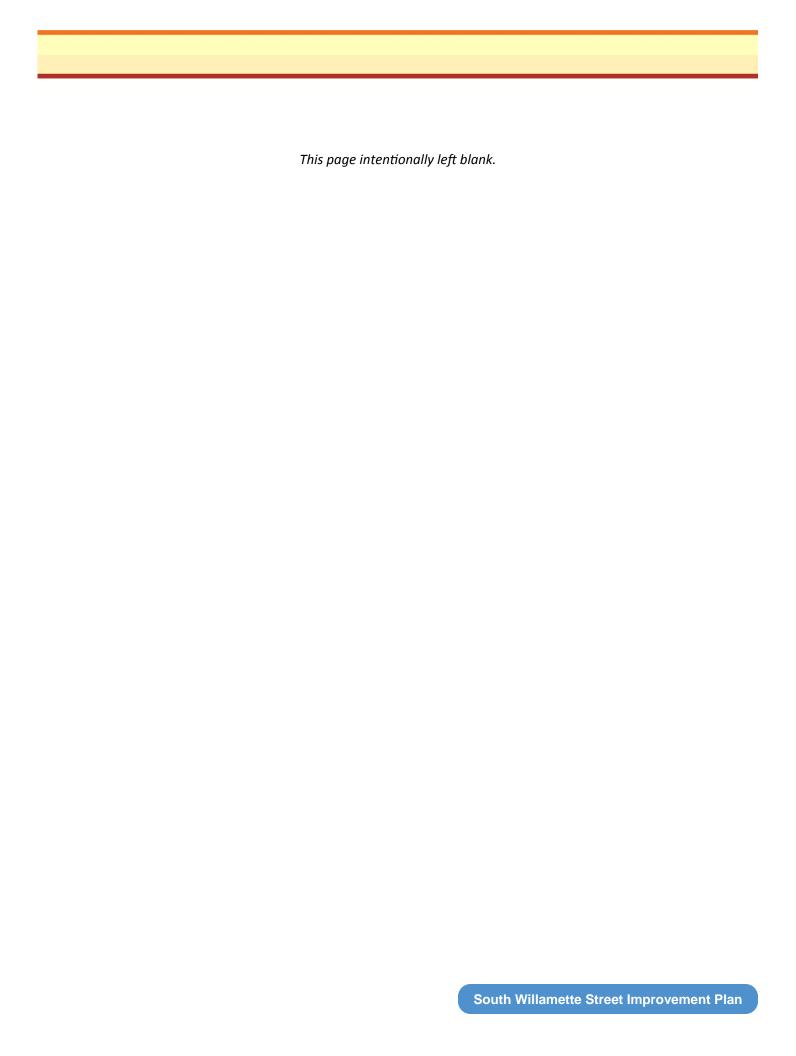
Willamette Street Traffic Analysis

A traffic analysis⁴ was conducted in 2001 to evaluate alternative designs for the section of Willamette Street between 24th and 29th Avenues. It was directed at improving pedestrian access while maintaining traffic capacity and safety. The recommended alternative involved re-striping Willamette Street to a three-lane section with a center turn lane, bicycle lanes, and pedestrian refuges at strategic points. The analysis also evaluated a variable three/four-lane section with pedestrian refuges, as well as traffic signal options (full signal vs. mid-block pedestrian signal) at or near the Willamette Street/25th Avenue intersection.

⁴ Willamette Street Traffic Analysis, McKenney Engineering, June 2001



PUBLIC INVOLVEMENT PLAN





South Willamette Street Improvement Plan June 2012- June 2013

PUBLIC INVOLVEMENT PLAN

INTRODUCTION

Guiding Principles

- Respect the intelligence of the public
- Seek out and facilitate the involvement of those potentially affected
- Identify issues and concerns early and throughout the process
- Widely disseminate complete information in a timely manner
- Include the public's contribution in decisions
- Report how input was considered & reasons for decisions in each phase
- Encourage open and honest communication

Goals

- Broad participation
- Confidence in process integrity
- Timely, authentic & useful public input
- Thoughtful responses to individual comments, concerns, questions
- Public information on city policies, such as the 20-minute neighborhood

Decisions and Roles

A Project Management Team that includes the City of Eugene, Oregon Department of Transportation and DKS Associates consulting team will guide this 13-month long project. The Project Management Team will gather input at four critical junctures from the Technical Advisory Committee, a series of stakeholder conversations, and public meetings to help formulate plans and give feedback as they evolve to a preferred alternative. It will then move to Eugene's Planning Commission and City Council for approval.

KEY ELEMENTS OF PLAN

- Stakeholder Outreach Meetings at four junctures
- Focus Groups on two key perspectives: Businesses & Users
- Community Meetings at three points: Alternatives, Evaluation, & Refinement
- Direct Outreach (street displays, speaking with organizations)
- Meetings with Planning Commission & City Council

Stakeholder Outreach Meetings (Four times at specific project steps)

Identified individuals, who are directly impacted or represent key issues and regularly communicate with their constituencies, will be consulted throughout the project, and function as advisors with an ear to the ground. Developed in collaboration with the City, an inclusive analysis of stakeholders will be the basis for this dynamic stakeholders' list.

#1 Stakeholder Outreach Meetings August- up to 6 small groups):

- Establish positive relationship
- Explain project overview and parameters
- Ensure the public involvement plan will work for them
- Learn how to communicate with their constituencies (i.e. newsletters, websites, guest speaker, high volume events)
- Collect input on existing conditions, problem statement, goals, and evaluation criteria
- Listen for other ideas and alternatives to be considered
- Identify any "hot button" issues and highly concerned individuals

#2 Stakeholder Outreach Meeting (January 2013- up to 4 small groups):

- Collect input on results of Tier 1 Screening analysis
- Listen for concerns, questions, new information
- Get information to those with constituencies in formats to easily shared

#3 Stakeholder Outreach Meeting (May 2013- up to 4 small groups):

- Collect input on results to Tier 2 Screening analysis
- Listen for concerns, questions, new information
- Get information to those with constituencies in formats easily shared

#4 Stakeholder Outreach Meeting (June 2013- up to 4 small groups):

- Collect input on Draft Plan
- Listen for concerns, questions, new information
- Get information to those with constituencies in formats easily shared

Two Focus Groups (September 2012)

Focus Groups tap the insight and knowledge of local experts and advocates to identify creative solutions to core challenges and provide a venue for explaining limitations and tradeoffs in a small setting. Consultants can get feedback on assumptions and help on challenges. Participants will learn more about the study area and expand the pool of people who can be articulate about the complexities, and contribute to setting realistic project goals and parameters.

Focus groups were organized by constituency, such as (1) Corridor Users-- bicycle, pedestrian, bus and car commuters and (2) Businesses & Property Owners.

The Focus Groups, ranging from twelve to twenty participants, were held in a group discussion format and included the wisdom and experiences of:

Bicyclists, Pedestrians and Transit Users

- Owners, customers, and employees of area businesses
- Residents who live in or near the Project Study Area
- Commuters living to the south, who use Willamette Street
- People with physical disabilities
- Seniors

All Focus Groups invitees will receive a project update that shows how their input affected the outcome to date. Why did some ideas move forward, while others seem to have dropped off the table? We will also invite their ongoing participation and ask them to help us to expand the public engagement.

Public Events

Three large community meetings will build on the knowledge gained from the Stakeholder Meetings and Focus Groups. Well advertised, meetings will have an engaging agenda, pleasant atmosphere, and amenities designed to maximize attendance and diversity, such as food and accommodations for people with disabilities. Public events also provide an opportunity to address the broader community through direct "street" outreach with displays prior to the event and media coverage of the event. The website offers an additional way to receive public input. Meeting Summaries will be emailed to all who attend and posted on the project website.

#1 Community Forum: Explore the Alternatives (November 2012)

This first meeting will introduce the project to the broader community and explain the role of the three public meetings in developing the preferred alternative. Public input will be collected on the existing conditions, deficiencies and needs, potential improvements and evaluation criteria. Ideas for improvements will be welcomed and evaluated.

#2 Community Forum: Evaluate the Alternatives (February 2013)

The second meeting will welcome new participants and recap the project. Three to six alternatives will be presented in light of how they meet Tier 1 evaluation criteria. This event will collect input to help narrow which two to three alternatives advance to Tier 2 screening.

#3 Community Forum: Refine the Preferred Alternative (May 2013)

The third meeting will present the results of Tier 2 evaluation, giving more information about the remaining alternatives for public consideration. The participants will be asked to give input in regards to a preferred alternative.

Planning Commission & City Council

The project team will keep the Planning Commission and City Council informed, and will meet to review alternatives with them prior to the second public event, and again in the formal process of adoption during the summer and fall of 2013.

Website

The City will develop a webpage that is interesting, visually pleasing, and easy to navigate and understand. It will have a unique project masthead and include key project information, including a brief overview, meeting dates, other public involvement opportunities, and a library of technical memos and public involvement summaries. The website will provide the opportunity for public comments and questions, and will be regularly monitored.

Interested Parties List

Cogito will develop a comprehensive Interested Parties List that identifies individuals based on their connection to the project. All interested parties will receive project information and invitations to the Community Forums. The project will use the City's existing list, and connect to organizations' lists with interest in the project. Additionally, Cogito will collect contact information from those we meet during "street" outreach with display boards.

Display Outreach

As a method of outreach prior to each Community Forum, Cogito will staff display boards where there is high-volume foot traffic. This expands the base of who attends the meeting, shares accurate information with those who do not attend, provides the project with a broader view of public concerns and sentiments, and ensures that the public involvement results are representative. Display board graphics are also used at the Community Forums, speaking engagements, and provide the high quality graphics required for media.

Media

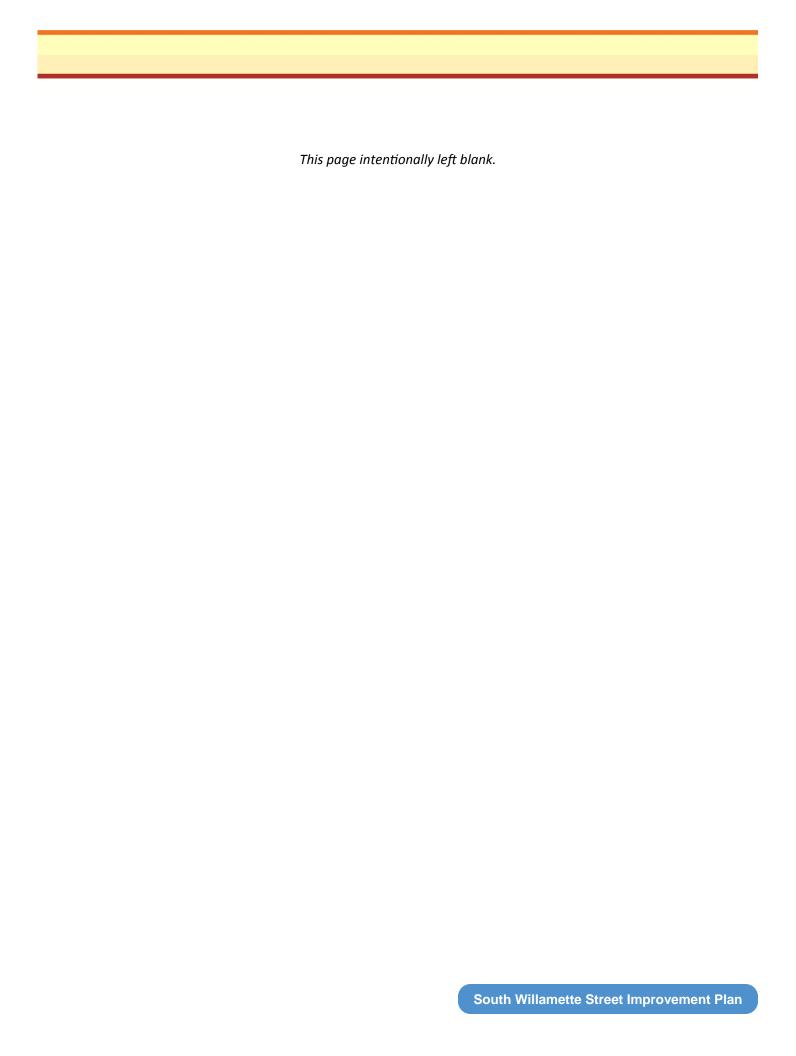
While there is not a budget for media work for this project, the city will use media to advertise public events and seek to gain media coverage of public events to reach a broader audience. To the degree the project generates discussion in the media, the project will monitor and respond with accurate information, if necessary.

Public Comments

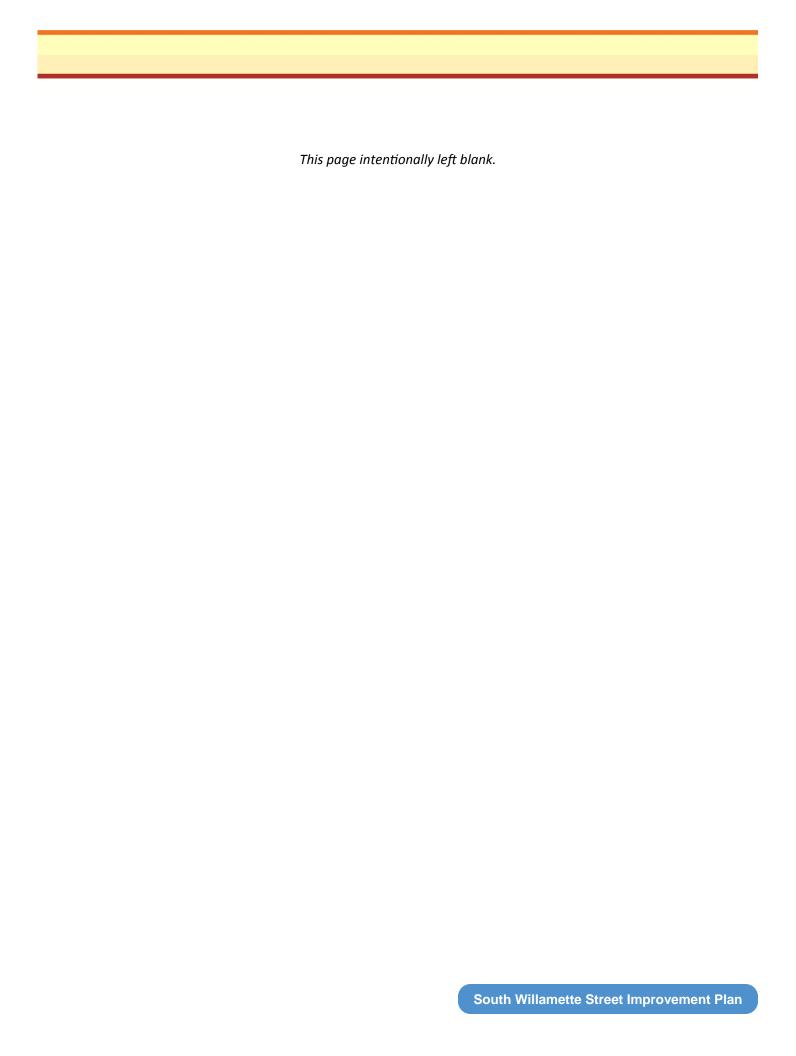
Calls and emails about the project will be received by the city and routed to the appropriate staff person to answer the question or respond to the comment. This is critical, as thoughtful and timely responses to vocal advocates earns trust and credibility. The city will catalogue all comments and responses, and share with the project team in a timely manner for consideration.

Level of Public Involvement

According to the ODOT Public Involvement Resource Guide, the South Willamette Street Improvement Plan ranks "Tier 3": Complex repair, safety, replacement or modernization scope of work. Public involvement for this project will be comprehensive, ongoing, and target a variety of key stakeholders. According to the principles of International Association for Public Participation (IAP2) Spectrum of Public Involvement, the process will range from "inform" to "collaborate." We will also use the Hans & Anne Marie Bleiker Strategic Development of Informed Consent (SDIC) method to check-in with identified stakeholders.



SECTION E COMMUNITY FORUM #1 SUMMARY





Meeting Summary Community Forum #1: Explore the Alternatives

Tuesday, November 13, 6-8 pm, Roosevelt Middle School Cafeteria, Eugene

Overview

The goal of the first of three community forums was to share project goals, report the results of recent traffic studies, explore alternatives, and listen to community priorities for future improvements to the design and condition of Willamette Street between 24th and 32nd avenues. The study aims to help South Willamette Street become a vibrant urban corridor accessible by bicycle, foot, car, and bus. The area includes residential, commercial, and mixed uses, and has six intersections being studied over about a ¾ mile stretch.

Community participation was robust! Approximately one hundred and fifty people filled the room to capacity and respectfully engaged with staff and consultants for two hours. Many perspectives were aired at the meeting, and geographic representation was balanced. Cyclists represented about 30% of all participants, and the crowd was primarily middle aged. After the Register Guard erroneously reported that the forum would be held on Wednesday, November 14, City staff quickly organized a second meeting, and this report includes the results of both events.

Participants overwhelmingly agreed that Willamette Street is a stressful experience for all modes of travel. Adding bike lanes, improving pedestrian crossings, and enhancing sidewalks were key priorities for participants. When participants were asked a specific question about improving bicycle facilities, bike lanes on Willamette Street was the preferred option of the majority. However, participants also questioned the impacts of reducing travel lanes in order to add bike lanes. Individuals who use the corridor to commute to work and school expressed a clear desire for the street to continue to move automobile traffic efficiently. Merchants located on Willamette Street stressed that they need current traffic volumes to maintain their businesses. Additionally, there was near unanimous support for undergrounding utilities, careful landscaping to beautify and to improve stormwater problems, and consolidating some of the corridor's more than seventy driveways. The idea of slowing car traffic to the speed limit was acceptable to almost all attendees.

Electronic polling, or "clickers," were used in the meeting, and are not meant to provide scientific data. Clickers ensure that everyone's opinion is heard in large time-limited meetings, and create transparency, since everyone sees response results immediately. This allows the facilitator to focus and draw out the various points of view for discussion. Also the first questions asked allowed everyone to quickly understand the representation in the room.

"The staff is here to hear what 'fix it' means to you and wants to find a solution that fits most," said Chris Henry, City of Eugene Project Manager. Ellen Teninty, who facilitated the meeting, discussed that the next steps in this project will be to have two more Community Forums – one likely in February and one likely in May- to first evaluate the alternatives and then refine the preferred alternatives. These will help inform recommendations to the Eugene Planning

Commission and City Council in the fall of 2013. For more information, or to view the slideshow presentation, please visit the project website at http://www.eugene-or.gov/SWillametteStreet.

Detailed Summary

Ellen Teninty asked the audience a couple of hand-raise polling questions to help audience members understand who is in the room. The conclusion was that there were more men than women present, and the audience was overwhelmingly over the age of 34. Then the audience used clickers to participate in a group poll of the following three questions. Due to audience size, some participants did not get clickers and instead filled out a paper survey. Results of the poll were automatically displayed for all to see on a large screen. (Note: Percentages do not equal 100% because some people did not vote).

- 1. Willamette Street between 24th & 32nd Avenues has some problems.
 - A. Agree: 90% B. Disagree: 4% C. Not sure: 5%
- 2. How do you <u>usually</u> travel on Willamette Street?
 - A. Walk or mobility device (wheelchair, scooter): 20%
 - B. Bus: 1%C. Bike: 29%D. Car: 48%E. Other: 1%
- 3. Where is your neighborhood?
 - A. South of 32nd Ave towards Spencer's Butte: 19%
 - B. West of Willamette Street towards Friendly Street: 34%
 - C. East of Willamette Street towards Amazon Parkway: 31%
 - D. Elsewhere: 15%

Project consultants Scott Mansur of DKS Associates and Tom Litster of OTAK presented information about existing conditions and design elements; visit http://www.eugene-or.gov/SWillametteStreet to view the presentation. Questions and comments from the audience followed:

Audience member: Will this project address sidewalk issues near the baseball stadium? Response: That is north of the project area, however it is part of a separate paving project also currently in planning.

Audience member: I believe there might be a lot of people not represented here, especially drivers, and I have concerns about equitable representation.

Audience member asked questions about counts of automobile traffic and freight in the presentation, airing concern that bicycles and pedestrians were not counted.

Response: We did count bicyclists and pedestrians however we don't have 24-hour counts. Freight counts provide a typical measure for pavement design (or thickness).

Audience member describes concern over drainage deficiencies that affect the sidewalk usability.

Response: The drainage would be fixed as part of paving project improvements regardless.

Audience member: Is there a safe way to have cyclists use roundabouts?

Response: Some ways were described.

Other audience members expressed concern about these methods and the safety of roundabouts for cyclists.

Audience member: I would love to see streets with a "sharrow" on them however there isn't anywhere for it to link up and sidewalks are dangerous to ride on.

Audience member: In a recent survey we conducted at the Market of Choice on Willamette Street, a lot of people said they drove, but wished they biked.

Audience member: When talking about the roundabout possibility with bikes, what would that look like?

Response: It can be shared use, or separate paths.

Audience member: With a lot of right of way, we could consider some alternative stormwater drainage.

Audience member: The bus works well for people. I think that Willamette Street gets clogged up with commuters and they should use another route.

Audience member: It would be great to have undergrounded utilities. *Some clapping of approval is heard from the audience.

Response: We will be asking EWEB what it would take to underground the utilities on Willamette Street.

Audience member: Do we know where the traffic is destined? It seems important to know. Response: The planning team had to scale back on the scope and remove destination research from this project. We are relying upon other methods described in the presentation.

Audience member: Asks a question regarding the project deadline.

Response: The next five years is the deadline for the project because it needs to meet the bond timing.

Audience member: More stop signs and a reduction in speed limits might increase the number of people using the Amazon Parkway.

Audience member: The bus doesn't loop from Willamette Street through the neighborhood. I would take the bus if that were the case. *Some clapping in agreement is heard from the audience.

Audience member: Have you thought of using bioswales?

Response: Yes and this is a consideration.

Audience member: A comment about a preference for improvement to turn lanes on 29th Ave.

Audience member: I would like to see bicycle counts on the bike ways paralleling Willamette to better understand how cyclists connect to Willamette Street.

Response: Our partners at LCOG might have that information for us. This project may need more funding to work on bicycle and pedestrian improvement.

Audience member: Why does the City Council need to approve this project for it to move forward?

Response: As a capital improvement project, it needs to be approved by Council.

Community members used clickers to answer two more questions:

- 4. What's your #1 priority for improving this section of Willamette Street?
 - A. Make bus stop improvements: 2%
 - B. Add bike lanes: 47%
 - C. Improve pedestrian crossings: 23%
 - D. Have better sidewalks: 29%
 - E. Improve traffic flow: 18%

If not one of these, then write your #1 priority here:

- Complete streets
- Find an alternate route for through traffic
- Improve bike infrastructure, not necessarily lanes
- Improve bus service to the area. Today there are no local buses to and from nearby neighborhoods
- Make bike lanes at least 6' wide
- Plan that includes all
- Reduce speeding
- Safety for all modes, slower speed
- Safety! It is hard to see bikes from car
- Stormwater management
- Streetside housing, trees
- 5. What's your #2 priority for improving this section of Willamette Street?
 - A. Make bus stop improvements: 4%
 - B. Add bike lanes: 24%
 - C. Improve pedestrian crossings: 23%
 - D. Have better sidewalks: 29%
 - E. Improve traffic flow: 11%

If not one of these, then write your #2 priority here:

- Find an alternate route for through traffic
- Traffic law enforcement
- Improve cohesiveness/boundaries of neighborhood
- Streetside housing, trees

Responses from audience members who selected having better sidewalks as one of their top priorities:

Audience member: I walk a lot on Willamette Street and the utility poles and other obstructions are in the way.

Audience member: The cars coming from the street into parking lots are under a lot of pressure to get off the street and it makes it dangerous for pedestrians.

Audience member: Sidewalk in front of Woodfield Station does not have room for pedestrians because cars have to pull all the way out to see traffic and get their opportunity to pull out.

Audience member: We should be able to park in one area —whether you bike or car- and walk to other destinations. People want to visit more than one place on Willamette Street.

Audience member: Can we get grant funding for improvements to the private way as well as the public way?

Response: Some longer-term planning can provide for public-private alternatives and improvements that require redevelopment.

Audience member: The sidewalks need a buffer between the traffic and pedestrians.

Audience member: I've been drenched walking on the sidewalk by cars driving by. Also automobile side mirrors are dangerous to pedestrians on Willamette Street.

Responses from audience members who selected improving traffic flow as one of their top priorities:

Audience member: Traffic stress makes people do weird things. We need to slow down traffic to reduce the stress.

Audience member: 'Improve bike lanes' and 'improve traffic flow' seems like the same thing.

Audience member: What if the lanes are reduced and the idea is that automobiles shouldn't use the street? That would be bad, because it would actually cause a lot of problems.

Audience member: To me, improving traffic flow is for walking, biking, and cars and it means slowing it down, making it more thoughtful. I would like to sit outside at a restaurant and enjoy it but you can't do that now on Willamette Street.

Audience member: I don't drive a lot, but I do if I have to go to Willamette Street, especially if I bring my kids. Even driving there is very stressful.

Audience member: Regardless of the mode, it is stressful for people.

Audience member: Sometimes congestion is a calming (slowing) device.

- 6. Cyclists only (self-define): What would you prefer?
 - A. Bike lanes on street: 23%
 - B. Bike lanes separated from street: 27%
 - C. Bikes & cars sharing lanes: 0%
 - D. Parallel bike route off Willamette Street: 8%
 - E. Other: 0%

This question was asked again removing separated bike lanes as an option, since it is likely to be a more long-term solution.

- A. Bike lanes on street: 40%
- B. Bikes & cars sharing lanes: 1%
- C. Parallel bike route off Willamette Street: 11%
- D. Other: 0%
- 7. Non-cyclists only: What would help you to ride your bike on Willamette Street?
 - A. Bike lanes on street: 11%
 - B. Bike lanes separated from street 15%
 - C. Bikes & cars sharing lanes 0%
 - D. Parallel bike route off Willamette Street 10%
 - E. Other: 3%

Discussion on bikes:

Audience member: Bicycle safety means anti texting laws.

Audience member: Despite the fact that you have to have the speed limit the way it is, the lights could be set slower.

Audience member: My middle school son used to ride his bike up Willamette Street and there are tons of kids in the area. This needs to be safe for kids because even if you tell them not to use a certain route, if it's the most direct way, and it has no hills, they will use it.

Audience member: Question about application of a multi-modal level of service analysis for each of the modes.

Response: We performed an analysis of each of the transportation modes (auto, bicycle, pedestrian, and transit) for the existing street segments on Willamette Street, but found that the results were not particularly helpful at this stage. For example, the results indicated a current level of service for bicycles of "D" while we don't think most users would agree with that assessment. We do plan to use the multi-modal level of service analysis tool to help compare alternatives to better understand some of the trade-offs. (The MMLOS results are included in the Existing Conditions Report on the project website).

Audience member: Use concrete because it is better than asphalt and lasts longer. It's good for cyclists.

Audience member: Cycling up Willamette Street is a gauntlet and stressful.

Ellen Teninty asks some questions for response by raising hands: and the audience overwhelming agreed that they supported slower traffic on Willamette Street, undergrounding of utilities, improvements to stormwater, and consolidation of some driveways.

Additional comments and questions:

Audience member: I have concerns about this project being separate from the long-term planning.

Response: We have coordinated between the efforts and we hope to be responsible stewards for the public interest.

Audience member: Even if we had separate bike lanes, I wouldn't use them because of all the driveways.

Audience member: We need to look at the possibility of bus pullouts.

Audience member: Is there any effort to link this project more with 18th & 20th & Willamette and the rest of the routes to downtown?

Response: We have done a lot of work and will continue.

Audience member: Many people here walk and bike or would like to and I think this speaks to the need for complete streets.

Audience member: Alternate bikeways are very important and I believe that if you put a bicycle lane on Willamette Street, it will put the bicyclists in great danger. A lot of people are not used to sharing the road with bicyclists.

Audience member: For the next forum, I wish you would put in a slide with options for painting the lanes on the street for the full range of potential alternatives.

Audience member: I heard some people bring up parallel ways to get around the area in bikeways off of Willamette Street. What I want to do is access the businesses and services on the street and have equal access as anybody else.

Audience member: If I'm riding my bike, I'm more likely to just stop by one of the stores on a whim.

Before people left, they filled out the following two questions and dropped them in a collection bin:

- 1. One thing I want to make sure is front and center in the plan is: (see below for answers)
- 2. After this evening, I am most encouraged by: (see below for answers)

Ellen Teninty concludes at the meeting at 8pm and thanks everyone for attending. The audience applauds. Some audience members shout 'thank you' for having the meeting.

Additional Meeting

Wednesday, November 14, 6-8 pm, Roosevelt Middle School Cafeteria, Eugene

The morning after the forum, the Register Guard erroneously reported that the forum would be held that night. The City recognized the implications of the error and quickly made plans to host an additional meeting. Following are the comments, questions, and survey responses from this group of 20 participants, some of whom also attended the previous evening:

Audience member: So there isn't a design already?

Response: No, we are developing alternatives.

Audience member: Is there a specific design on the table?

Response: We are developing the alternatives and are in the idea-generation phase.

Audience member: If the group said, "Let's not do a project," would you listen to us and not do a project?

Response: We would report it to the City Council. There will still be a paving project. There is a need to fix some of the major issues, however, and the money is already there (for the pavement preservation project through recent passage of the pavement preservation bond).

Forms were handed out to mark, and show of hands was requested. A total of 16 surveys were returned and the results are:

1. Willamette Street between 24th & 32nd Avenues has some problems.

A. Agree: 100% B. Disagree: 0% C. Not sure: 0%

2. How do you <u>usually</u> travel on Willamette Street?

F. Walk or mobility device (wheelchair, scooter): 19%

G. Bus: 13%H. Bike: 12%I. Car: 56%J. Other: 0%

3. Where is your neighborhood?

E. South of 32nd Ave towards Spencer's Butte: 51%

- F. West of Willamette Street towards Friendly Street: 14%
- G. East of Willamette Street towards Amazon Parkway: 33%

H. Elsewhere: 2%

Audience member: How will these tallies be used? I'm concerned about equal representation. Response: We are concerned about equal representation too. The information will help us determine where else we need to reach out to in order to gain equitable and robust representation.

Audience member: Was the Bailey Hill project effective as far as travel and congestion?

Response: There was a fatality there before the project and it is not yet known what the effectiveness is since completion of the project.

Audience member: How was the traffic study done? Specifically how were the number of lanes studied?

Response: Autos, pedestrians, and bicycles were counted during three-hour periods in the morning and evening peaks at the studied intersections.

Audience member: It sounds like you're there: that there is a problem. Are we beyond debating that there is a problem or not?

Response: Yes. We believe it is our duty to do something about fixing this problem for the community. We are at the point of identifying what the solution is and going after grant funding to further achieve the solutions desired.

Audience member: Is this project reality-based or is it what we would like to do down the road? Response: We are engineers and are very reality-based. We want to make it work with what we have.

Audience member: Is there any reason why a concrete barrier separation like a short wall might not be able to fit or work in the area? Her stepson ran out and got hit by a car and something like that would help prevent that.

Audience member: I would like to make sure that whatever happens in the planning area, that it connects and hooks into the bigger system.

Audience member: I was going to make the comment about how I appreciate the stats on injury and crashes on Willamette Street. I usually use Amazon Parkway. Wider sidewalks and lighting and having drivers be more aware of what's going on will help [the vision impaired]. The crosswalks seem to be in the right positions and I appreciate that the talking signals were installed.

Audience member: Are there plans for bus rapid transit to be installed?

Response: That could be 50 years from now, it's a long ways out.

Audience member: I'm delighted to hear plans to fix this dysfunctional disaster. I think it has tremendous potential and I encourage bold steps because it could be a wonderful place. I like the whole idea about facilitating bikes, because it needs to be easier and safer. I won't get out to ride my bike. I think that the planning should focus on enabling other modes, like pedestrians and bikes. When people walk on Willamette Street, people get sprayed with water by busses. There should be some specific stormwater improvements to areas around bus stops.

Audience member: Bicycles share the sidewalks with pedestrians and I think the speed of the bikes is a problem.

Response: Mostly it's because of two modes sharing the same space.

Audience member: I bike more risky and faster when I'm on Willamette Street because you sort of have to in order to be successful if you want to ride that corridor on a bike. It seemed like there was an opportunity to do some of the development I've seen in a better way than has been.

Response: We are looking into form-based code and design standards to help prevent some of those kinds of problems from happening.

Audience member: There are a lot of driveways and that causes a lot of issues. I see it as incompatible to have walking where there are a lot of driveways. There is a redundancy with bike and bus lanes but the sidewalk is what matters. I think we can't be all things to all people or it will be a disaster.

Audience member: I am not an all-weather biker. There isn't bicycle infrastructure that makes me feel safe there.

Audience member: Bikes can park and walk and in my opinion.

- 4. What's your #1 priority for improving this section of Willamette Street?
 - A. Make bus stop improvements: 0%
 - B. Add bike lanes: 31%
 - C. Improve pedestrian crossings: 12%
 - D. Have better sidewalks: 44%
 - E. Improve traffic flow: 0%

If not one of these, then write your #1 priority here: Trees

- 5. What's your #2 priority for improving this section of Willamette Street?
 - A. Make bus stop improvements: 1%
 - B. Add bike lanes: 31%
 - C. Improve pedestrian crossings: 19%
 - D. Have better sidewalks: 19%
 - E. Improve traffic flow: 13%

If not one of these, then write your #2 priority here: (no responses)

Discussion:

Audience member: Amazon Parkway is an alternative bicycle route to Willamette Street. Isn't it a little easier for drivers to go a little further away than it is for someone in a human-powered vehicle? There also needs to be more bike signage for where these bicycle routes are.

Audience member: I wonder if there are any ideas floating around about how you can widen the corridor while not compromising the business parking access?

Response: Yes. Chris discussed alternatives and options, some of which would require more right-of-way.

Audience member: There just isn't enough room on Willamette Street. Why not just run a parallel bike route to Willamette Street? Reducing traffic lanes would be disastrous in this area.

Audience member: Alleys could be helpful and considered for improvements.

Audience member: The bicycle interest in this is not about finding alternative routes to bypass Willamette Street. It is because cyclists would like to access the businesses equally. For some people, walking is their primary mode of transportation. It is a basic right to be able to get there the way that people want to get there. The people who want to use the area will use the area more because it won't be a hellish place to visit.

Audience member: If we widen the sidewalk, people who bike can also use the sidewalk –they are already having to do that. And thank you for having this public forum.

Audience member: Is it clear at this point whether or not we need 4 lanes of traffic? Response: We haven't done that analysis yet. It can work today and it may work in the future, however in the future, the place may be different and we may need to adopt different mobility standards.

- 6. Cyclists only (self-define): What would you prefer?
 - A. Bike lanes on street: 13%
 - B. Bike lanes separated from street: 38%
 - C. Bikes & cars sharing lanes: 0%
 - D. Parallel bike route off Willamette Street: 0%
 - E. Other: 0%
- 7. Non-cyclists only: What would help you to ride your bike on Willamette Street?
 - A. Bike lanes on street: 0%
 - B. Bike lanes separated from street: 25%
 - C. Bikes & cars sharing lanes: 12%
 - D. Parallel bike route off Willamette Street: 0%
 - E. Other: 0%

Additional discussion:

Audience member: There could be a dedicated through-lane.

Response: Please send us more information about that. It's a new idea.

Audience member: There is a growing movement about mechanized bikes and there could be a shift where there are just a lot more bikes and that should be accommodated.

Audience member: I'm not thrilled about alternative bike routes.

Audience member: Would it be possible to have the speed limit be slower, like 20 mph? It could help with congestion.

Response: We set the speed limit to the travelled speeds so it would be highly unlikely that people would actually drive that speed.

Audience member: I'm not saying we should not try to accommodate bicycle access, however I think that alternative routes would be best. Also, there should be covered bike parking areas that maybe even have a special lock. They could be so that you could ride your bike, park it, and then ride your bike and park it. I also think there should be improved cross walks. There is a huge drop in traffic during the summer months and that's probably due to UO enrollment.

Audience member: My general appeal is to expand the way we think about this project to be more than just about traffic to be instead more about the creation of place. Thinking of a redesign of that space. I think front and center is really aesthetics: what does it look like to be there and what does it feel like to be there?

Audience member: Greenery and good materials and things that make it look nice and make it look inviting.

Audience member: I really think this corridor could be more things to more people. I think we need to be sensitive to look at what really does work for people with disabilities, people who walk and ride bikes and what they need.

Audience member: On beautification: greenscaping sounds lovely. They should underground the power lines. They are so ugly. It should be a top priority.

Response: We will be asking EWEB what it will take to underground the utilities and that will be a decision by the City Council and the EWEB Board.

Audience member: Some kind of little topographical bump or something would be nice for cars to be able to tell that they are in the pedestrian realm.

Audience member: It would be good to have some signal to cars that they are in the pedestrian area.

After this evening, I am most encouraged by:
Principles for aesthetics, livability, social public space
How many people showed up! Good notification from City and interested parties/groups
Good sense by participants - excellent points by all (most)
Discouraged by the lack of opinions by those living South of 29th Avenue
Sorry. I could not stay.
Number in attendance, recyclable plates.
The variety of helpful community suggestions.
The great process
People caring, showing up, and getting involved.
Vocalization of bike and ped advocates.
Citizen turnout
The openness of staff/consultants to consider unconventional approaches/ideas. If it goes rigid with engineering "RULES" it will be resisted.
Support for bike infrastructure.

One thing I want to make sure is front and center in the plan is:	After this evening, I am most encouraged by:
Equitable space for all modes - access along the street for motors/ped/bike	The variety of options being explored which
motoro, pour ante	includes bike and ped access.
This corridor needs a lot of fixing, not just pavement and re-striping	Good start by City. Tremendous potential for improving corridor.
Beautification	The idea that the best ideas will come to the forefront.
Street aesthetics improved	The emphasis on "complete street"
Business interests are foremost in the plan	Discouraged by the possibility of adding bike lanes to the street!
Consideration of the needs of business and property owners	Diverse opinions including consideration of those not present. i.e. the vast majority of car users. Statistical info was very helpful
There is not a parallel/alternate route to access Willamette.	The number of people who want to access the businesses on S Willamette.
Not overbuilding multi residences on street. (like the new one on 24th Place and Willamette). Supporting successful small businesses.	
Traffic calming that incorporates bike lanes to vegetated stormwater system	
Family destination oriented, multi-modal road design (not thoroughfare capacity)	Overwhelming community need/support for bike/multi-modal transit on S Willamette
Bike lanes	The timeline to make changes
Physical safety from cars for bicyclists. Please have a physical barrier!	Mention of bikes and peds having dedicated lanes
Adequately wide bike lanes (at least 6 feet)/paint the crosswalks!	Nothing. The opening comment that stated that there was no room on the road for bikes was beyond insulting. Bikes Belong!
Traffic flow with bike lanes on street. Two lane traffic with center turn out, 3 lane total. We need to keep traffic on Willamette not send elsewhere	Keeping business and property owners in mind

One thing I want to make sure is front and center in the plan is:	After this evening, I am most encouraged by:
Bike lanes	The focus on "complete streets"
Bike lanes on the street will help peds as well as calm traffic	Not much. There are a lot of confusing questions, people didn't get their personal questions answered as well as they could have.
Possibility of parallel bike route. Reducing to 3 lanes with buses using only lane available would make traffic flow unbearable.	Study by the City to arrive at solution.
Safe movement of bikes on Willamette	You are considering bike/ped/wheelchair transit since "concept" does not
Bike lanes and traffic stress reduction	The broad consensus in improving the state of the street
On street bike lanes (although separation would be awesome!)	Landscaping and utilities changes possibilities
Would love to see bike lane between 18th and 24th as well	Needed to leave early
Bike lanes with no car parking to the right of it since the lanes are always put in the door zone.	I am discouraged by staff's reluctance to slow traffic. Also seems like Eugene spends a huge amount of its funds on the city planners, and the hired consultants. Maybe we could skip the planners and just hire consultants.
Bike lanes	Openness to different design options
Reduction of through traffic. Two travel lanes w/center turn lane and on-street bike lanes	level of interest/turn-out for workshop
Good bicycle access on Willamette	The turnout! Must have been the pizza!
Bike lanes on street, 2 travel, 1 turn lane	This appears to be your goal - good!
Creating bike lanes whether separated or on street. BUT if we could increase buses to every 10-20 min I would prefer bus to bike. I commute from S Will to Valley River and if it was faster I would take bus.	Project appears very bike friendly.

improvements could go a long way toward slowing traffic in a natural manner. Ideally thru a combination of public (brick sidewalks) and private (signage, lighting, awnings). Plantings, underground utilities, etc. could make a big difference. Driveway consolidation would be a great step.	That city staff (Chris Henry et al) seem to genuinely care about improving bike transit in that corridor, as well as including all types of users.
Cood facilities for hilles	
bi	Consideration being given to a cross-section with bike lanes, 2 travel lanes, one turn lane, like the road diet" concept.
Bicycle safety Tu	Furnout was encouraging
	The emphasis in the presentations on balanced multi-modal usage, not just traffic
and peds alike - both by getting bikes off the sidewalk and by the	Near-consensus that better facilities for walking and bicycling on S Willamette is desired and needed.
ho	Finally looks like we will get an improved and nopefully more aesthetically pleasing ransportation corridor.
Less cars, more safety for bikes and peds	
Bike lanes and an aesthetic that encourages people to walk and enjoy the area	
st C	Plans to extend bike paths. Need to add a striped/lighted crosswalk for entering Market of Choice at the True Value/Citibank end of parking ot.
Safety for bikes, pedestrians Va	/ariety of ideas offered including public use/social
Encourage people to leave their cars at home	

One thing I want to make sure is front and center in the plan is:	After this evening, I am most encouraged by:
Bikes and pedestrians are apart	You are talking about peds being apart from bikes - YES! Also that the buildings will be nice.
Clear bike/ped travel corridors	Inclusion of ped and bicycle needs and services
Ability to safely travel with children along the corridor as a ped or cyclist.	The decorum of the presenters. Thank you.
Clear division of a shared bike/ped path	The # of people concerned re. changing a short but dangerous section of Willamette.
Enhancing access for walking and biking.	
Bikes on Willamette and safer ped environment	Community interest
Disincentives to drive, encouragement for non-motorized travel.	The turnout
Inclusion of bike lanes and ped improvements	The interest in making a bikeway. It has to be safe from the cars.
Pedestrian and bike friendly/safety	Something might improve in the not-too-distant future.
Bike lanes and better sidewalks	The fact that things might change, however I hope it happens while I am still able to walk and bike!!
A way to bike and walk safely	Outstanding turnout. Need a larger facility.
Bike lanes and better sidewalks	It is clear the project team is hearing what the users and future users see as needs for the street. It is great to see such a large population at a community forum.
Making walking/biking pleasing and safe and sustainable	Any changes to Willamette
Good separation between traffic and walkers.	The fact that planners sincerely want smart community input and are open to affordable creative solutions they've NOT yet imagined.
Bike and ped improvements	All the ideas, especially ones improving ped and bike

One thing I want to make sure is front and center in the plan is:	After this evening, I am most encouraged by:
Multi-modal: safe walking and biking for FAMILIES	The number of people!
Pedestrian and bike lane options	I loved the patience, knowledge and respect that Chris showed toward the audience.
"exposure" is first priority: pedestrians given highest, bikes next, cars last	high concern for non-auto transportation
Pedestrians	
Pedestrian friendly, inviting access. Storefronts closer to sidewalks.	Involvement of people from Portland
Pedestrian experience	Full consideration of all transportation modes, not just automobiles. For me, the ped experience is the most important.
No bikes or skateboards on sidewalks. Make Willamette one way from 20th to 24th to reduce congestion and make room for bike lanes past the Civic Stadium site.	The number of people who wanted slower traffic on Willamette and bike routes off Willamette
Sidewalks and driveways	
Safety for pedestrians - free of roostertail sprays and drivers who might at any moment hit a ped (me!)	Competency of the people running the meeting (the women!) and thanks, Chris for a good job, too.
Pedestrian access to businesses - safety	Some good ideas. I'm glad people are working on solutions. The audience had some of the best ones.
A better safer pedestrian environment with pedestrian crossings and aesthetic separation btwn the high speed traffic and the sidewalk - ie. planter boxes (concrete, 2' high) vs. just a flat planter strip. The sidewalk design should include parallel bike parking btwn the curb and ped walkway perhaps intersperse the concrete planter boxes with bike parking spaces. Include topo/relief marker where sidewalk intersects driveways so that the driver can feel that they are crossing into the ped area. I would like to see the old wooden electric poles and the traffic light poles replaced with modern sleek metal poles that serve both as elect. poles, traffic lights, and as lower street lights, and can accommodate banners for special events.	One way or another it's going to look better!

After this evening, I am most encouraged by:
The fact that something may finally happen to improve this part of Eugene.
All the planning that has happened so far and how well laid out the plans are.
bicycle advocacy in this city
Comment: There was no mention of how climate change might guide your direction/planning
Preparation for participation
The city planners do seem to be aware of the above problem.
The participants overwhelming desire for safe, separated bike facilities on Willamette.
City staff really wants to make this better :)
Discussion about bike ways
large turnout
I'm confident the City will hear great ideas from cyclists.

One thing I want to make sure is front and center in the plan is:	After this evening, I am most encouraged by:
Why are we limited to right-of-way? Use eminent domain to get more right of way to do it right the 1st time.	
Expand analysis to entire corridor i.e amazon, high, pearl, oak. Make the whole flow better!	Level of support for alt modes.
Roundabouts	
Run-off is huge and bioswales are a must!	Good change
Trees	An energetic citizen involvement. I appreciate the City doing this planning process, not only to improve transportation for cars, bikes and peds, but in the process to make a better place to live.
Keep costs under control. Consider what impacts will be on other alternate routes if Willamette becomes more difficult to get thru. Recognize the dependency we have on the CAR. Also recognize how the development at Civic Stadium may impact area.	
Coordinate with the rezoning project	
Street design and Trish's planning project need to work hand-in-hand to create a sense of place to assure both designs will work together. Work with police to enforce slow speeds and create a new norm of how traffic moves thru space. I am stressed by the poor driving, tailgating, etc.	Closure of driveways and possibility of reducing speed.
Safer for bikes/peds. I'd love to see on-street separated bike lane but if it can't be done well, then parallel route is my choice. High density of driveways concerns me greatly.	Consideration of kids in the discussion.
There are 2 gas stations and one lube in this area. Hindering traffic into these locations would be devastating for those businesses.	Please more vegetarian pizzas next time. This is Eugene after all.

Community Forum #1: Explore the Alternatives

Tuesday, November 13, 6-8 pm, Roosevelt Middle School Cafeteria, Eugene

Welcome and Introductions: Ellen Teninty, Cogito

Project Overview and Process: Chris Henry, City of Eugene

Clicker Questions #1, #2, and #3

Existing Conditions & Design Elements: Scott Mansur, DKS Associates

Ideas and Questions from the Audience

Clicker Questions #4, #5, #6, #7

Table Discussion

CLICKER QUESTIONS

- A. Agree
- B. Disagree
- C. Not sure

2. How do you <u>usually</u> travel on Willamette Street?

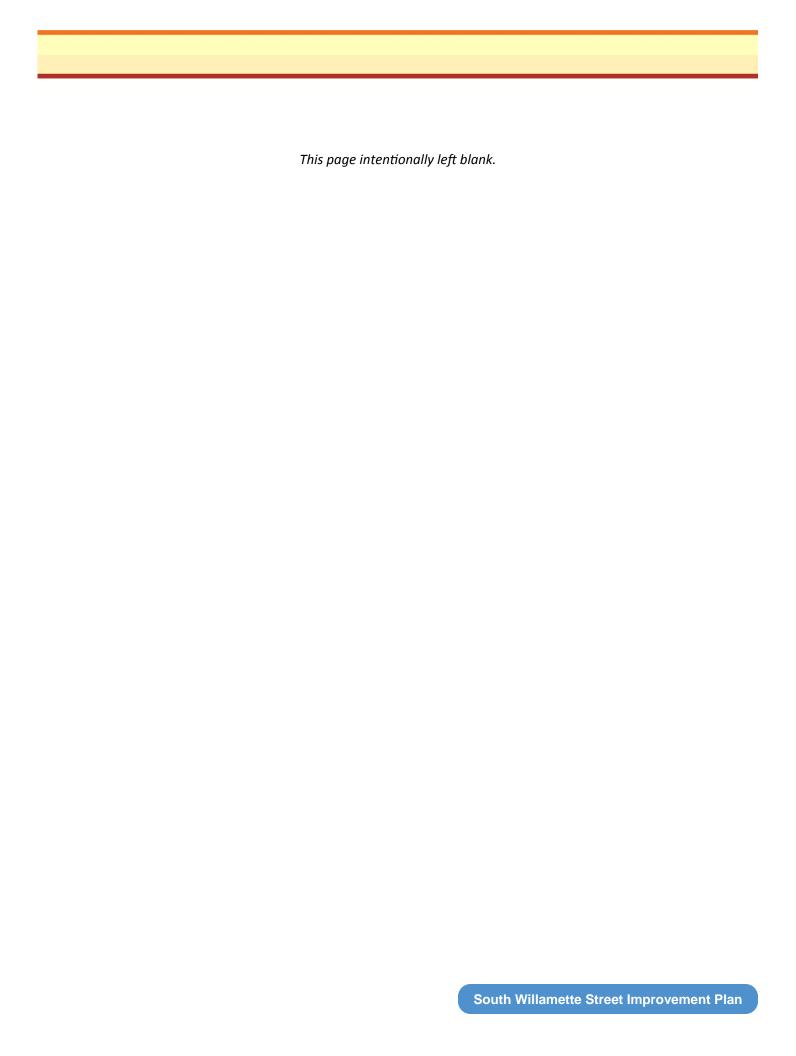
- A. Walk or mobility device (wheelchair, scooter)
- B. Bus
- C. Bike
- D. Car
- E. Other:

3. Where is your neighborhood?

- A. South of 32nd Ave towards Spencer's Butte?
- B. East of Willamette Street towards Amazon Parkway
- C. West of Willamette Street towards Friendly Street
- D. Elsewhere:

A. What's your #1 priority for improving this section of Willamette? A. Make bus stop improvements B. Add bike lanes C. Improve pedestrian crossings D. Have better sidewalks E. Improve traffic flow If not one of these, then write your #1 priority here:
 5. What's your #2 priority for improving this section of Willamette? A. Make bus stop improvements B. Add bike lanes C. Improve pedestrian crossings D. Have better sidewalks E. Improve traffic flow If not one of these, then write your #2 priority here:
if not one of these, then write your #2 priority here.
 6. Cyclists only (self-define): What would you prefer? A. Bike lanes on street B. Bike lanes separated from street C. Bikes & cars sharing lanes D. Parallel bike route off Willamette Street E. Other:
 7. Non-cyclists only: What would help you to ride your bike on Willamette? A. Bike lanes on street B. Bike lanes separated from street C. Bikes & cars sharing lanes D. Parallel bike route off Willamette Street E. Other:
Table Discussion 1. One thing I want to make sure is front and center in the plan is:
2. After this evening, I am most encouraged by:

SECTION F COMMUNITY FORUM #2 SUMMARY





Meeting Summary Community Forum #2: Evaluate the Alternatives

Wednesday, February 27th, 6:00-8:00 pm, South Eugene High School, Eugene, Oregon

Overview

At the second of three community forums, the public learned about six alternatives for redesigning Willamette Street between 24th and 32nd Avenues, asked questions, and shared views on which three alternatives should be chosen for further study. The study aims to help South Willamette Street become a vibrant urban corridor accessible by bicycle, foot, car, and bus. The area includes residential, commercial, and mixed uses, and has six intersections being studied over about a ¾ mile stretch.

The large cafeteria at South Eugene High School was filled to capacity with over 300 participants: almost double the number of participants that attended the first forum. The public listened carefully to the alternatives and was respectful and thoughtful in asking questions and sharing a wide range of opinions. After meeting in small groups to discuss the alternatives, participants completed Input Forms to indicate which three alternatives they prefer to forward for further study. The meeting ended with a large group discussion.

When making the decision about which alternatives to study further, the City considers several elements, including:

- Comments from stakeholder meetings
- The results of Community Forum #2
- Email and phone comments to City staff
- Technical issues and how each alternative meets the Project Criteria
- Review and concurrence by the project Technical Advisory Committee

For more information, or to view the slideshow presentation or Forum #2 Appendix, please visit the website at http://www.eugene-or.gov/SWillametteStreet. The files are located in Project Documents, upper left section of the webpage.

Results of the Public Input Forms

285 People signed-in at the meeting and **301** Input Forms were filled out (see the end of this document for a sample Input Form). Of the Input Forms, **114** checked off alternatives without comments or adaptations, and **187** included adaptations or written comments.

Question #1: Please check the 3 alternatives that you would like the City to evaluate in more depth. Results:

Alternative 3: 3-Lane with bike lanes (**208** preferences)

Alternative 4: 3-Lane with buffered bike lanes (142 preferences)

Alternative 5: 3-Lane with wide sidewalks (139 preferences)

Alternative 6: 2-Lane with bike lanes, median & roundabout (113 preferences)

Alternative 1: 4-Lane (97 preferences)

Alternative 2: 4-Lane with center left-turn lane (83 preferences)

There were 3 "I don't know's"

Question #2: Are there modifications you suggest to the above checked alternatives (such as width of lanes, sidewalk, etc.)? Results:

For details, please review Forum #2 Appendix: Input Form Comments. Written comments from the Input Forms were sorted into three categories: Modifications, Questions, and Comments. Then, the input was sorted according to topic. Following are reflections on the written input:

Suggested Modifications

Alternative	Total	Topics Addressed
	Comments	
1	10	varied
2	7	varied
3	23	19 suggestions on lane width or bike lanes
4	17	10 on lane width or bike lanes, 7 on sidewalk issues
5	45	24 on cycle track ideas, 7 on sidewalk issues
6	22	varied
Mixed	22	varied
All	100	27 on pedestrian or sidewalk issues
		16 on transit
		9 on utilities
		9 on parallel bike route

Questions

Topics varied widely.

Comments

Alternative 6 generated the most comments (16), perhaps because it was challenging to conceptualize. Bike and pedestrian issues were the most common comment topics (28%)

Detailed Summary of the Meeting

Kurt Corey, the City's Public Works Director, gave opening comments of confidence in the team and gratitude for community turn out. Project Manager Chris Henry discussed funding and the decision-making process for the project, "Tonight we will look at six alternatives and then we will narrow them down to three." Mr. Henry said that asking for public input will help the City be good stewards of public dollars by not wasting time

exploring alternatives that are not supported. Chris Watchie, Public Involvement Specialist, briefly reviewed 150 years of history of Willamette Street. She showed slides of old photos of the street illustrating its evolution. Robin Hostick, City of Eugene Senior Planner, described the long-range concept for the street. He showed a slide illustrating how street-side development may change in the future due to future redevelopment. Ellen Teninty, Public Involvement Specialist, asked participants to use this forum as an opportunity to think beyond their personal experience and more holistically about all users and broader considerations.

Scott Mansur, Project Manager from DKS Associates, explained the framework for the alternatives that have been developed. He described the stakeholder outreach process, the first community forum, Technical Advisory Committee feedback, and elected official involvement. He described the alternatives screening process, the study corridor, and each alternative design option in detail. Peter Coffey, Principal of DKS Associates, reviewed the screening criteria evaluation and findings that have taken place so far and the screening that will occur for the next three alternatives.

Questions on the alternatives:

Audience Member: You said you would talk about capacities tonight. What capacities will these plans cover?

Response: Capacity refers to the potential for each street design alternative to accommodate the demand for mobility from motorists, bicyclists, pedestrians, and bus riders. We won't have those details until we perform more in-depth analysis.

Audience Member: What is the measurement of the sidewalk right now? Response: Up to and less than 9 feet of width.

Audience Member: Will you also be taking into account the traffic pattern changes on related streets? Will this be a part of your study? Specifically Lincoln, Jefferson and other streets located in that area.

Response: It is beyond our scope to go into that level of detail.

Audience Member: Are the traffic counts higher going South than North?

Response: Yes (explains and includes discussion of traffic patterns).

Audience Member: Do any of these plans address left turn signals at intersections? Response: Yes.

Audience Member: Is there any standard for a sidewalk where pedestrians and cyclists use just the sidewalks?

Response: Alternative 5, and a cycle track could do that.

Audience Member: Who pays and how are they assessed?

Response: There are multiple funding sources for any street improvement. (explains funding opportunities and sources).

Audience Member: Will the 3-lane options hold the current capacity that the street does now?

Response: It reduces the capacity, however, it should be adequate depending on the number and location of driveway accesses.

Audience Member: How are you measuring current capacity for bikes? How about for Vehicle Miles Traveled (VMT)?

Response: Explains the measure and process. Notes that the trend is that people are driving less.

Audience Member: Will businesses have to give up any of their parking spaces or any of their property for these alternatives?

Response: Largely not.

Audience Member: Can you describe these alternatives by comparing them to other existing streets in Eugene?

Response: Somewhat. Describes comparisons for consideration, and the project team will try to find more examples.

Audience Member: Regarding option 6, is it certain that eliminating traffic signals will reduce capacity?

Response: Option 6 is the safest. The roundabouts have the capacity to accommodate the motor vehicle demand on the intersection.

Audience Member: If driveways will have to be eliminated, who will cover the cost?

Response: We are not at that level of detail yet, however that is an important consideration that we will look at in the future.

The group moved to a 20-minute small group discussion. Group instructions:

Review each alternative as a group. Ask questions. Talk to people with different points of view to understand perspectives. Table materials:

- 11x17 copies of 6 alternatives
- Half sheet input forms
- Flip chart paper and pens to draw, record group ideas if desired We do not expect you to come to agreement as a group or record your conversation during this time. The goal is to help you make an informed decision about which alternatives you think the City should study further. The flip chart paper and pens are there if you feel inspired to draw or write ideas. This is an opportunity for each participant to study the alternatives, ask questions, listen to different perspectives, and formulate your own opinion.

Participants then moved into a full group discussion:

Audience Member: I was wondering why we are not looking at a 2-way cycle track like on Alder Street?

Response: It was reviewed for the goals of the project and how it impacts the other modes. Other options were developed that provide a balance of access, mobility, and safety for users of the street.

Audience Member: How does the number of trips per day affect the alternatives? Response: The number of bicyclists is not a factor in calculating the level of service. Bicycle level of service (as well as the pedestrian experience) is dependent on the speed, volume, and proximity of adjacent motor vehicle traffic.

Audience Member: Cycle tracks would make it more difficult for cyclists to get to different destinations. Is there anything that is not a part of the pedestrian master plan?

Response: The Pedestrian and Bicycle Master Plan (PBMP) identified the need for bike lanes and parallel routes in the corridor. The alternatives presented respond to the PBMP guidance.

Audience Member: One of the primary problems is that the sidewalks are too narrow and the alternatives, except for number 5, all are going to require expanding beyond the necessary profile.

Response: All of the options work within the public right of way.

Audience Member: [During small group discussion] options 3 through 6 concerned our table about conflicts with busses. Could you talk about that?

Response: We will look at opportunities for bus turnouts in next analysis.

Audience Member: I'm concerned about your bike counts because many of us use the alleyways as an alternative. How is the planning for the northern section going to work with these alternatives?

Response: The project to the north (pavement preservation between 19th and 24th avenues) provides the opportunity to connect the bike lane system on 18th Avenue and further north on Willamette Street to the bike lane system on 24th Avenue. So, that project serves a need independent of what occurs between 24th and 32nd avenues.

Audience Member: What are your criteria for the number of driveways allowed?

Response: Fewer are better. Reducing the number of driveway conflicts improves safety for all the users of the street and also improves the flow of motor vehicle traffic.

Audience Member: What is the collision rate?

Response: It is almost twice as high as we would expect for similar streets in Oregon (5.2 collisions per million vehicle miles travelled versus 2.91 collisions per million vehicle miles statewide).

Audience Member: Are there plans that you have for Amazon Parkway to divert some of this traffic?

Response: We don't have specific plans for Amazon Parkway but we will look at what diversion may happen with the alternatives.

Audience Member: When will there be an analysis of the economic consequences for any of these alternatives on the 120 businesses that rely upon the traffic?

Response: In the refinement of the alternatives "tier 2 analysis."

Audience Member: Instead of having bus pullouts, the center turn lane could be striped to allow the traffic to pass the bus at stops.

Response: Yes and they will look into that. It's certainly possible.

Audience Member: I did my own count and there were 250 cars and 1 bicycle go by in 15 minutes [within the study area on Willamette Street].

Response: The project team is following established City policies to provide for a balanced transportation system that provides options for people to meet their mobility needs.

Audience Member: How much would the gridlock be with option 6? Will it be a five-minute delay or a half an hour delay?

Response: There will be a delay and we will look at the specifics in the next analysis.

Audience Member: The street itself should cater to all modes since it is public money.

Response: A balanced transportation system will serve all users. That is our goal and our policy guidance.

Audience Member: I'm wondering if we could consider encouraging Eugene Police to enforce the rules on Willamette Street?

Response: I shared with the police today that people have been concerned about the travel speeds on the street.

Audience Member: Is there a difference between people commuting on Willamette or people stopping and shopping?

Response: Our grant funding constrains our work but we will try to estimate that.

Audience Member: If you build it, will they [bicycles] come?

Response: We don't have the ability to tell, however we could do some case studies with other communities to compare what happened to them and what they saw afterwards.

Audience Member: I would be scared to go through a roundabout as a cyclist or pedestrian. Do the roundabouts have safety accommodations for them?

Response: We will look at ways to make people more comfortable using them. There are two options for how a roundabout would be designed for bicycle use.

Audience Member: There are a lot of things in our master planning processes over the years. Which one of these alternatives will point us in the direction of what we have already said we want?

Response: The plans say what we want. They do not say how and that is why we are here.

Audience Member: Why not have bikes on the sidewalks?

Response: Typically we do not have bikes share where there are a lot of driveways. Pedestrians do not feel safe with bikes on the sidewalk. Cars do not see bikes when they are pulling out of driveways.

Audience Member: Surveys should be done to talk to residents, asking them what should be done to make them want to walk it more.

Response: We are looking at that information in some other ways.

Audience Member: Concern over decline in availability of fossil fuel. How are you incorporating this into your study long term?

Response: By giving people options for how they travel.

Chris Watchie wraps up the meeting by thanking participants for coming, reminding participants to leave their opinion forms on the tables for collection, reviewing the next steps, and taking final questions. Meeting ends at 8:00pm.

Example of Public Input Form

South Willamette Street Improvement Plan Community Forum #2: Evaluate the Alternatives

1.	Please check the 3 alternatives that you would like the City to evaluate in more depth:
	□ 1: 4-Lane
	☐ 2: 4-Lane with Center Left-turn Lane
	☐ 3: 3-Lane with Bike Lanes
	☐ 4: 3-Lane with Buffered Bike Lanes
	☐ 5: 3-Lane with Wide Sidewalks
	☐ 6: 2-Lane with Bike Lanes, Median & Roundabouts
	□ I don't know
2.	Are there modifications you suggest to the above checked alternatives (such as width of lanes, sidewalk, etc.)? If so, please describe on the back of this sheet. Suggestions that expand the project beyond the current right of way (60 feet) cannot be included in the short-term.

Alt	1. Suggested Modifications to Alternatives
#	
1	With a turn lane.
1	At least reduce driveways, improve and widen sidewalks, add trees, do something about ugly
	utility poles; also add bus turnouts; designate/improve bike routes in study area.
1	Might work with consistent 9' sidewalks.
1	I like the idea of one side 6'/ other side 12' sidewalks in this one.
1	Reduce sidewalks to maintain some parking lots.
1	With sharrows.
1	Include sharrow lanes
1	Add better bike accommodations/ sharrows/ or adjacent bike infrastructure.
1	Needs sharrows. Not optional.
1	I would like to see LTD bus turnouts on both sides.
2	Could you consider some center islands in left turn lanes so that crossing would be easier? For
	pedestrians? Not great for emergency vehicles.
2	It is important to consider aesthetics, like trees, street lamps.
2	With bike "lane" on sidewalk
2	Must have sharrows and lower speed limits.
2	With sharrows.
2	If two lane with center lane: Has anyone considered bus only lane in center with bus stops with
	proper pedestrian crossings from center point of stops (no bike lanes).
2	And bus pull out or center lane modified
3	Consider raised bike lanes.
3	Needs soft hit posts to separate bikes from cars.
3	If option 3 was chosen I would want the bike lanes to be marked clearly, perhaps with different
	pavement coloring at driveways/ intersections or a different striping pattern.
3	Need bike buffer, less width sidewalk.
3	Need buffers -safety for bike.
3	Is there a way of having the center left turn lane be a variable lane so that during peak capacity
	times, the center lane becomes a traffic lane in the peak direction, with no left turn allowed
	during peak times? Overhead indicator lights could tell you whether you drive or turn left or not
	enter that lane.
3	Consider narrowing the bike lanes (or ped sidewalks) slightly to allow for 11' wide lanes. Or
	provide special design considerations where bus stops exist.
3	Create 11 foot travel lanes by reducing bike lanes to 5.5' and reduce sidewalks to 8.5'.
3	With 11' thru lanes and 5' bike lanes.
3	Reduce bike lane to 5' and add back to travel lanes to get 11'.
3	Put 5' bike lanes and 11' travel lanes.
3	And travel lanes at 11'
3	8.5' - 5' - 11' - 11' center turn -11' - 5' -8.5': Change the bike lanes to 5 feet, increase the car
	travel lanes to 11 feet to accommodate trucks. This would also require decreasing the sidewalks
	by 1/2 foot on both sides (only) to gain the extra foot width in the center lane.

3	Can the bike lanes be reduced to 5', giving back to the two travel lanes?
3	Possibly #3 with 5 or 5 1/2 ' bike lanes and 11' outside travel lane?
3	If you make the bike lanes 5 feet wide (standard width), you could either make 10' sidewalks or
	11' travel lanes (better for transit vehicles).
3	8', 6', 11', 11', 11', 6', 8' Can sidewalks be 8'? Then we would gain more room for vegetation and
	bike lanes and retain 3 car lanes.
3	11' travel lanes and 8' sidewalks
3	Reducing the bike lanes to five feet and expanding the through lanes to eleven feet would seem
	to help transit without impacting bikers much.
3	I think it is important to have bike lanes and to have a turn lane. Alternative 3 best meets this,
	but has narrower than standard width. Meeting travel lane width standards could be met by
	shifting 6 inches from each of the bike lanes and sidewalks.
3	"Bike lane" on sidewalk possibility.
3	Need bus turnouts!
3	Needs to have bus pullouts -narrower sidewalks or no center turn lane to allow traffic to pass.
	Sharrows in the 29th Ave vicinity. Limit restrict left turns during peak traffic periods to maximize
	through traffic capacity.
3	To minimize bus/ bike conflict, maybe sidewalk curbs could be lowered at the front and back of
	the bus stop so that if a bus is at the stop or coming in it, a biker could roll easily into a
	designated lane on the sidewalk to bypass the bus stop. Pedestrians could stay out of the bus-
	stop-bike-bypass designated area! Maybe could keep things moving.
4	Raised bike lanes.
4	Buffers not needed.
4	Don't need buffer.
4	Take option 4, eliminate 2' buffer give one foot to bike lane and one ft to sidewalk.
4	Reduce 11' lanes to 10' and increase sidewalk to 8' wide with 3' wide tree and light planters
	(rectangular) and 5' sidewalk. Keep or move telephone/ power poles into 3' planters -keep
	them out of sidewalks!
4	With 10 ft. travel lanes to allow for wider sidewalks.
4	Make car lanes 10' wide or widen sidewalks.
4	Reduce car lane width to 10' to make sidewalks wider.
4	Maybe travel lanes 10' each, that will provide buffer for bikes as well as 8' sidewalks.
4	Lessen sidewalk widths. Why the need for 9' sidewalks? Not necessary. Lessen width, more
	space for bike lanes and buffers etc.
4	If neighborhood concept plan requires 5ft setback so sidewalks would eventually be wider.
4	This option would need to consider larger sidewalks to be viable.
4	Would like a wide enough sidewalk where bikers that aren't comfortable w/ the road are able to
	ride on.
4	Could be almost 8' sidewalk by narrowing travel and turn lanes could happen.
4	Narrows the sidewalk too much.
4	I like this option because, as a cyclist, I would feel more safe and comfortable and less
	vulnerable to vehicles. However, I wish the sidewalks could also be widened to 9'.

4	I like this option, but the space provided as a buffer could better be used to give more space on
	the sidewalk (where it will provide more value than the 2-ft buffer).
4	How will busses be accommodated?
5	Could ther instead be a curbed single lane cycle track w/a 6 foot sidewalk?
5	With cycle track.
5	Can there be wider sidewalk that has dedicated bike track on one side and narrower sidewalk on other?
5	Add or include raised bike track as medial part of sidewalk.
5	There should be a future plans for bicycle access using cycletracks in each direction.
5	That have designated bike ways on sidewalk
5	sidewalk 9', car 11', center left turn lane 12', car 11', cycle track 8', sidewalk 9'
5	Cycle track. Cycle tracks is the only safe option for bicyclists!
5	Add cycle tracks. I like the sidewalks here but there must be a bike option.
5	Cycle tracks.
5	I actually like number five too if it had a cycle track to accommodate bikes and reduce conflict
5	with bikes and peds.
5	I would also be interested in pirsuing a cycle track option. Perhaps a 2-way cycletrack on one
	side of the street. I would not like cyclists and pedestrians to be on the same gradient.
5	Show how bike/ ped separation might occur.
5	With designated bike lane on sidewalk.
5	Preferred alternative if cycle track will be included!
5	What about some kind of signage to facilitate sharing the sidewalk; no speeding bikes through
	groups, no groups blocking the entire pathway? i.e. "cycle track"!
5	Is impractical for bikeways (cycle tracks) shared with pedestrians and stormwater treatment
	areas it seems. Or is 13' wide enough for both stormwater and cycle track?
5	Only if sidewalk includes cycle track to separate bikes from pedestrians.
5	Include space for bikes on sidewalks i.e. cycletrack.
5	I really like the shared sidewalk idea -where the wide sidewalk has a lane in it for bicycles.
5	Allow bicycles on sidewalk -have a special lane.
5	Needs to consider options to improve bicycle options and access to businesses. Possible investment in Agate Alley/ Oak or shared sidewalk use with clear division between pedestrian
	and bikes.
5	Love the wide sidewalk with cycle markings/ tracks for shared ped/ cycle use.
5	Shared w/ bikes and delineated with striping.
5	W/ devoted bike lane.
5	11.5' wide shared bicycle pedestrian sidewalk, 11' wide South Bound traffic lane, 10' wide South
	Bound traffic lane, 10' wide North Bound traffic lane, 11' wide North Bound traffic lane, 6.5'
	sidewalk
5	Sidewalk 13.5', travel 11', turn 10', travel 11', sidewalk 13.5': take 1 foot from the center turn
_	lane, put it to the sidewalk.
5	Would only be good if sharrow are added to the lane.
5	With bike sharrows.

5	Do not plan on bikes on the sidewalk. What kind of signage possible to help vehicles understand
	bikes allowed full use of lane?
5	With shared pedestrian/ bicycle use of sidewalk
5	Discuss cycle use in sidewalk "realm." If we do not incorporate bikes into this realm, bikes/ peds
	conflicts could be a concern.
5	Wider sidewalks 13' are a big priority!
5	w/ sidewalks being mixed use (bike and pedestrian friendly as are the park and river bike paths.
5	Big sidewalks. Sidewalks should be used for bikes as well as pedestrians.
5	Could bikes and peds share?
5	I would like to be able to bike on the sidewalk.
5	Only if it can accommodate bikes.
5	Show options that accommodate bikes. Or perhaps have widened sidewalks only in the key 2-3 block area (where good side streets for bike commuters exist hopefully). Or wide on one side
	only.
5	At least reduce driveways, improve and widen sidewalks, add trees, do something about ugly
	utility poles; allow bikes to share sidewalk, add bus turnouts or allow vehicles to pass bus when
	stopped.
5	Wider sidewalks, w/ bicycle parking in #5 with possible bus pull-offs makes most sense. Bicycles
	could (and possibly cars) use an alt. route if not visiting businesses/ locations in the area. Bike
	parking area & walk to businesses on wider sidewalks. Also cars could park in one lot and walk/
	cross streets up and down to other businesses and locations = less car pull ins and outs. Less
	driveways for bikes, peds, less car turning to prevent backups.
5	And bus pullouts. Get poles off the sidewalks. Bikes allowed on sidewalks. Fewer driveways.
5	All modes eventually are pedestrians -wider, obstacle-free sidewalks are a must. I am concerned
	that Option 5, despite the wonderful 13' walkways, doesn't adequately address bike/ ped/
	sidewalk seating conflict.
5	With bike turnouts.
5	For bikes and peds.
6	4 lanes w/ roundabouts would be my first choice, moving bicycle traffic to alternative routes
	with some level of dedicated access.
6	Crucial to maintain cyclist safety at roundabouts -diverting cyclists onto sidewalks at
	roundabouts is unsafe in my experience.
6	Intrigued (but concerned about bike/ ped safety at roundabouts).
6	Make the median a little smaller for emergency vehicles to travel easily -maybe combine side-
	walk w/ bike lane.
6	No "raised" median: emergency vehicle access at risk. Raised medians scare me with regard to
	emergency vehicle access. And people get frustrated (road rage) w/ raised median
	inconveniences.
6	Six feet seems a bit wider than necessary for bikers. Maybe we could reduce it a bit to give
Ī	more width to the vehicle lanes and sidewalks.
6	Sidewalks could be expanded to 10' (standard) if median was narrowed.

6	Could reduce the median by 1-2' and add that width to the sidewalk. Also, the same could be
O	done with the buffer and added to the bike lane while adding soft -hit posts.
6	Need to make safer pedestrian crossings since roundabouts can create accessibility option.
6	With round-abouts, would likely make it impossible for the blind to cross the street at those
Ü	locations. It might work for vehicles but probably would add significant risk for pedestrians if
	there are no lights.
6	What about getting blind and disabled peds across Willamette? Without traffic lights?
6	No buffer, wider sidewalk.
6	To help with diversion of commuter traffic off of Willamette, put a roundabout at 24th and
o l	Amazon Parkway -I think this would encourage more people to use this route as now the lights
	are short and turning laborious.
6	If roundabout, then ped. Xing 150 away with signals.
6	
O	I would like to know more about how roundabouts work and if there are safety concerns for
6	peds and bikes. Would like to see what the roundabouts would look like.
	I like (after much thought) the roundabout idea for 29th and Willamette only.
6	I like this option, but the buffer could be used as more sidewalk space. Also I like the pedestrian
	crossings and roundabout. And I like that this is the safest alternative.
6	Put a roundabout on 29th and Willamette.
6	Ugly center median.
6	How about a wider sidewalk with a turning lane and a bike lane in the roundabout. How about
	busses routed to Oak or Portland Street or alley bus lanes.
	Hybridize this option with 3/4.
	Shared side walk, wider -less driveways.
	4 lanes with combo walk/ bike path w/o poles and standards.
1, 2, 3	With bike lanes moved to Oak and Portland
1, 2, 3	Combine sidewalk and bike plus let bike use the lanes with autos.
1, 3, 4	Keep street trees and lights in 4' planter.
2, 5	Include modifications to parallel streets for bicycles (a la Alder St. controls at 19th, 24th, 32nd)
,	or include bike travel accomodation/ protection on sidewalk (plan 5).
3, 4	Only if sharrows are going to be added.
	We need bike lanes that work for people of all ages and ability levels -the only plan that allows
	that is alternative 4. Unfortunately, alternative 4 has the worst facilities for pedestrians. Why
	can't we have the car lanes and sidewalks from alternative 3 and the buffered lanes from
	alternative 4? I like the idea of 8' sidewalks, 5' lanes, 2' buffers, and 3 10 ft vehicle lanes.
3, 4	Should have green paint to keep cyclists safe.
	Is it possible to incorporate roundabouts in options 3 & 4?
	Cycle tracks in sidewalk with three levels (2 curbs) like in Europe.
J, T , J	Cycle tracks in sidewalk with tillee levels (2 carbs) like in Europe.

2 4 5	
3, 4, 5	No to reduced capacity.
2 4 5	
3, 4, 5	Clarify, in 3-lane configurations, what the signalized intersections would be like -would there be
2 4 5	protected turn pockets at intersections?
3, 4, 5	Pull outs for busses would be good in three lane options.
3, 4,	Can we explore cycle tracks like on Alder? Or Pioneer Parkway? Why not.
5, - , 5, 6	can we explore eyele tracks like on Alaci: of Floricer Farkway: willy hot.
3, 4,	Any bike lane need not be wider than 5'. Rather have 11' vehicle lanes. Must have bike lanes!
5, 4 , 5, 6	Any bike faile field flot be wider than 3. Nather have 11 vehicle failes. Must have bike failes:
3, 4,	There is no bus consideration for slowing traffic on only three bike lanes.
5, 4, 5, 6	There is no bus consideration for slowing traine on only three bike lanes.
	Possible to make a 2-way bike lane in one lane like on Alder?
3, 4, 0	rossible to make a 2-way bike lane in one lane like on Alder:
3, 4, 6	5' Bike lanes, 10' sidewalks, reduce driveways. Common parking for bikes and cars to encourage
	visits to multiple businesses in a single trip.
3, 5	Combinations of 3 and 5. Consider narrower vehicular travel lanes -wide sidewalk- buffered bike
	lanes. Consider a 2 lane bike section to gain safety not at the cost of pedestrian use.
3, 5	Consider need to improve options for protected pedestrian crossing (islands) at inbetween
	intersections.
5, 6	Consider bus turnouts to reduce impact on blocking traffic flow.
All	Fixing (reducing) car turn access to businesses is critical for all alternatives and protected
	pedestrian crosswalks. They also have to be usable by bicyclists.
All	More police enforcement for the laws in the area, speed, rolling out of lots without stopping,
	passing cyclists safely.
All	Please consider vehicle- bike -pedestrian as in Amsterdam & Copenhagen
All	No reduction of traffic lanes.
All	Protected left turn lanes.
All	I like the concept of 2-lane South bound.
All	Would bike lanes have significant markings? E.g. bike boxes, reflective approach lanes (similar to
	Portland?), signage at driveways and intersections.
All	Each alternative should use bicycle travel lanes as a buffer between pedestrians and vehicles.
All	Bike lanes are a must.
All	I liked one suggestion that sidewalk be enlarged in width to accommodate bicycles, i.e. 13th on
	one side and 5' on the other, perhaps with a divider on the 13' side to separate pedestrians.
All	Primary importance- If no bike lanes chosen, highly support making adjacent N/S routes
	become most bike friencly and smooth connections to beyond.
All	Save \$ to ensure improvement to alternate routes for bikes for real improvement.
All	Please consider side street corridors for bike riders.
All	Side street bike route?
All	Evaluate potential alternative bike routes. Have to include bike lanes elsewhere.
All	Use parallel side streets for bikers. Way too much congestion already present with cars, etc. on
	Willamette.

All	Bike lanes and access on Portland and Oak.
	Portland and Amazon for bike traffic.
	Use Portland for bike lanes.
	I want peds to be able to cross safely at intersections and midblock.
	More mid-block crosswalks
	More trees and plants close to sidewalks.
	I like the consistent 9 foot sidewalks.
	Any and all options should include signage at intersections explaining peds have R-O-W when
	crossing.
All	More signaled pedestrian crossings.
All	Mid-block cross walks.
	I would like to see sidewalk improvements for ped. Bikes can use other off Willamette Street paths.
All	I would like to see sidewalk improvement as the highest priority -widening and removal of
	utilities. We also need a stop light at Woodfield Station's East entrance.
All	Put utilities in alley? More ped. Crossings!
All	Improve lighting from highway corridor to commercial scale.
All	Trees!
All	More trees for the health and beautification and safety of our community!
All	Would like sidewalks to be as wide as possible even buying a few of private property. A buffer
	would be the icing on the cake.
All	Pedestrians needs should get more attention. Is there some flasher crossings planned for ped.
	Xings.
All	Hide cars so people can see the building and sale items.
All	Include 'on demand' crossings mid-block.
All	More plants!
All	Show pedestrian-scale lighting!
All	Mount street lights off of businesses instead of on poles.
All	Access to Woodfield Station on end of Portland for strollers and bikes.
All	Keep sidewalks as wide as possible.
All	Do not reduce sidewalks whatever you do.
All	Wide sidewalks like Amazon Park "multi-use"
All	More bike parking on every block in plain view!
All	Possibly take the sidewalk width on a block (or property by property) basis. Of course, there
	would be a minimum width -say 5 feet, but not a standard 9 foot throughout.
All	Sidewalks need to be wider in all schemes. Existing sidewalks are too narrow.
All	We should consider roundabouts in alternatives other than just the center median alternative.
All	Option for sharrows.
All	Add sharrows on drawings with no bike lanes.
All	Bike sharrows are too dangerous for people on bikes.
All	Please fix drainage.
	Flease lix uraillage.
All	Make sure and fix drainage issues.

All	Previous re-paving projects have not included adequate drainage for heavy rain, viz flooding
	regularly on Hilyard St. Please create more drainage for ALL street projects.
All	Driveways need to be part of traffic flow plan. Some parking areas may have to be used
	differently to fit traffic flow on streets.
All	Discourage thru traffic on Willamette; signs suggesting alternatives such as Amazon.
All	Yes to traffic signal at Woodfield Sta. driveway!
All	We need a traffic light for vehicles to/ from Market of Choice.
All	I have a priority of slowing traffic on Willamette.
All	Perhaps a slightly raised intersection, and/or crosswalks with a different texture to get traffic to
	slow down in intersections where bikes and people are at greater risk.
All	Suggest adding in other traffic-calming, especially at intersections. For example, intersections
	and or crosswalks that are just slightly raised, different color, and or different types of pavement
	such as bricks.
All	How about a slight raising of the street grade at intersections to help calm traffic?
All	Bus turnouts must be included in all concepts.
All	Yes on LTD pullouts.
All	I am concerned about stops for LTD busses. It would be ideal if there were "cutouts" or
	"turnouts" to prevent busses from blocking bicycle and auto traffic. I don't see this in the
	proposals.
All	Yes to increasing existing R-O-W to allow bus turnouts.
All	I would like to see cutouts along Willamette for busses for better traffic flow.
All	Pull outs for busses seem critical in any alternative.
All	With all alternatives bus pullouts are critical to improving safety.
All	Build transit stops w/ pull outs and shelters and accommodate the other modes around those
	points -include prominent signage to yield to bus and pedestrians -good to co-locate zebra
	crossings with transit stops (e.g. sharrows in lane adjacent to stop). Get LTD and businesses on
	board!
All	Bus turn outs.
All	Bus turnouts such as at Woodfield Station would help the flow of traffic.
All	Incorporate bus turn outs?
All	Bus pull outs?
All	Bus turnouts or better ways for cars to go around bus (without running into turn lane) could let
	bikes merge into travel lane and reduce sidewalk width at those spots.
All	Bus turn outs. Busses need turn outs not to stop in streets.
All	Bus lane pull outs.
All	Some accommodation needs to be made for bus pullouts.
All	Put utilities underground -safer all the way around (including during storm weather).
All	Utilities underground.
All	Underground all EWEB overheads -just street/ stop lights.
All	Utility poles need to be relocated to the outside of the ROW as far from the curb as possible.
All	Consider utility lines (telephone and electricity) in the alleys -would greatly improve look and
	feel of the entire corridor.
All	Bury utilities.

All	Put utilities underground. It will give an extra 2 ft to sidewalks.
All	Put phone/ utilities underground!! It creates space on sidewalk for bikes and peds.
All	Consolidate lighting poles with utility poles to reduce conflicts in sidewalks.
All	Non-buffered bike lane and 11 ft lanes for busses.
All	Bus turnouts. Divert through traffic to Amazon Parkway. Reduce speed limit on Willamette.
All	The selected alternatives for further study should increase accessibility to the business district
	and provide improved conditions for bikers and pedestrians, prioritizing those concerns above
	thru- traffic for p.m. commuters -Amazon Parkway serves that purpose, and by increasing foot
	and bicycle access and improving safety, business will be more supported.
All	Focus on creating a vital economic district that creates a positive experience and is accessible to
	all modes of complete street transport. Commuter vehicle traffic should be shifted toward
	Amazon Parkway (with roundabouts at 24th and 27th and 29th).
All	Concentrate on designs that enhance all user access to the businesses as opposed to catering to
	drivers that just are passing through. Same goes for cyclists -design lanes to enhance bike
	access to businesses -not bike freeway through the area.
All	Could there be a concrete barrier just North of 29th so people could not turn left across 3 lanes
	into the bank. This would be in middle of median. Not a whole wide strip, just a narrow 8"
	barrier.
All	Please use buffers, they are critical for safety and to attract new users who require the
	perception of safety.
All	Just repair or replace existing damaged roads and sidewalks or use effective striping and
	signage.
All	Left turn lanes and fewer driveways would be great. Removing utility poles would also be
	awesome
All	Add light traffic control at Woodfield Station. How about using light control that changes
	direction based on how busy the street get (South vs. North)?
	2. Questions
	In option 5, would telephone poles etc. be put underground?
	Would roundabouts be safe for peds?
	Does option 6 need a median? Less concrete = less cost without decreasing safety.
	How will the Woodfield Station parking lot driveways be consolidated to reduce congestion at
	29th and Willamette?
	Will bikes really stay off the sidewalks if they have a designated lane?
	What similarities does this stretch of Willamette Street have with successful re-designs with
	three auto lanes and two bike lanes? How have these re-designs changed bicycle use? And how
	have the re-designs affected business?
	What evidence exists from other communities that reducing the number of automobile lanes
	Intial exidence exists from other communities that readoning the number of automobile lanes
	_
	can improve automobile traffic flow and/or reduce accidents? What is the future of housing development on S. Willamette -then there is the Civic sound

What is the "expected" demographic change (psu???) for the affected residential
 neighborhoods? No young families are currently inbound.
 What are the anticipated multi-family structures that will be allowed?
Is the middle turn lane in the 3 auto lane options safer for left turning cars than the current two
lanes in each direction, especially when cars from each direction make lefts from the center
 lane at approximately the same spot?
Capacity and street flexibility: If South bound traffic is peaking at a certain time and north
bound at another time, why not have one way during the morning and the other way during the
 afternoon?
 Why isn't Willamette one way and Amazon Parkway the other way?
What plans are being made to improve alleys for bike traffic and side streets and bike access
from Amazon Parkway? And between Amazon Parkway(all the way to Hilyard) to Willamette
 Street. 24th Ave to 30th or 32nd.
Why is the alley running from Capella parking lot South to Oak Street chopped up and not even
fully accessible to any vehicle (not even emergency) and how will this plan remedy this even
 slightly?
With the alts. w/o bike lanes what is the bike path? Would Portland and Oak be developed as
bike boulevards?
If there is reduced auto capacity, can alternate routes for bikes be different than auto alternate
routes?
What after-dark safety measures are in place for peds' safety at night?
No mention of aesthetics -the current street is just ugly. While any change would be
improvement, maybe some alternative would provide better aesthetics?
Will any of the options require any properties or businesses along Willamette to give up any
parking space or property?
What are the safety factors that we are trying to fix? And I do mean -what are the statistics on
accidents compared to other similar traffic situations? Please share this information widely.
Due to the instability of Northbound traffic to turn West on 18th, lots of traffic turns West on
24th and 23rd through the neighborhood. Is there a solution using the Civic property to link Oak
with Amazon?
 What plans best support bus stop improvements and bus turnouts?
Which option might best accommodate EMX in the future?
 Is LTD going to get basements for busses?
 Could the utility poles be moved back from the pavement? Would that be cheaper than
undergrounding?
What physical design steps are being taken to prevent cars cyclists and pedestrians from
"gaming the system" (ie. Cyclists going the wrong way, cars driving in parking lots, jay-walking
peds) when frustrated?
 What is the life expectancy of the "project": 10 years? 15 years? More?
What is happening South of Willamette/ 29th? Does it matter?
 Are there any near term sewer, water, wastewater "projects" needed/ anticipated? Dig a ditch,
fill it in, dig a ditch, fill it in

ŧ	The first alternative is] the only way! Maintain what works. Don't fix something that is not
1 -	proken.
٠.	#1 seems to be the least expensive project.
٠	Option 3] Afraid of lowering people to business
4	Alternative 3 is my highest priority, I think this is a reasonable design that provides access for
	everyone, regardless of their mode of travel.
-	Option 5] Sounds good for a far off future.
	Very concerned that roundabouts would be very dangerous for pedestrians and cyclists.
	Roundabouts are different for pedestrian crossing.
٠	Option 6] is too dangerous for emergency vehicles! Think Crest Drive.
-	disagree that roundabouts are safer in Eugene. It doesn't seem safer for pedestrians or
1	wheelchair users or those with limited vision.
Ī	Option 6] Bad idea for such a stop and go traffic "flow."
F	Roundabouts at small-scale intersections are ridiculous! Terrible for bikes and peds! How ca
t	this be safer? Needs more R.O.W.
Ī	think we need to be very careful to make clear how bikers/ cars should behave in roundable
	to keep things as safe as possible.
Ĩ	am concerned about roundabouts and their safety to the visually impaired.
٦	The roundabout idea is a good one but has been coupled with a raised median which seems
ā	a deal-breaker due to emergency access/ business access issues. Consider roundabouts for
5	some portion of the other alternatives in the next design phase.
	Option 6 is a terrible idea: Northbound from 32nd (pws) would need to travel all the way to
2	27th roundabout to "come back" to Woodfield Station. Narrows access South forever. Very
e	expensive to return to higher capacity.
	No roundabouts. They stink.
-	Roundabouts are dangerous for bikes and pedestrians.
~	Option 6 is] not great for businesses, emergency vehicles or neighbor streets.
(Option 6: This plan would be a disasterous waste of space.
F	Roundabouts are dangerous for bicyclists (pedestrians also).
1	Roundabouts are incompatible with slower, denser, more urban character and also with
Ŧ.	pedestrians and bikes.
-	Leave the 4 lanes -traffic is bad going South or rush hour and not good going North.
	Bikes shouldn't be on road.
	The 2-lane option is too extreme. Willamette is major route -like you said emergency vehicle
	would be affected and parallel streets would be significantly affected by traffic.
1	No bike lanes. They are too dangerous. Bike riders should dismount and walk in pedestrian
-	areas. Bike commuters should be required to use existing bike lanes and routes.
-	205 Cars, 1 bicycle: 15 min tally in the afternoon.
	of 6 altern. Reduce travel lanes, not in favor, there should be more options including/
r	maintaining travel lanes.

	We really must have a discussion of what is going to happen between 18th - 24th because
	connections must be made. How or will 18th -24th be altered? Makes a big difference in future
	designs.
	I am very skeptical that the 3 lane options will have less capacity than existing conditions
	including the mess on Willamette between 24th and 18th.
	I would prefer any alternative with a center turn lane or similar (roundabout for example) to
	improve safety (for drivers, peds, bikes) and avoid the current "slalom" driving experience.
	The street is crying to be 3 lanes in my opinion.
	Options 1, 2, 5 do nothing to accommodate bicycle traffic on Willamette Street. The other
	options are better for all users.
	Too many driveways.
	Accessibility should also be considered -A turn lane provides greatly enhanced accessibility
	(don't just look at mobility).
	Restricting drive cuts and connecting businesses at rear could reduce vehicles on Willamette,
	Rear connectivity would be important.
	Intersection of 24th & Pearl/ Amazon is narrow for S.B cyclists
	"Detour" via Amazon Path adds more time and distance for cyclists. Plus no good access to
	west, i.e. S. Willamette.
	More safety and comfort for families with young kids to walk and bike.
	I ride an incumbent tricycle to many businesses along this segment of Willamette. I can usually
	get to anywhere I want to go, but bike parking is inadequate in all cases except Market of
	Choice and Capella. Bikes have to squeeze between cars and hop a sidewalk to get to bike racks.
	Parking is inherently the biggest deterent to cyclists use of Willamette Street.
	We should not consider any of these that don't include bike lanes.
	We should not consider any plans which don't ensure bike lanes.
	When you add bikes on the sidewalk or add a cycle track, 4 and 5 become almost the same.
	Bikes sharing sidewalks doesn't seem safe.
	Although improving parallel routes for through traffic is great, cyclists want to access the
	businesses on Willamette.
	It is important not to just consider "capacity of the road" being vehicles -what is the bicycle
	capacity?
	Bike lanes need signal change installed on street so bike can trip the signal to green.
	I disagree with the evaluation that all alternatives listed except for #4 are equal in social equity,
	because options 1 and 2 are unacceptable for cicyclists, offering no improvement whatsoever
	and maintaining current terrible conditions.
	I'm concerned that none of these options are sufficient for reticent cyclists to feel safe. Say, for
	parents with young kids to abandon their cars and bike with kids instead. Research shows they
	need to feel more protected to move forward in taking that step. And we want their activity,
	business, and inclusion on Willamette Street.
	We live near Amazon Community garden. We walk and ride bikes in this area for errands, etc
	We mostly use the side streets because of Willamette's condition.

Through traffic that does not patronize any of the businesses could be "pushed" onto Amazon Parkway. Most automobile drivers and bus drivers know how to handle a bike lane on the right hand side so I don't think it should be thought of as a negative aspect for vehicle or transit access/ mobility. Need to make bike lanes visible to cars, sidewalks less ideal. Safe bike blvds. Or lane with buffer. This is difficult to choose as details of a potential "cycle track" on the wider sidewalks were not provided. Comparing the bike path and non-bike path plans is difficult with this info missing. First and foremost I believe we need dedicated bicycle lanes. But where the bike lanes are provided for, if lane widths can be narrowed to provide more sidewalk, that is my next personal priority. I cycle on this stretch of Willamette 4-6 days/ week and do most of my shopping there, using a bicycle and a trailer. Please consider the needs of aging citizens. We walk, use the bus and our children and grand children bike and bus to work and school. The auto continues to be importat but is not the most important. Willamette between 24th and 29th has wonderful businesses who would thrive under improved conditions for pedestrians and bicyclists. Bike/ ped infrastructure improvements = increased business! If you are looking at Portland for a bikeway, what happens southbound to get from 27th and 29th, no way you can expect cyclists to climb the hill on 27th to Linclon. Oak North doesn't get you to 24th either. Getting cyclists and pedestrians in the other area will increase business. Goal decrease cars, increase bikes and pedestrians. Top priorities in my opinion (in this order): Safety; bike lanes; cleaned up sidewalks, as wide as possible; fewer, wider driveways. Please become aware of your language use, it has real life consequences: capacity, volume are making yourdesigns but always refering to automobiles that then have to be accomodated. What about taking biking/walking capacity as your guideline? Bike and ped only and public trans (bus). With a town as progressive as Eugene, start a greenway where cars have to go around. Pedestrians and bikes and trees will have the right of way. There would be space for parades, "street" performers, musicians, etc... Improve street trees. Improve bike ways in the area. Both as a driver and a cyclist, I prefer having a curb between cars and bikes. I have some concerns re: mixing bikes and pedestrians on the sidewalk, although that already happens on the existing 6 ft sidewalk. I believe left turn lanes would please businesses and car commuters to continue car flow. However, I am a pedestrian hoping to be a bicyclist visiting these businesses (already a bicyclist) if it were safer. Fewer lanes would make it so much better to cross. Wider sidewalks make a real visual impact that the area is being slowed down... also potential for beautification, which should be positive for businesses. Prevent driving on sidewalk with trees? Landscaping? Bicycle parking that doubles as preventing cars? Envision Eugene ideas for furniture, etc. I understand that VMT are decreasing, suggesting that we should weight non-auto needs higher than current usage to better prepare for the future.

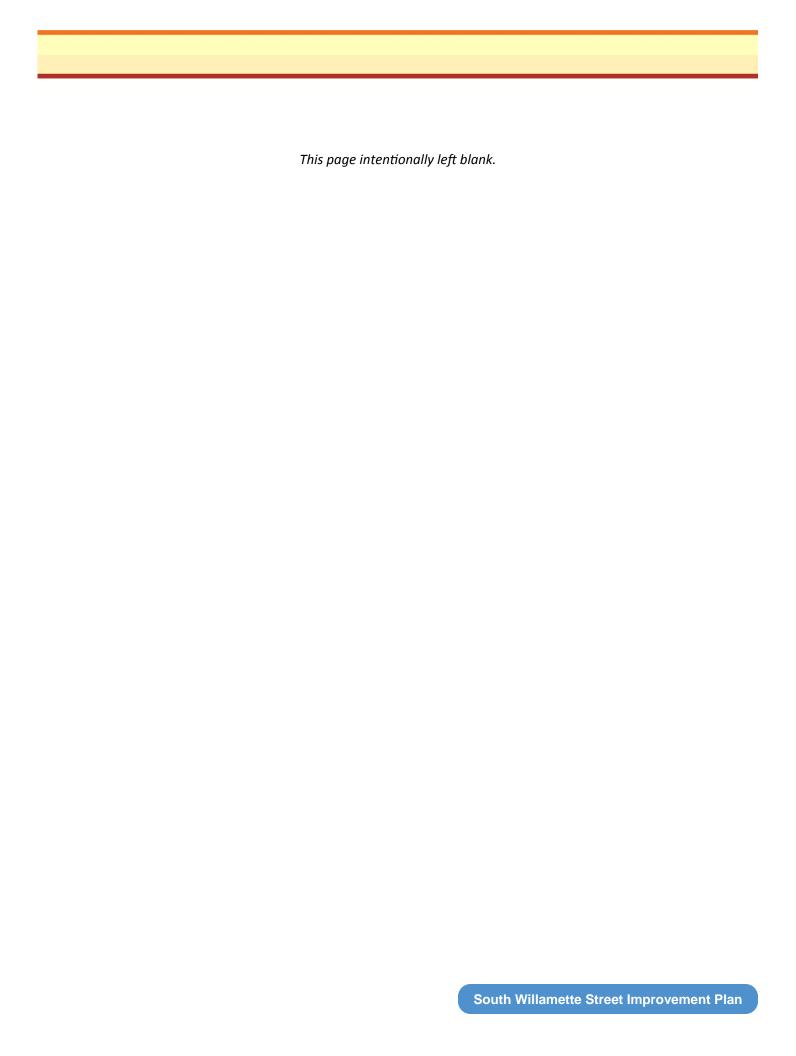
I own a not-yet - open business at 25th and Willamette and my customers (99% of them) drive to my bird seed and nature store to buy their bags of seed. So I'm concerned about a loss of convenience to drivers who are my customers. And I also ride my bike to and from work too! But I do avoid riding on Willamette. I am concerned about fewer people to my business area will make an impact in customers to my shop! This area of Willamette is not a great bike neighborhood to me the most important is to keep our special shopping area vital. I walk a lot to the shops -sidewalks are important. The section at Woodfield Station needs special consideration. Please question the need to have a left-turn lane. It takes a lot of the very limited avaliable space (percentage wise) for a benefit that is high conflict for lots of user groups includign competing left-turners from the opposite direction vehicle lane, In these budget crunching times, it is difficult to choose options without knowing costs. Budget consideration -Improved Willamette corridor will increase real estate prices enough to The "cost effectiveness" screen for evaluating alternatives should consider the cost of operating vehicles -not just the cost of building the infrastructure. Police enforcement on 18th to 20th to keep bikes from riding wrong way (N) on the one-way street (I bike, but I also drive -don't want to fill anyone!) Compare design scenarios to similarly designed streets in Eugene with similar traffic counts. Compare lane widths for autos and bikes (and sidewalk widths) to help the public assess options. Carefully assess intersections and provide this data/ design. Use economic development research on different street redesigns and how the public investment drives market value. Look to PDX 2040 Growth Plan for examples. This would greatly diminish likelihood option 1 and 2 pens out for the Envision Eugene plan for long-term growth/ change. These should be considered jointly and not separately. Both in the Northern corridor, but especially in the Southern structures will be replaced! And they will be denser! (See SE corner of 24th/ Willamette) Also: a younger demo increase could substantially increase bike usage!! More energy needs to be spent on envisioning how these alternatives will affect the beyondstreet realm: parking, business store fronts, residential, etc. Robin addressed this briefly, but if this is what we want, why not integrate it now? These are critical to discuss now not in the distant future. If we want complete streets, we need to think in a complete way. "Capacity" as a cup is a false analogy. The most important criteria are how attractive the area is for people to live, walk, work, etc. not how many cars and bikes it supports. Thinking about this project as a transportation project is a too narrow of a perspective. Reducing speed is a very positive outcome for most, if not all stakeholders. We should consider reducing travel lanes from 11' to 10' to slow traffic and increase sidewalk width.

At a first meeting on this subject, I and others on our table suggested using Willamette as a one way street (coupled with Amazon). No mention has been made of why this would not work. All plans were to be talked about tonight. Not so, I see. There should be talk and studies on future transporation needs on Willamette. The planned apartment buildings will add a great many cars to the road. Give us the future planning figures. We need to know the "capacity" of each plan. The side street bikeway idea will hopefully involve some way to get on Oak down the alley at the very North end and also how to get from Portland directly into Southtowne without carrying it down steps. I'd like alternate bike street option or improved alleys. Oak and Portland streets are viable routes for bicyclists. I am very concerned about bicyclists' safety if bike lanes are added in 3, 4, or 6. Bike lanes, parallel to Willamette St. would allow cars to drive to their businesses. The older generation living in the South area may not be biking as frequently as the University areas. I'm in favor of bikes on alternative routes or cycle tracks sharing sidewalk. The combination of cars and bikes on this opens a lot of safety issues, bikes and crossing each other, multiple driveways, etc. Concerns for economic loss, cost, and esp. safety -fire trucks? Police? Busses? Like the sidewalk width. Not sure 3 lanes can work in option 5. Although I support improving and increasing bike access and facilities, Willamette is a major arterial and I don't think there is adequate space to add safe, dedicated bike lanes. I support improveing bike routes on Oak and Portland and alleys. I also would like to see bus turnouts. If there was an option to get additional space beyond the 60 ft I would like to see sidewalks wider than 9ft. Definetly need to reduce number of driveways. When I bike, I prefer not to bike on busy streets anyway, even with bike paths. If Willamette was pretty, if people could walk and gather there, if businesees integrated smoothly w/ the pedestrians, the notion of café's, places to sid, accomodation of bicycle traffic are wonderful and Willamette would stop being a blighted, ugly, car motivated place. Raised sidewalks at In-Shape. As a business owner I am very much looking forward to the 'upgrading' of Willamette Street to make it an attractive -tree lined- no pot holes- area that attracts families, singles, students, etc. to come to our area. Lighting can be a problem. Wider sidewalks create safer space. If sidewalk next to traffic lane, maybe some form of protective buffer to protect pedestrians. Mid block crosswalks should be considered in front of Market of Choice and around the Holy Cow area. There are big distances between cross walk for peds to travel. The traffic light for Woodfield Station would be great for pedestrians, like across 29th coming out of Market of Choice and Asian Market. Please provide all background data prior to the next meeting/ decision.

There seems to be a distorted representation of bicycle advocates. The general public is far less prone to use bicycles to shop or commute in this area when there are perfectly flat areas
immediately to the east. It seems irresponsible to cater to the minority at a significant
disadvantage to the majority -with a blind optimism as to the economic consequences, and a
failure to tudey where traffic uses Willamette to get to and whose neighborhood it would
congest if we lost lanes.
 Please take these surveys with a grain of salt at the "more" road biking community seems to be
more represented at this meeting.
Get Mark Gillem involved.
A horse designed by a committee looks an awful lot like a camel.
Evaluate alternatives for safety. How does it impact safety compared with the current reality.
Safety has to be part of evaluation.
Drainage is hazardous on many sections of the northern corridor!!
I assume all alternatives would improve drainage and prevent water pooling at curb and
pedestrians being splashed. Not mentioned.
The transportation model to analyze alternatives should account for the trend of decreasing
 VMT -or at least not assume continued growth in VMT.
We cannot give up maximum traffic capacity for this critical travel route. We do need to
establish a plan that will extend R.O.W. as soon as possible in the future, so at least sometime in
the future, space would be available to make bike lanes and sidewalks as it seems people want
 but we don't have room to do it safely now.
 Carefully account for changes in traffic patterns in nearby residential streets. Provide this data.
Assess nearby auto routes to accommodate thru commuter traffic (similar to your assessement
 of alt. bike routes). Spend money to fix overall problem.
 I wish substantial traffic could be diverted from Willamette to Amazon Parkway.
 Amazon Parkway should be an alternative for traffic N and S.
Suggest adding in other traffic-calming, especially at intersections. For example, intersections
and or crosswalks that are just slightly raised, different color, and or different types of pavement
 such as bricks.
 Traffic needs to slow to support urbanization.
 The assumed 30 mph speed limit doesn't seem compatible with sharrows.
Are there any studies on whether 3 lanes (one middle lane turn lane) slows over all traffic -
otherwise I have concerns about people still going too fast (as they do now) when trying to
 "commute" through) with fewer lanes for traffic -hence more dangerous.
Sharrows would depend on car speed and density work better if cars go slower and not so many
 of them.
Clarify how much travel time would increase when stated. Is it significant?
Roads should not be designed around enforcement issues -such as vehicles not letting busses
 pull back into lanes.
Though it cannot be considered within the scope of this project, pullouts for LTD buses are desired.
 North bound bus stop across from Woodfield Station needs a pedestrian crossing: people are
 sprinting out into the roadway now.

 Make public transit more affordable/ desirable to reduce vehicular capacities.
We're concerned about bus pull-outs.
I have a big concern about bus stops. Big delays unless have a pullout, but how would that
affect sidewalks?
Option 3 is too narrow for busses.
Transit pull out alley -utilities reduced traffic from more walking/ biking.
Spend money now to bury utilities -it will cost more in 10 years!
In the long term, the overhead "utilities" must be buried.
Utility poles are a hazard for bikes and pedestrians.
Left turn signals are very much needed.
Although I am in favor of any option that fits the needs of many people -with safety at the
forefront- I feel it must be said that bike and pedestrian traffic occurs mostly in the summer -the
rest of the year is too cold and rainy for most people to walk or bike.
I really like alternative 6 but am concerned about the through traffic volumes. Then again if we
make it easier to walk and bike than drive, people will be more likely to choose those modes.
This would be beneficial in lessening the traffic volume and improving the atmosphere. I
imagine that would be the best environment for the businesses but more difficult for through
traffic. The way I see it, someone who is new to town and has not decided how they are going
to transport themselves, should be able to look at Willamette and see a representation of our
community values and unbiased options of how to use the street -it needs to work equally well
for vehicles, bikes, and pedestrians and busses!
I chose [the options I did] because they are the only complete streets and consider the safety
and social equity ideals that our community values.
All businesses applying for permits to expand or reconfigure their buildings or parking facilities
should be put on notice that in the future the City may choose to widen its right or way and
would be unwilling to compensate the businesses for any losses resulting from design changes.

SECTION G COMMUNITY FORUM #3 SUMMARY



Meeting Summary Community Forum #3: Evaluate the Alternatives

June 11th 4:00-5:45 and 7:00-7:45 pm South Eugene High School

Overview

At the final of three community forums, participants reviewed the transportation study results regarding three alternatives for improving Willamette Street between 24th and 32nd Avenues, asked questions, discussed and shared their preferences. The study aims to help South Willamette Street be a vibrant urban corridor accessible by bicycle, foot, car, and bus. The area includes residential, commercial, and mixed uses, and six intersections over about a ¾ mile stretch. The study area is currently forty-two feet curb to curb, has sixty feet of right of way, no bike lanes, and irregular sidewalks with more than seventy driveways.

Two well-attended meetings were held on June 11, 2013 to accommodate the high level of civic engagement, filling the large cafeteria at South Eugene High School with 275 participants. DKS Associates presented their study findings and answered questions. In small groups, participants discussed what new information might influence their preference, and then reconvened as a full group for a thoughtful, structured discussion of the options. Surveys of participant opinions were collected at the end of the meeting.

Meeting

Chris Henry, City of Eugene Project Manager, explained that this project is part of a larger land use planning effort, "Envision Eugene," that plans for the 20-year future of Eugene's population and employment. The district around Willamette St. is the subject of a Concept Plan dealing with infill and redevelopment opportunities. The Street Improvement Plan project aims to develop a complete street redesign plan for an active transportation corridor, where people can walk, bike, access transit, drive, and access the area's businesses. The plan aims to support the area's existing businesses and the commercial district's vitality, create a balanced multi-modal transportation system, further City planning efforts to identify compact growth and redevelopment opportunities, and foster a well-informed community supportive of the plan.

Scott Mansur, DKS Associates, explained the inclusive process that led to selection of the three alternatives for deeper analysis being presented, and explained that in the autumn of 2013 the plan will be presented to the Eugene Planning Commission followed by a presentation and recommendation for action to the Eugene City Council. To see the presentation PowerPoint of study findings visit the project website at <code>eugene-or.gov/SWillametteStreet</code>.

Ellen Teninty, Cogito, facilitated clarifying questions prior to breaking into small groups for an opportunity to think about what participants had heard and listen to one another's views. Cogito then facilitated a process of large group discussion coordinated with survey feedback. See Attachment A for discussion notes, and Appendix B for survey comments.

In addition to the meeting, the survey was available on-line for the following week. The following pages are the separate survey results of (1) the June 11th meetings, and (2) the on-line survey.

Survey Results

The project developed a survey to gather public input on the impacts of the three remaining design alternatives for the South Willamette Street Improvement Plan. Survey questions were designed to gather public opinion on the results of the traffic analysis conducted by DKS Associates for the City of Eugene. The goal of the traffic analysis was to understand the impact on motor vehicle traffic of each alternative. See page 12 for a copy of the survey.

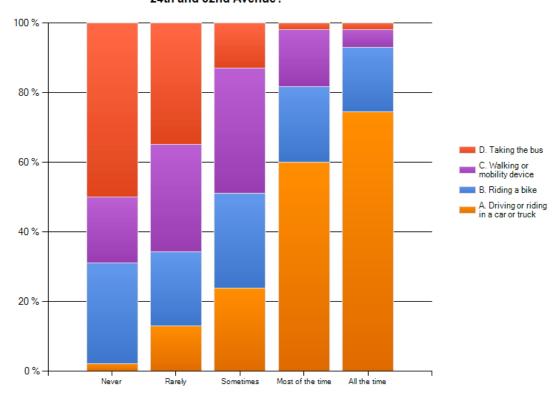
To develop the survey, the project team reviewed the results of the traffic analysis, discussed the content for Community Forum #3, and identified areas where public input would be valuable to decision makers. To view the results of the traffic analysis, visit the website documents: Technical Memos #7 and #8.

The survey was conducted at both Community Forum #3 on June 11th and online for a 7-day period following the forum. Because some individuals chose to complete the survey at both the forum and online, the results are compiled separately. Out of approximately 275 people who attended Forum #3, 223 completed surveys. 394 surveys were conducted online. Forum participants benefited from a presentation and group discussion, while online participants relied on graphics built into the survey. Survey completion rates were very high: only 4 surveys were incomplete at the forum, and 12 online surveys were incomplete. The surveys are unscientific and the results do not represent the demographics of the community:

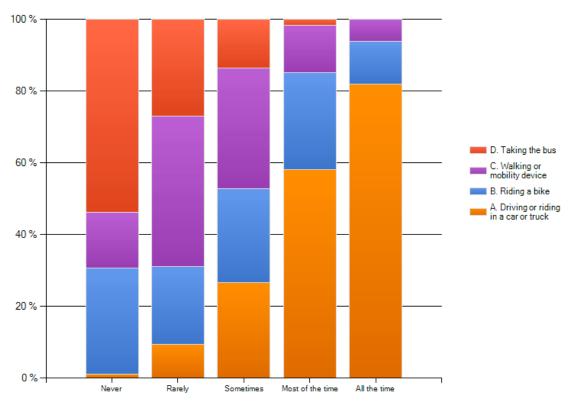
- Both the forum and online respondents were significantly older than the median age in the area and youth were not well represented. According to the City of Eugene Neighborhood Analysis, about 20% of the population in South Eugene is over 60 years old. In contrast, 43% of forum survey respondents were over 60, and 30% of online survey respondents were over 60.
- Both surveys showed strong representation by individuals who shop on Willamette Street, and significant representation by people who live in the immediate area. There was low representation by businesses and individuals who live South of 32nd Avenue: out of 394 online responses, only 36 people who own or work at a business completed the survey.
- Specific questions about traffic signals (Q4), delay (Q5), and traffic shift (Q6) received less support from online participants than forum participants. This could be because the online participants did not benefit from the forum presentation and discussion.

In the following pages, survey results are organized sequentially by question: the first graph shows responses at Forum #3 and the second graph is the response from online participants.

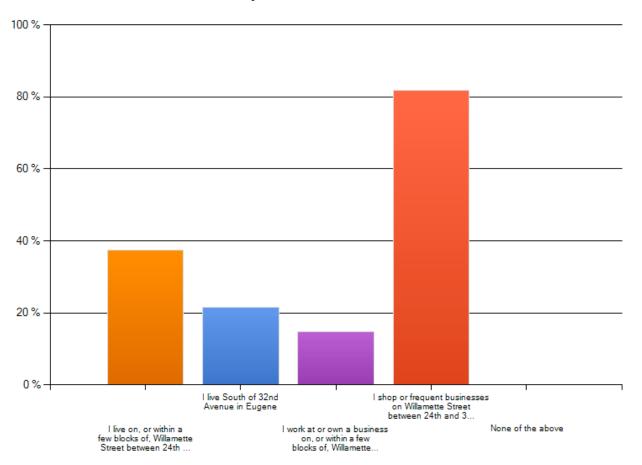
Q1 Forum: How do you currently travel Willamette Street between 24th and 32nd Avenue?



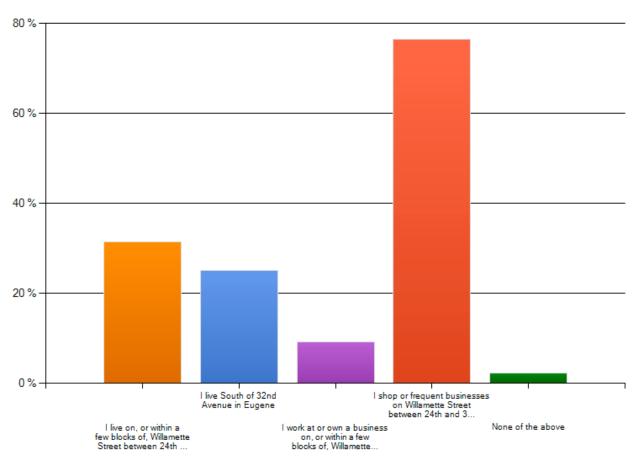
Q1 Online: How do you currently travel Willamette Street between 24th and 32nd Avenue?



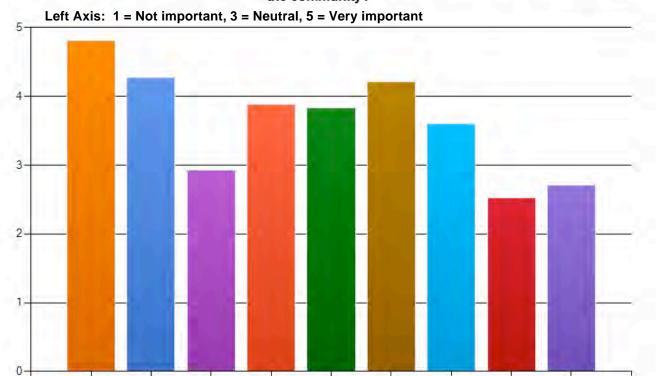
Q2 Forum: What is your connection to Willamette Street?



Q2 Online: What is your connection to Willamette Street?



Q3 Forum: In your opinion, how important do you think each element listed below is to the community?



Q3 Online: In your opinion, how important do you think each element listed below is to the community?

e.Bicycle routes

on parallel streets

f.Improved/more

pedestrian crossings

g.Sidewalk amenities

(such as cafe

seating, larger trees)

h.Lowest project cost

iLeast

construction impacts

d.On-street

bicycle lanes

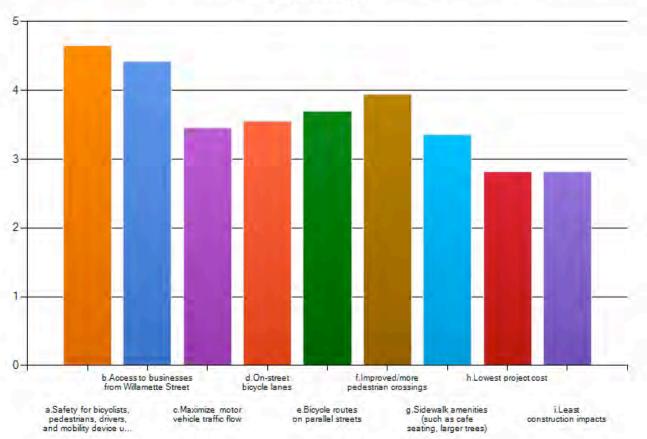
c.Maximize motor

vehicle traffic flow

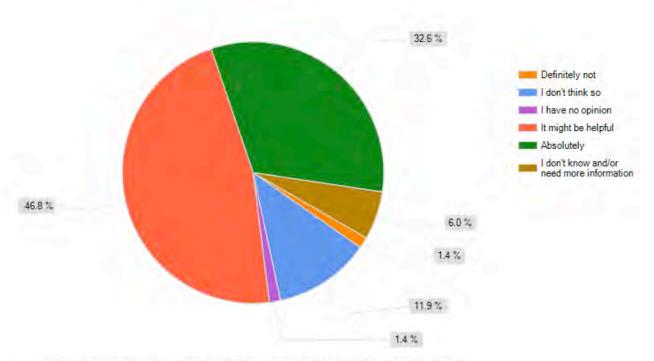
b.Access to businesses

from Willamette Street

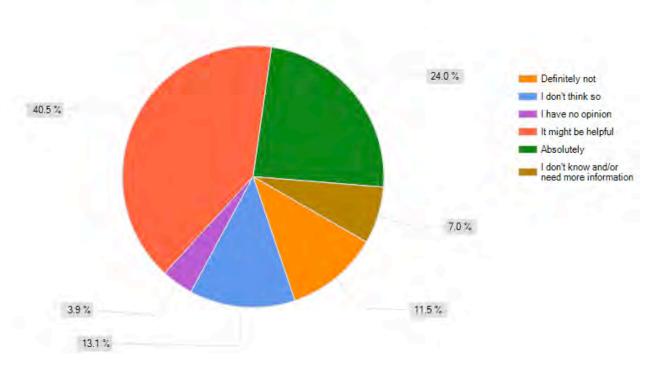
a.Safety for bicyclists, pedestrians, drivers, and mobility device u...



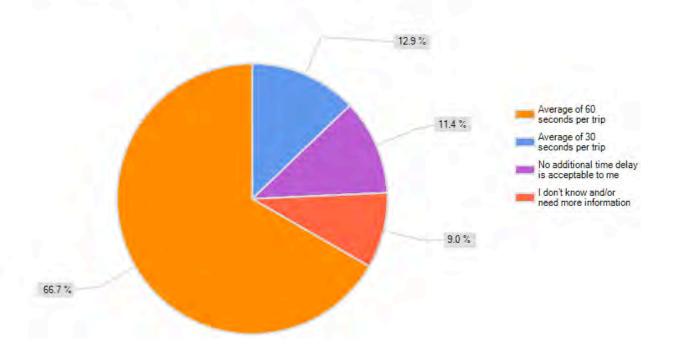
Q4 Forum: A new traffic signal on Willamette at the Woodfield Station (Market of Choice) driveway between 28th and 29th Avenue could provide better access for turning vehicles and a safer pedestrian crossing opportunity. This would likely mean closing some business driveways on the east side of Willamette and designing alternative accesses. Should the City install a traffic signal at Woodfield Station and Willamette Street?



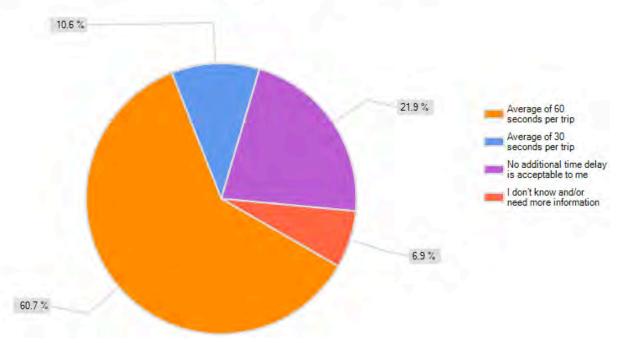
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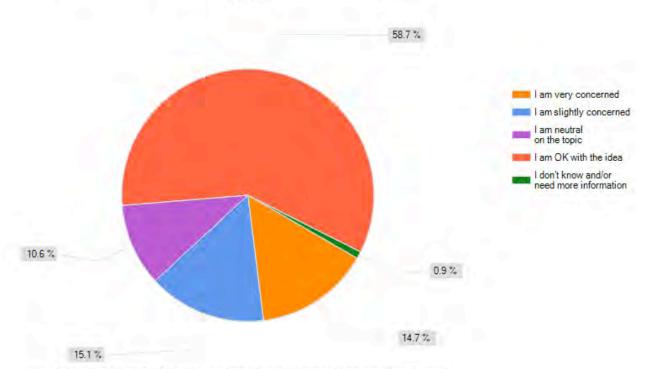
Q5 Forum: Corridor Function: Analysis shows that Alternatives #3 and #5 will increase delay along the corridor. The projected average increase in travel time during evening rush hour in 2018 would be about 30 seconds longer per one-way trip than Alternative #1. When driving Willamette during rush hour, how much additional delay is acceptable to you?



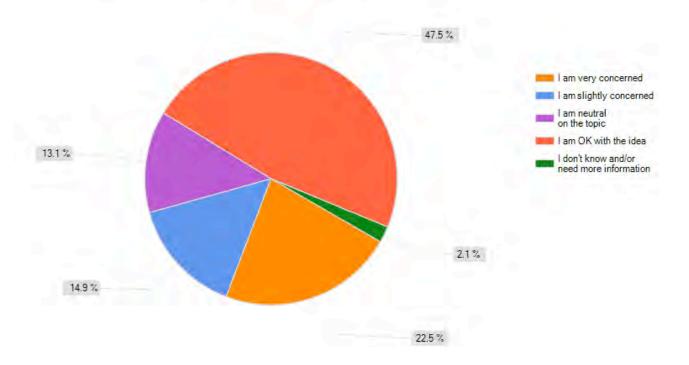
Q5 Online: Corridor Function: Analysis shows that Alternatives #3 and #5 will increase delay along the corridor. The projected average increase in travel time during evening rush hour in 2018 would be about 30 seconds longer per one-way trip than Alternative #1. When driving Willamette during rush hour, how much additional delay is acceptable to you?



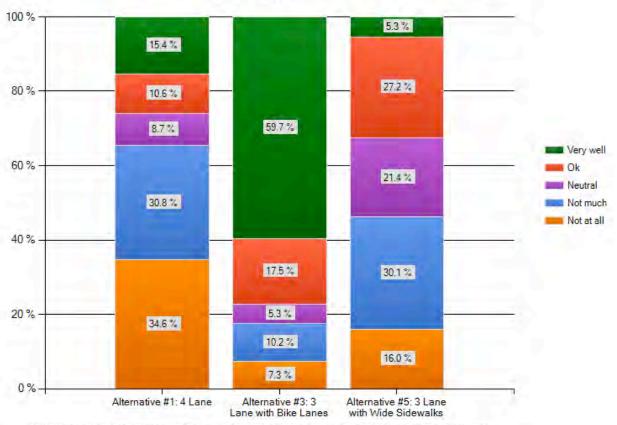
Q6 Forum: Traffic Shift: Alternatives #3 and #5 may shift a small portion of Willamette Street traffic to parallel streets during the busiest hour of the day (4:45 pm-5:45 pm). Of the traffic shifting away: •About two thirds would use to streets east of Willamette, such as Amazon Parkway and Hilyard. •About one third would shift to streets west of Willamette, such as Lincoln, Jefferson, Adams and Polk. How concerned are you about traffic shifting to parallel streets?



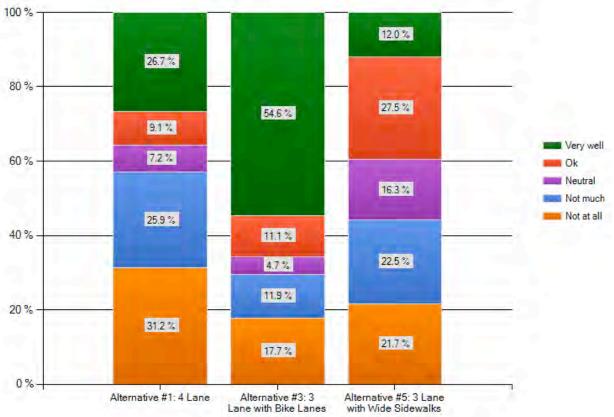
Q6 Online: Traffic Shift: Alternatives #3 and #5 may shift a small portion of Willamette Street traffic to parallel streets during the busiest hour of the day (4:45 pm-5:45 pm). Of the traffic shifting away: •About two thirds would use to streets east of Willamette, such as Amazon Parkway and Hilyard. •About one third would shift to streets west of Willamette, such as Lincoln, Jefferson, Adams and Polk. How concerned are you about traffic shifting to parallel streets?



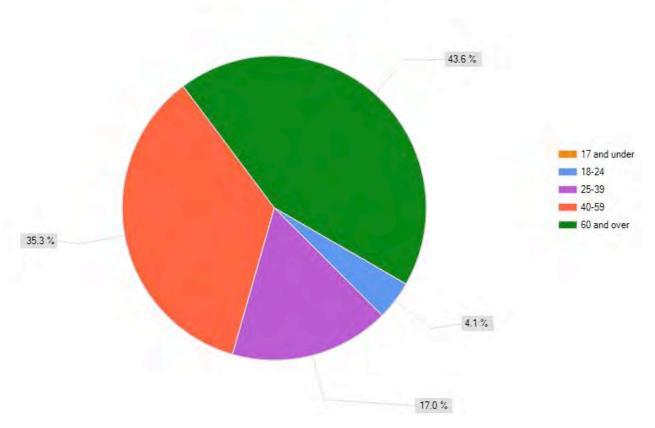
Q7 Forum: Public opinion is one of several factors that the City will consider in the final decision. How well do you think each alternative meets the needs of the community?



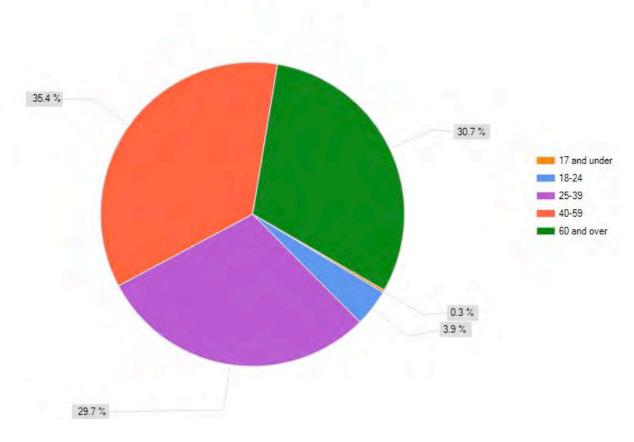
Q7 Online: Public opinion is one of several factors that the City will consider in the final decision. How well do you think each alternative meets the needs of the community?



Q8 Forum: What is your age?



Q8 Online: What is your age?



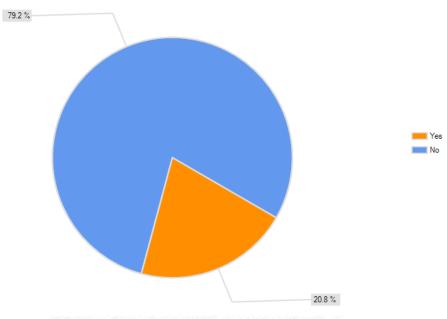
Q9 Forum: What is your gender?

Male	54.2%
Female	45.8%

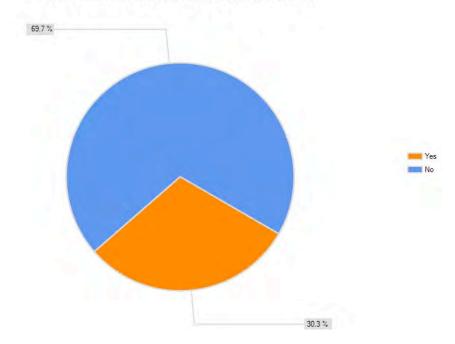
Q9 Online: What is your gender?

Male	50.0%
Female	51.1%

Q10 Forum: Do people under 18 live in your household?



Q10 Online: Do people under 18 live in your household?



The South Willamette Street Improvement Plan will explore options for people to easily and safely walk, bike, take the bus, or drive in an eight-block study area from 24th Avenue to 32nd Avenue. The goal of this study is to help South Willamette Street become a vibrant urban corridor accessible by bicycle, foot, car, and bus. The project developed 6 alternatives for Willamette Street and chose Alternative #1, #3, and #5 for further study. This survey will help the project team understand public opinion about the 3 remaining alternatives.

How do you currently travel Willamette Street between 24 th and 32 nd Avenue?	Never	Rarely	Sometimes	Most of the time	All the time
A. Driving or riding in a car or truck					
B. Riding a bike					
C. Walking or mobility device					
D. Taking the bus					
E. Other:					

2.	What is your connection to Willamette Street? (check all that apply)
	☐ Live on, or within a few blocks of, Willamette Street between 24 th and 32 nd Ave.
	☐ Live south of 32 nd Avenue in Eugene
	☐ Work at or own a business or commercial property on, or within a few blocks of, Willamette
	Street between 24 th and 32 nd Ave.
	☐ Shop or frequent businesses on Willamette Street between 24 th and 32 nd Ave.
	☐ None of the above
	□ Other:

3. In your opinion, how important do you think each element listed below is to the community?	Not Important 1	2	Neutral 3	4	Very Important 5		
A. Safety for bicyclists, pedestrians, drivers, and mobility device users		_			_		
B. Access to businesses from Willamette Street	_	_	_	_	_		
C. Maximize traffic flow							
D. On-street bicycle lanes							
E. Bicycle routes on parallel streets							
F. Improved/more pedestrian crossings							
G. Sidewalk amenities							
H. Lowest project cost							
I. Least construction impacts							
J. Other:							
4. A new traffic signal on Willamette at the Woodfield Station (Market of Choice) driveway between 28 th and 29 th Avenue could provide better access for turning vehicles and a safer pedestrian crossing opportunity. This would likely mean closing some business driveways on the east side of Willamette and designing alternative accesses. Should the City should install a traffic signal at Woodfield Station and Willamette Street? Definitely not I don't think so I have no opinion Station and Willamette Street? I don't know and/or need more information							
5. Corridor Function: Analysis shows that Alternatives #3 and #5 will increase delay along the corridor. The projected average increase in travel time during evening rush hour in 2018 would be about 30 seconds longer per one-way trip than Alternative #1. When driving Willamette during rush hour, how much additional delay is acceptable to you? Average of 60 seconds per trip Average of 30 seconds per trip No additional time delay is acceptable to me I don't know and/or need more information							

6.	 Traffic Shift: Alternatives #3 and #5 may shift a small portion of Willamette Street traffic to paral streets during the busiest hour of the day (4:45 pm-5:45 pm). Of the traffic shifting away: About two thirds would use streets east of Willamette, such as Amazon Parkway and Hilyard. About one third would shift to streets west of Willamette, such as Lincoln, Jefferson, Adams ar Polk. How concerned are you about traffic shifting to parallel streets? I am very concerned I am slightly concerned I am neutral on the topic I am OK with the idea I don't know and/or need more information 					: Hilyard.
7.	Public opinion is one of several factors that the City will consider in the final decision. How well do you think each alternative meets the needs of the community?	Not at all	Not much	Neutral	Ok	Very well
Α	Alternative #1: 4 Lane					
В	Alternative #3: 3 Lane with Bike Lanes					
С	. Alternative #5: 3 Lane with Wide Sidewalk					
8.	What is your age? ☐ 17 and under ☐ 18 – 24 ☐ 25 – 39 ☐ 40 – 59 ☐ 60 and over					
9.	What is your gender? ☐ Male ☐ Female					
10	. Do people under 18 live in your household? □ Yes □ No	?				
11	. You are welcome to share additional comm	ents or qu	estions. Pl	lease be cond	cise and to	the point.

Please turn in survey before you leave the meeting. If you need to turn it in later, deliver it to Chris Henry, City of Eugene Public Works, 99 E. Broadway, Eugene, OR.



Community Forum #3 Discussion Notes

GENERAL QUESTIONS

Audience member: You were discussing the queuing that happens when people are turning. So if we have the alternative designs that include the designated turn lane (Alt 3 and 5), how will that increase flow, as opposed to the long period of time waiting in the queuing as we wait for someone to turn?

Chris Henry: Well it depends, if you are turning left- the alternatives 3 and 5 provide a turning lane and you have a place to be out of the travel lane to do so. The queuing in a lane, as referred to in the presentation, is when the traffic signal turns red, how many cars stack up. And in alternatives 3 and 5, you can expect the length of those queues to double over what it is currently. You have two lanes of cars that would be going into one lane.

Audience member: (Follow up question) This would be at the signals, not turning at the signals?

Chris Henry: You will have a left turn lane to turn from and to.

Audience member: Have you considered placing the bus stops so there is no left turn adjacent to the bus stops, so it would be much easier for people to move into the center turn lane to get around the bus?

Chris Henry: That is a possibility. We have not yet looked into that level of detail.

Audience member: I planted trees along Willamette Street and alternative 1 shows the tree on the outside, next to the street. It's not adequate to do that because of trucks. How are you going to get that tree out there without it losing all of its limbs? Chris Henry: This is just a conceptual drawing; it's not the actual design. The idea is that there will be an opportunity to have trees. Somewhere on the sidewalk we will have the opportunity for trees. That is not an exact location for the tree.

Audience member: At the previous community forum, you said that Willamette Street between 24th and 29th experiences about twice as many accidents as one would expect on a street that capacity and volume and traffic. How many fewer accidents do you expect for each of these three alternatives?

Scott Mansur: In alternative 3, when you go from a four lane to a three lane, you see a lot of reduced travel speeds. You would expect about a 30% reduction in your crashes along the corridor. And you would see similar reduction in alternative 5 like 3, as compared to alternative 1.

Audience member: Regardless of which plan is chosen, is the implementation going to be in 2018 or before that time?

Chris Henry: 2018 is the projected year that we would get bond funding for the preservation project.

Audience member: 18th Avenue is a 3 lane with a center turn lane, so what's the traffic count on that street compared to Willamette, and has the city looked into how the two compare?

Chris Henry: The volumes on 18th Ave. are higher. We have just over 16,000 vehicles a day on Willamette St. while 18th Ave, depending on where you look, is about 20,000 vehicles per day.

Audience member: Research shows that when you have a more controlled environment with vehicles, there tends to be more access by pedestrians and bicyclists, which reduces the amount of traffic by 10-20%. Which of these figures did you use? Scott Mansur: All of the traffic volume evaluations are based on the LCOG model traffic assumptions. We applied those and saw some traffic diversion but we are going to still see some growth on Willamette St. Not a lot, but we were not seeing the reduction. Audience member: How many people have told you that they choose not to walk along Willamette St. now because the sidewalks aren't wide enough? [Audience laughs]

Chris Henry: We've heard that concern, its very inhospitable for pedestrians. You can walk there but if you encounter bicyclists, it is uncomfortable.

Audience member: Back to the vehicle queuing slide, did you account for cars that were turning left or did you say just assume that the lines are going to double? **Chris Henry**: It is based on how many vehicles are trying to go through the signal and the light turns red, everybody stops. That's what the queuing was based on, its not so much about left turns.

Audience member: As usually being a pedestrian, I realize that no one enforces the 25 MPH limit. When I called about it, they said, 'Well, no one is really going much over 30 [MPH].' You really do notice the difference in the sound of vehicles and the feeling that I should belong here or I don't belong here. Are the people who run these things going to enforce 25 [MPH] as a limit in any of these plans or are they going to tolerate the 5 miles over?

Chris Henry: Law enforcement is a conversation to have with the Eugene Police Department [EPD] or with their funding by the Eugene City Council. There is a traffic enforcement unit. The speed limit is 25 MPH. In alternatives 3 and 5, compared to the existing condition, the most prudent driver on the road sets the speed. We expect speeds to drop because of that.

Audience member: With the sidewalk being widened in all the alternatives, are you intending that cyclists and pedestrians be using the sidewalks together in alternatives 1 and 5?

Scott Mansur: By city ordinance, you are allowed to ride bikes on sidewalks. It's not something we are real excited about, having cyclists on the sidewalks.

Chris Henry: We do not encourage people to ride bicycles on the sidewalks. In fact, in the downtown the Eugene code prohibits it. When we talked about widening sidewalks, alt 5 is the one with real widening opportunity to go from 9 ft. to 13 ft. The potential in alt 1 and 3, the existing condition is to realize the full width – 9ft. In many cases, that has already been built- in other cases, it has not.

Audience member: Can you walk us thorough the historical ADT? I guess you guys have been studying it. Has there been a drop in the ADT? I guess through the last five or six years throughout the city and state they have been seeing drops. So I'm just

wondering what your reasoning and what you have been using as the average annual increase.

Ellen Teninty: Can you say what ADT stands for?

Chris Henry: The Average Daily Traffic volume, the number of vehicles traveling on the street in a 24-hour period. This data was collected in 2011 as a part of our transportation system plan. It was factored up to 2013 and 2018 for this analysis. With a modest growth based on Lane Council of Government Metropolitan Transportation Model, the volumes have been relatively flat since 2008. We haven't seen the amount of growth we have seen in decades past.

Audience member: Now I am concerned about the walking on Willamette Street. I do that all the time and it's perfectly easy to walk down to 24th street. It's that as soon as you go north of 24th, you risk life and limb. Bikes do come barreling down that walkway. And I really don't understand how you think you can keep bikes from riding on the sidewalks and being a threat to pedestrians.

Chris Henry: Well we are not prohibiting bicyclists from riding on the sidewalk.

Audience member: The city council adopted a goal to reduce fossil fuel use by 50% by 2030. And one would assume that alternatives 3 and 5 would lead to a decrease in fossil fuel use by encouraging pedestrians and bicyclists. Is that being taken into consideration in making this decision, this goal?

Chris Henry: Yes, that is part of the policy context that the Eugene City Council needs to consider in making this decision here. Creating a balanced multimodal transportation system provides people opportunities to walk, bike, and ride the bus in addition to motor vehicles. It helps achieve that goal.

Scott Mansur: Just to add to that, we are using that in the criteria that we are using to evaluate the alternatives.

Audience member: I wonder if you could elaborate on the impact of wide sidewalks on businesses along Willamette. I'm thinking of like Turtles, Eugene Hardware, and Down to Earth. That their parking lots are awfully close to the streets and I would imagine that increasing the width of the sidewalk would impact those businesses.

Chris Henry: The sidewalk in front of Eugene Hardware is a full 9 ft.

Audience member: (clarification) Well alternative 5...

Chris Henry: It is towards the center of the roadway, not outward. There are other locations, like in front of Turtles where people are parking in the public right of way now. Increasing the sidewalk width over there would mean that the cars would be parked on the sidewalk or they wouldn't park in those two adjacent spaces.

Audience member: Hey, I have a question about safety. So I am a mom and I ride my bike with my two young sons on Willamette Street to go to businesses. As I am looking at alternative 5, I am wondering what would you say to someone who is biking with young kids where it is safe to bike on Willamette, in that scenario. Is it the sidewalk? Is it the road? That's something I would consider and want your input on.

Chris Henry: If you absolutely had to ride your bicycle on the sidewalk on Willamette St, I would encourage you to ride in the direction of traffic. Otherwise, I would suggest that you ride on parallel streets and then move laterally to Willamette St. Riding a bicycle on the sidewalk is not a safe activity. There are many conflicts with motorists that aren't looking for bicyclists moving at that speed. They might be looking for pedestrians, but they're not typically looking for or able to see cyclists. The alternative 5 is offered

because some people said, 'Well, why don't you just widen the sidewalks and bikes can share that space.' It's not something we recommend.

Audience member: First I was wondering if there are currently 16 to 18,000 cars a day using that corridor, how many bicycles do you expect to use it once the bike lanes are in place?

Chris Henry: I can't answer that question directly. There is certainly some demand now for cycling on Willamette St. We did count bicyclists and pedestrians at the intersections during two-hour periods in the morning, day, and evening. But we didn't predict what the future demand would be.

Audience member: It was stated in the presentation that alternative 5 would be best for pedestrians. I understand that's kind of the quick presentation. But given the curb will still be right up against the sidewalk in this alternative. And there will be lots and lots of bicyclists in both directions in alternative 5, and that all those bicyclists would essentially create a 8 or 9 ft. space for pedestrians. And there is even talk of adding a bikeways or lanes onto the sidewalk as a potential down the road. I am wondering how is it that then this is the best alternative for pedestrians?

Scott Mansur: That was applying a national standard for multimodal level of service. So when you apply that multimodal level of service based on that width, it says that this will provide the best level of service for pedestrians. But this didn't really take into account the limitations that come with that. So take that with a grain of salt.

Audience member: You mentioned that it would take up to 30 seconds longer to go up 29th Ave. Does that take into considerations left turns on the 4 lane alternative? I've been stuck behind cars many times waiting to turn, and that very often adds more than 30 seconds of wait time.

Scott Mansur: Yes this does for all the alternatives.

Audience member: (Follow up) Then a different question is, on 27th Ave. the slide you showed, showed only one car making a left turn to get on to 27th in the alternative 3. It seems to me that there is more than one car at any particular time waiting to make a left turn.

Scott Mansur: That was just to show the queues, it wasn't the number of vehicles represented in that figure.

Audience member: (Follow up) Well, it seems to me if there are more cars waiting to make a left turn that would make the queues shorter.

Scott Mansur: That's true and the left turns were removed and left out of the queuing evaluation.

Audience member: Thank you for providing the multimodal level of service. It seems like we are using some different terms though. In the results that you found, you noted you found increased speeds in alternative one for motorists, and increased comfort for bicyclists in alternative three. And I missed what was mentioned for alternative five. So one question is, is there increased comfort for motorists and is there increased speed for bicyclists and pedestrians in these alternatives. So specifically I am thinking, like pedestrians crossing a five-lane road versus a three lane road. Is there increased delay on someone on a bicycle using the alternative routes versus using Willamette St? **Scott Mansur**: So unfortunately, there is not a great tool for measuring travel times for bicyclists and pedestrians on this corridor, especially when there are no bike lanes there today to measure where we are.

Peter Coffey: I think that it would be fair to assume for through cyclists that having bike lanes from 32nd to 24th on Willamette St. will be the fastest route but we have simply not quantified that. Did that answer all your questions?

Audience member: (Follow up) Also in terms of pedestrians you know in the delays for crossing wider streets? Just so that we know the distinction of the terms using delays and comfort.

Peter Coffey: Right, and the crossing differences is not in the lengths, the distance is the same in alternative 1 and 3. We haven't widened the cross section.

Chris Henry: The multimodal level of service tool is to gauge the user comfort in the corridor. And for automobiles, this is based on delay. So its not apples to apples for all these comparisons and it has its limitations.

Audience member: So did your study project the price of gasoline in the year 2018? **Chris Henry**: We did not.

Audience member: My understanding is that level of service, the letter grades don't actually mean good or bad. It depends on what is happening in the corridor. So the level of service with a letter grade D, doesn't necessarily mean nearly failing, it just means it's slower. The merchants are getting businesses because the traffic is moving slower to see the businesses. Is that right?

Chris Henry: That is perhaps a subjective interpretation. What it does represent is a range; each letter grade represents a range of so many seconds of delay for motor vehicles. From the user perspective of pedestrians or bicyclists, and transit riders, it would be similar to their comfort level in the corridor.

Audience member: Twice in the presentation alternative 5 is more comfortable for pedestrians. I wonder if that takes into account that bicyclists will most likely be using the sidewalk in that alternative. As a person who walks down that street daily that is not comfortable for me. So I was just wondering if that was taken into consideration. **Scott Mansur**: It was initially clarified that when we looked at the pedestrian multimodal level of service was the result that it provided and I know that is one of the limitations in the software is that it doesn't account for the fact that there is no bike lanes there. So the conflicts that would be created between bicyclists and pedestrians weren't part of that equation. It's a tool we used but as we go through the process, it is going to have to be updated because we saw a lot of limitations through that.

Chris Henry: This is an opportunity for me to say something here, I forgot at the beginning of the meeting to mention that in regards to sidewalks, and what happens the curb-line and back. Later this summer, separate from this process and more related to land use, will be another community meeting, not scheduled yet, but the discussion will be about what we do as the area grows regarding public right of way next to the businesses and properties.

Audience member: You talked a lot about the different alternatives and their analysis at their level of service. But I am wondering if you could speak to the analysis of safety? Scott Mansur: If you remember at the last community forum, we said that this Willamette St. corridor has about 50% more collisions when compared to other similar facilities and streets like this in Oregon. Alternative 1 would keep it similar to what we have today. Alternatives 3 and 5, by removing the two travel lanes we reduce the corridor speed as well as providing other amenities. Studies have shown if we were to implement alternatives 3 or 5, you would typically see a range of reduction in collisions of 10-30%. So over the last 3 years of data, there were 72 collisions within our study

area and 26 of those were at private driveways. In alternative one, you will see lots of rear ending collisions because of conflicts between left turning vehicles and driveways and through traffic. Alternatives 3 and 5 would provide the most improvement in safety as you look at reducing the collisions along the corridor.

Audience member: I heard your bring up the alternatives 3 and 5 which I think you were talking about the alternative routes the cars would be taking because of the slowed street, over to Hilyard, Lincoln, or Jefferson in either direction. You were just talking about the cars and the change in vehicle traffic. Did you bring up or did you look at the increase in revenue the bicyclists or pedestrians would bring to Willamette, were there to be alternative 3 or 5?

Chris Henry: Not necessarily. There have been recent studies that bicyclists and pedestrians do spend more than motorists at the businesses but you really need to get down to the details of those studies to get to what they really mean. What we did look at was where would vehicles divert to in cases of increased traffic or congestion. Those results came from the LCOG Transportation Model.

Audience member: I'm curious how much detail has been given to the alternative bike routes into the making of these maps. Some of these have some tricky things here. I think some of these were hail marys in the bike/ped master plan because of a lack of a corridor route along Willamette. The worst of the intersections is on 29th and Oak, especially if you are on 29th and taking a left turn on Oak...the question is, what are you going to do for the improvements for those crossings and then still get us to the businesses?

Chris Henry: There is more work to do for the development of the parallel routes. They have been identified in the Pedestrian and Bicycle Master Plan but they have not been prioritized for re-development yet. The next steps are to identify traffic, way-finding signs, and lane markings in the pavement. Also we are looking into some kind of crossing treatment, could be a pedestrian hybrid beacon which is a flashing red signal like what was shown on 29th place. We just haven't gotten to that level of detail.

Audience member: So you mentioned a question about safety. You mentioned that plan 1 would be similar in safety as to what we have today. And it seems like that is not taking into consideration for the parallel bike lanes on Oak, potential for roundabouts-25th for example, and extra lanes at 29th. Couldn't there be gains in safety there that you are not looking at? I mean do you see what I am saying?

Chris Henry: Alternative 1 doesn't add extra lanes at 29th. There are already extra lanes there. 5 travel lanes. One of the most problematic issues with Willamette St., Scott mentioned it, is the driveway accesses. Many of the collisions that happen are because of the numerous driveway accesses. So one of the ways to improve safety in the corridor, regardless of which alternative is ultimately selected, is to really consider the changes to accesses, driveways to be eliminated or consolidated to improve safety.

Audience member: (Follow up) And you mention that an extra light could free up some of the problems and increase safety in plan 1.

Chris Henry: Yes and the traffic signal in front of Woodfield Station could regulate left turns more safely and for pedestrians to cross more safely.

Audience member: The city is planning building quite a bit of apartment buildings along 24th and 29th. What is the amount of anticipated residence in that area and how much more crowded will it get? Not just for the next 4 years but for the next 10 or 15 years.

You are also considering a roundabout- is it going to be the standard size or super size. That area is so crowded right now, would a roundabout handle the irregular traffic? **Chris Henry**: I think the anticipated growth is just a few hundred new dwelling units. Those new residences are included in the modeling for the transportation. So they are accounted for in this analysis. As far as roundabouts on 29th Ave, in order to accommodate the volume of traffic there, it would have to be a multi-lane or a two-lane roundabout. It would have an effect on all the corner properties so we are suggesting not doing that, although it could be done.

Audience member: In your presentation you talk about the impact on EMS so models 3 and 5, did you study that? And how will that impact the emergency system?

Chris Henry: Alternatives 1 and 3 had the same curb distance of 42 feet. Talking with the fire chief, they're concerned that motorists will not pull into the bike lane to get out of the way. Alternative 5 does narrow the roadway and that does cause some concern for not only emergency vehicles but also for Lane Transit District.

Audience member: Speaking of safety, wouldn't front-end collisions increase with alternatives 3 and 5 with people turning in the middle lanes both ways? **Chris Henry**: Perhaps, but the data doesn't support that. It shows a reduction of collisions between 10-30%.

SURVEY DISCUSSION

Question 3 regarding what is most important to you

Audience member: So in 3C, you say, "maximize traffic flow." Is your definition of "traffic" just cars or does it include human beings that might be walking or bicycling? **Ellen Teninty**: There are other questions for biking and walking. This one is about cars.

Audience member: So same question, 3b. Access to businesses--Is that for car drivers or everybody? Or...bikes?

Chris Henry: Motor Vehicle access.

Audience member: Could you define sidewalk amenities?

Chris Henry: Sidewalk amenities could include decorative street light poles to illuminate the street. You might consider the removal of the overhead lines, relocating the utilities out of the corridor. Planting street trees could be an amenity. Including a bicycle rack in the sidewalk. Benches. Green storm water treatment that filter the water. Vegetation. Landscaping.

Audience member: My primary concern with this route is for the cyclists coming through Willamette St. from 18th Ave. I'm looking at the bicycle routes on parallel streets and I know that taking Oak street involves going through at least two more traffic lights and taking a significant detour as a cyclist. And Portland St. is not a through street that offers any useful ability commute northbound from this area. So I am curious, are there any substantial improvements to those bike routes that will improve my experience as a cyclist commuting between Willamette St. and 18th?

Chris Henry: The Eugene Pedestrian and Bicycle Master Plan identified bike lanes on Willamette St. but not how to do that. And that is in part why we are here. The parallel routes were identified in the map as bicycle boulevards. Those treatments could include traffic calming, way finding signs, and shared lane markings. It's a bike symbol with two

chevron signs so bike share the road with motor vehicles, indicating to both drivers and cyclists that both will be in the lane at the same time.

Audience member: Shouldn't the bike lanes be considered pedestrian amenities? **Chris Henry**: They could be, the bike lanes would create more separation between pedestrians and the motor vehicle traffic. Unfortunately the multimodal level of service model doesn't factor in that separation, interestingly.

Audience member: 3B, where it says to rate the bicycle routes on parallel streets from very important to not important... I am wondering why that's on there and wouldn't there also be a place on this list for car lanes or automobile lanes on Willamette St. as not important or very important? I say that because the automobile lanes exist on Willamette St. and they will be on Willamette St. and that's a given. These parallel routes for bicycles that we are talking about exist already. And they are there now, and they have been there for the 23 years I've lived in Eugene, and they will be there for another 23 years. So we are not talking about something that is not important or very important, it just exists. I want to make sure that everyone knows that. We are not talking about some new amenity that will preclude the need for a bike lane on Willamette St. This is not that. This is something that already exists and does not work to get you where you want to go on Willamette St. And the question is, you can just stretch it out because it really makes no difference to the topic.

Chris Henry: To respond to the earlier part, 3C, maximize traffic flow, lets you express the importance of traffic flow.

Audience member: One more comment, presuming that we go ahead with option 1, with 4 lanes of traffic. How likely is it that, 15 years from now that we go through this process to develop bike lanes? How expensive would it be, at that time?

Chris Henry: If we don't make a change on Willamette St. now, we've made a statement about how we like it as it is and want it to stay the way it is for some indeterminate amount of time until we ask the question again. I can't predict when that might be.

Audience member: I didn't understand on 29th and Willamette when you are talking about extra turn lanes, does that mean you will have sidewalks or bike lanes or not? **Chris Henry**: At 29th there are two southbound travel lanes and a left turn lane and one northbound travel lane. And bike lanes in alternative 3 or wide sidewalks in alternative 5. **Audience member**: (Follow up) So you would remove a sidewalk? **Chris Henry**: No, we would remove a travel lane. There are currently 5, and we would go down to 4.

Audience member: I am neutral about three or four lanes at this point. Because I own a business there and that business will be closed by the time this occurs. What I am very concerned about is why nothing is being done about it right now. I am hearing that as of the year 2010, we had 50% more accidents than the state rate. I lowered the speed limit 12-14 years from 45 MPH TO 35 MPH by getting people to sign a statement. Then I easily got a \$250,000 stoplight put up there on 25th. Why are we not talking at all about how we can slow traffic right now? Is that impossible? Is that outside the realm of this? Is that outside the realm of the city? Why have we not been in stakeholder meetings or in one discussion about how we can slow traffic, and how we can make it more livable if we can make people go 25 MPH, which IS the speed limit? Anyone here not know right now that is the speed limit? It's changed quite a bit. People are now starting to know. If we can do that, we can have a clean slate to see what the possibilities are. Pardon me, but

let's wait five years to do that? I've had to pick two people up off of the street. I am ready to get out there right now. If you have a business on Willamette or if you use Willamette, call the police right now and demand that they are out there taking care of the traffic. If we are going to have bicycles on Willamette, the bicycle cops stop at 18! But when I talk to Pete Kerns or Kitty Piercy, the next day, they are out here. I am just one person. I make seven bucks an hour running my business. I don't want to wait another five years. Pardon me for being a little radical about that, but you pull a kid out from underneath a car in front of your business... you do that one of these times. I'm asking everyone here, let's be community about slowing it down. You got a business on Willamette, put a red 25 MPH speed limit up. So do something so there is a downturn in the collision rate in our town. Put that up. Let's make it a nice place right now. It will make it easy to be a bicycle friendly place in five years.

Audience member: One thing that helps make decisions is knowing the impact this has on our community's health. One thing that this helps is the impact and just coming for a sustainable and holistic approach. And how the street can help our local economy and community.

Audience member: Regarding the buses, you are not going to do pull outs. So the thing to do is to stagger the bus stops. We have enough of a center lane there and people are allowed to go around the busses like the police said you could. And you put on the back of the bus, a signage that says, 'When this bus stops, you are allowed to go around the bus using the center lane.' So use the staggered bus stops and driveways. That's what you ought to do!

Chris Henry: The challenge is that there are many driveways and it's hard to find a place without one. 18th Ave. is the same situation with fewer turning and driveway accesses but the same situation otherwise.

Audience member: Any businessperson will tell you that they first need potential customers then after that, actual customers. This is often calculated by the amount of cars passing by during the day. If we survive the construction process, and if Willamette St. is reduced from 4 to 2 lanes, and if anybody says 3 lanes, there are only 2 traffic lanes. Potential customers will find other avenues and loss of business might occur. Loss of business equals less potential rent, which makes property values stagnant. If the city wants more money from property tax, they should support the alternative that has less cost to property values increasing. We hope that if we build it, they will come. From an economic standpoint, this is far from true. I don't believe that for one instant. We have built it, and it has four lanes and it's thriving.

Audience member: Eugene kind of prides itself as an environmentally progressive city. We even have a climate action plan. I know that we take steps to be more sustainable. And it is just kind of perplexing to me, that given this culture, it is so difficult to even get bike lanes on a major street and encourage people to get out of their cars and bike more.

Audience member: It comes from the perspective of being an older person who unfortunately, because of physical conditions cannot ride a bike anymore. So I do the best I can by bussing and using a Prius. But what its all about in my mind, is looking to the future. And if we don't understand, global warming on a personal level, as well as larger corporations, we are missing something extremely important, especially for our younger people. As someone who can't take advantage necessarily of a bike lane, I am looking to little children and their families and I want safe places for those kids to be. I

don't want all those wonderful businesses to suffer but I think the people who love those businesses will still going to be there. But please let's think about what the consequences are, that are maybe to some people are way out there. But we are already seeing some now and we need to make change.

Audience member: Scott, when you came to our table we asked you a question to explain some similar situations. I just ask you to reiterate some of those experiences and some of the concerns that business owners in other cities involved in similar projects and their reactions?

Scott Mansur: One of those projects I worked on, was a similar project on E Street in Washougal, Washington. The volumes were much smaller about 9 to 10 thousand. There were a lot of concern from businesses and property owners. We got a lot of feedback. We went to city council and shared our findings. Council supported that project and we got a lot of positive feedback since that project was done. Businesses had actually been pleased with the results. Emergency services had been very concerned but they were now very happy with the way things were operating and movement of travel. And the police chief shared a comment that they rarely write a ticket on E street and they rarely have to go deal with accidents on that corridor. Another example in Oregon City had a higher volume with 19,500 vehicles and they were also very pleased with those results over there.

Question 4 regarding a traffic signal at the Woodfield Station driveway.

Audience member: So I have a question for you guys. Are you only considering only a traffic signal at that intersection or are you considering some other form of control that would enable pedestrians to cross more safely?

Scott Mansur: Both. Both would be considered.

Audience member: (Follow up) How do we answer this question? Do you want us to assume that both would be considered? Because I will definitely say, not for one thing, and absolutely for another.

Scott Mansur: The intent is, what we have heard, from a motor vehicle standpoint, pedestrian/bike traffic signal would be desired. If we were able to look through the design, we need more detailed analysis is going to be needed to determine what the signal would be needed.

Audience member: So, if you put a traffic signal in there, would it be activated by cars? By pedestrians? Could it be synced to other traffic lights that are so close to it so that it doesn't automatically go off and make sense from other angles?

Scott Mansur: Yes, 100% would it be coordinated with other traffic signals along 29th so that as a pedestrian you could push it and you would wait and the lights would be synchronized with Willamette St. and side streets. It would be served simultaneously to help serve traffic.

Audience member: The projected volume of the street, whatever it was, how do you factor in a traffic light at Woodfield station anyway? Do you have estimated interruptions into the traffic flow down Willamette? How do we figure that into traffic flow? **Scott Mansur**: So we have a traffic model that was created with existing signals and we've provided an analysis with a signal at Woodfield station and analyzed the flow with that signal.

Audience member: (follow up) So the current delay in these options up and down Willamette in the current model, that factors in at the Woodfield Station light, or does it not?

Scott Mansur: It does in each of the alternatives though. Each of the alternatives, when we talk about the travel time delay, each of those alternatives has a traffic light at Woodfield station assumed so it would be equally impacted.

Audience member: I think my question was just answered. You've already studied this and it is a possibility. Because when Woodfield Station went in, we discussed it with the traffic department and a light being there and they said it was impossible to do. Is it now been studied enough that we know for sure that it can go in?

Scott Mansur: There are a lot of different variables. We have done preliminary evaluation but we will need a more detailed evaluation. That still needs to take place. Part of our process is to install a traffic signal you actually have to meet national standards. There are a few more steps in the process to confirm if the signal is a viable option at this point.

Audience member: (follow up) So it's a maybe now?

Scott Mansur: It's a maybe.

Audience member: Is it possible to have a temporary light there and study it and see how it works before we do it permanently?

Chris Henry: A lot of things are possible. We have considered that as a possibility. We are considering the possibility of a trial. But we haven't landed on yes or no.

Audience member: The question says it would be closing some businesses' driveways on the east side of Willamette. Why does it say that?

Chris Henry: Typically you don't want driveways immediately adjacent to the signal, it's harder to control the traffic that way. Ideally, we would eliminate driveways. While we haven't gotten to the level of detail yet, we have identified it as a concern.

Audience member: Have you considered that the traffic might back up into the intersection of 29th and Willamette?

Peter Coffey: Yes, that is a significant concern. We are still looking at howo to make it work effectively. So it's still a work in progress.

Audience member: Did you ever think of just closing the driveway to the shopping center and just keeping the one on 29th open?

Chris Henry: Typically, that is not popular. A lot of things are possible but businesses are entitled to give their customers access. How they achieve that access is a matter of discussion.

Audience member: To me this turn out of that access onto Willamette is quite scary, particularly when traffic is busy. So if this is the best alternative to come out with, I think it is absolutely necessary to do something, so I support it.

Chris Henry: It's scary for pedestrians and that's where the signal comes into play and provides opportunity for pedestrians to cross safely.

Audience member: I have a quick question about measuring delay. Is that for peak hours only?

Chris Henry: Yes, during the PM Peak hour.

Audience member: (Follow up) So what is the change in the delay in the rest of the day for 23 hours?

Chris Henry: This is something DKS is working on to be able to describe what the other times of the day will look like, off peak and how it changes throughout the day.

Audience member: You know there is a bank or something that is a little north of Woodfield entrance. Technically it is connected to that parking lot. Is it possible to make the light further north?

Chris Henry: We looked at that, we talked with the people who owned the Woodfield Station property and managers of Market of Choice and some businesses. Ideally, it would be help and it would be nice to move the intersection up north. However, we have no firmed up plans and it would take work and talk with more multiple property owners.

Audience member: Would it be possible to have the traffic light there but only operate at peak traffic times?

Chris Henry: You could do that but traffic operations like predictability. And things that aren't always working aren't always noticed and that reduces the effectiveness for the users.

Question 5 regarding motor vehicle delays in alternatives 3 and 5.

Audience member: Is there any data on the new redesign on 29th, going west and the impact that has had on the traffic delay?

Chris Henry: The operation of the signal as it is today, is included in this analysis. **Audience member**: (Follow up) How did that change from a year and a half ago when you put it in?

Chris Henry: We have bike lanes now and its different for motorists. It's the consequence of creating options for mobility in the corridor. The consequence for adding bike lanes is, that it has made it a little bit more difficult for motorists moving east and west through 29th.

Audience member: As I understand the level of service and delay times, those are all concerning the 1-hour peak PM flow. Any idea what happens the rest of the 23 hours of the day?

Chris Henry: That is what we are working on. DKS is in the midst of doing that analysis and looking at off-peak effects to help answer that question an describe what it might be like on other times of the day. Right now we are showing the worst-case scenario here. And as 29th Ave. in alternatives 3 and 5, show Level of Service E, that is 1 second of average delay past the threshold. That is a policy question for the Eugene City Council, whether or not they want to accept higher levels of congestion along the corridor. And they can do that like they have in the downtown core. It's a very small amount of delay.

Audience member: I would like to see an additional box here say that, "Delay not a problem for such short distances." We are not queuing on a turnpike for 10 miles. We are going through a six-block section. For me, when I drive, I expect delays there. I expect delays in this very short, congested section. I would love to see this as an alternative.

Audience member: With options 3 and 5, you talked about a 30% decrease in traffic accidents. Does that take into account the slowed down speed into the picture?

Chris Henry: The operational models don't consider the crashes effecting mobility. That is something you might consider for recurring congestion on a freeway but not for this model.

Scott Mansur: A lot of the case studies used for the 30% reduction all showed reductions in corridor speeds, which is a lot of the relationship to having fewer collisions, especially in the severity in injuries. Slower speeds reduce life-altering crashes.

Audience member: I am noticing that for this question, especially in the impact on commute time for cars, I am wondering if you had considered what the commute time will be for cyclists?

Scott Mansur: That's not something we have looked at, at this time.

Chris Henry: As Scott said earlier, the multimodal level of service tool is relatively new, hasn't been used yet- until now in the City of Eugene. It has some limitations and one of them is that it doesn't predict some of those things.

Audience member: Just a quick opinion, and that is that the 30 seconds delay equals safer working roadway for most people and less – because things are slower- less accidents, I think that would make a lot of sense. And if I am really in a hurry, I would choose a different way to go.

Audience member: Since a very large number of people in this audience here are people who either do or choose to ride a bicycle through there if they had that opportunity, and the current delay on the street is a 20 year average for most of the people if they think about bicycle riding. It seems like there should at least be a discussion on the question- even though I understand that you might not have all the tools to do the multimodal level of access study...there's gotta be some consistence, constant recognition of how bad it is for pedestrians and bicyclists right now. And that could improve tremendously. If we are talking about a 30 second delay for automobiles, the increase in speed in which bicyclists and pedestrians could get through there could quadruple. So my question is will you be able to include information along those lines about what it does for everybody when you present this to City Council and when you put it out there on the web?

Chris Henry: Not much more beyond what we have already said. We have said that alternative 1 is good for transit and motor vehicles, alternative 3 is good for cyclists, and alternative 5 is good for pedestrians. Saying much more than that is guesswork.

Audience member: I understand that I am talking to people that are focused on traffic engineering, not city planning, urban planning, or community development. I understand that's what the focus is. If all of us visit other cities and look at areas that we find desirable that are enjoyable to spend time. We don't focus in on the speed of the traffic on a street. We focus on the sense of place and the activity within a street. And typically, that means the traffic flow is very slow. So I would contend that an objective associated with trying to increase the speed of traffic maybe counter to the object of creating a sense of place and creating community associated with the street. Now here's the question: as you were engaged in this assignment, was there a focus or an intention behind that assignment? You know like, what was the problem you were given that you were supposed to solve. Because sometimes that defines how you look at the problem in the first place.

Chris Henry: To clarify, our objective is not to improve the speed for motorists through the corridor. We are simply here reporting the facts of the analysis. Our charge here was to support the land use development and planning work with the transportation system.

The current system isn't working very well for most users and we are exploring options on how to do that. The plan is to be: supportive of the existing businesses, support the area's vitality, create balanced multimodal transportation system- so you can walk, bike, ride the bus, or drive to further land use work for opportunities for infill and redevelopment, and create a well informed community supportive of the plan. Those are our goals.

Audience member: I just had one question regarding the accident rate, we are making a supposition that we are going to have a 30% drop in the rate. I've driven that street twice a day since 1967 till about 5 years ago when I retired. So I saw very little accidents during rush hour. I am wondering when we compare the accident rates, what hours are they occurring? Are they occurring between 10 pm and 4 am? Do we have other factors causing accidents Have you also looked at where the severity of the accidents?

Chris Henry: The crash rates you describe looks at a lot of different factors. It is independent of the time of day. This includes the total number of crashes divided by the total number of vehicles traveling on the roadway for a three-year period. What we saw was, the crash rate that was about twice what we would expect for similar streets in the state of Oregon. The reduction figure that was quoted earlier was expecting anywhere between 10-30% reduction in the number of crashes. Many of those crashes, when we talk about locality are associated with driveway accesses and intersections. People turning create conflicts for pedestrians, bicyclists, and motorists alike.

Audience member: I would like to know if the option of no left turn from Woodfield Station, heading east onto Willamette has been considered. I think that a pedestrian crossing would be very helpful. An example of one I see is on Chambers, for the pedestrian to push the button and change the light. I do travel that stretch a lot and I see traffic hung up because of cars trying to get out of Woodfield Station and head north on Willamette. So I think I would opt for a "no left turn" at that point.

Audience member: I am not quite sure why the question phrased only about traffic delay. It's kind of a biased question.

Chris Henry: You are right.

Peter Coffey: This question is phrased about traffic because we were focused on that alternative and it is geared to measure your sensitivity on traffic delay.

Audience member: My thought is that sure maybe 30 seconds or maybe 1 minute of delay could happen. But it is far more hazardous to pedestrians and bicyclists. People could get into accidents three times more likely according to statistics. That's far more concerning than any traffic delay. I can't see any downside to making it more user-friendly to bikes and pedestrians.

Audience member: I have a question about driveway modification but not getting much detail. Because that would eliminate some of the delays from turning and I am curious about what your thoughts are. What do you want to do about it? Do you want to remove some of them? Have you talked to stakeholders about this as an option for them? Chris Henry: Modifying driveways for access to properties or businesses demand a one-on-one conversation with each of those effected parties. Some of them are dependent on which alternative is selected. We have looked at best practices of what potential modifications might be recommended. More conversations need to be had to prioritize them. We are at a very high concept level here and we will move to the detail and engineer design next.

Audience member: One of the things I have noticed in my completely unscientific observation is that people seem to speed a lot down Willamette. Does the time difference take into account maybe people actually going the speed limit or assuming it will be like the times now where people continue to speed?

Chris Henry: The travel times considered were based on the current conditions, providing an average speed. In alternatives 3 or 5, there is no opportunity to pass other vehicles, so you have to drive at the speed of the most prudent driver on the street. And particularly, speeds are lowered in alternatives 3 and 5 than alternative 1.

Audience member: You talked a little bit about turning at the different intersections. I am curious about how the different alternatives stack up when you are turning left at an intersection. So for example, as a car making a left turn into a driveway, is it going to be easier from one alternative to another to access those businesses?

Peter Coffey: I think there are two ways to look at it. If you are turning left in alternative 3 and 5 you will have the center turn lane to pull into. The other way to look at it though, is when you get in that center turning lane, you will have to yield to on-coming traffic. Today there are two lanes of on-coming traffic, but in alternatives 3 and 5, you only have one. Those would be the two perceptions I have about turning into a driveway.

Audience member: Can you elaborate about what you mean by 'bus pull outs' and the potential for bus pullouts? Because I really hate being stuck behind a bus putting a bicycle on.

Chris Henry: We've heard interest in bus pullouts and shared these views with Lane Transit District. We will need to look very closely for opportunities for them. It effects their operations and their preference is to stay in the lane of traffic or the traffic stream because they have difficulty getting back into traffic. Motorists don't typically yield to them

Audience member: (Follow up) Well, what is a bus pull out?

Chris Henry: A bus pull out is, instead of parking in the travel lane to let people on and off, they pull into the sidewalk space, which would require more space. There are very few opportunities where we could incorporate that without effecting adjacent properties.

Audience member: This is a question for Peter, Peter you said something about in the center turn lane some people worry about collisions. Do you have any statistics on that? **Peter Coffey**: What we see generally in facilities like this is an overall reduction in the collision rate when we reduce the number of travel lanes. And that's a combination of different types of collisions.

Audience member: I'm going to ask the question from a different aspect. How much funding is in place at the present time to implement any portions of this overall project? Chris Henry: Funding is in place thanks to voters passing the preservation bonds to fix streets and bikeways. That's just for the driving surface between the curbs, intersections, and wheelchair ramps. There is a portion of that funding set aside for pedestrian and bike improvements but they are not prioritized. There's about \$500,000 a year over the five years of the bond that hasn't been allocated for this project. Nor has funding for storm water improvements or sidewalk improvements or utility relocation out of the corridor. So we need to first define what the project is that we want. And then we can find the money to realize our goal.

Question 6 regarding traffic shifts

Audience member: So I just had a comment and an observation really. It would seem to me that people who are concerned about the delay are those who are using Willamette as a through route and not intending to stop at the businesses.

Audience member: Amazon parkway was an original road that was supposed to connect to the south instead of all of it going to the east. It was designed with the idea to take traffic away from Willamette St. So shouldn't there be an alternative here saying that it is a good idea to move some of the traffic off Willamette Street instead of the fact that it is only okay?

Chris Henry: The businesses might disagree.

Audience member: So I would like to add to that, I really think that this question should be two parts. Part 1, is about the residential neighborhood streets. Do we think its okay to have cut through traffic in the single-family residential neighborhoods? Part 2, would be the question of, is it okay to divert some traffic to streets that are designated as arterials and intended for that. I think this is difficult to answer as a single question.

Audience member: Eugenians don't change their habits that easily. They still go home for lunch. They did 46 years ago when I moved there, they are doing it now. They're not going to quit going to businesses on Willamette St. just because of some change. They'll complain; they won't change.

Audience member: I just wanted to make note. I didn't know if there was any traffic shift in mode choice. So by adding bike lanes on Willamette St. does that make some people feel safe enough to bike on them or if you widen the sidewalks are people who used to drive going to walk because they feel safer? So what is the traffic shift in our modes? **Chris Henry**: We'd like to know that too, but we don't.

Audience member: I just wanted to point out the biased nature of the wording question that is only referring to the shift of people away from Willamette by car rather than the potential of people shifting onto Willamette by bike.

Audience member: So I was thinking that it requires you to make an educated guess. If there is a traffic shift from 25 to 100 cars at the peak hour, wouldn't it stand to reason that by creating a balance in allowing modes of transportation, you would make up for that the rest of the day?

Chris Henry: How much you make up for that is undetermined. What we do know from other studies is that bike lanes will increase bicycle use by about 30%.

Audience member: The graphic showed that traffic will increase on Willamette St. under the status quo. Is traffic going to shift anyway because people don't want to sit in traffic?

Chris Henry: Perhaps in the long run, people will want to. Generally, everyone's response to congestion and delay is that you find an easier route. But once all the easy routes are gone, they will use the roads.

Audience member: As the population increases and traffic increases, people are going to seek alternative routes in their vehicles. From a bicyclist's standpoint, this is something I practice myself. I seek the flattest routes and Reed says that's what

everyone does pretty much. As vehicles move away, bicycles are going to move in because Willamette has the flattest route. I'm not going to ride my bike over a hill if I don't have to.

Chris Henry: Perhaps rethinking that earlier response that people go through easy routes. People also travel at different times of the day and people will travel at times of less congestion.

Audience member: So earlier, someone asked the question about 18th Ave and the diversion of traffic. I am also curious about the diversion of traffic onto Amazon. Do you know what the shift will be to Amazon? Can you share the numbers for 18th and Willamette and compare them?

Chris Henry: 18th Ave. ranges from 12,000 to 20,000 vehicles a day. Willamette St. averages 16,300 (in 2011). Amazon Pkwy, if I guess around 11,000 cars per day, 9,200 cars per day, and 17,000 cars per day both ways at different intersections.

Audience member: I am not concerned at all. The reason why is, because people who want to businesses on Willamette St. will continue to go there. I think that a lot of what Willamette St. offers is great businesses. We should select a design that supports those businesses. I think that a design that supports people going through is not going to benefit the area.

Forum Survey Question 3: In your opinion, how important do you think each element listed below is to the community? The following are suggested important elements:

bicycle traffic flow

Economic Impact to Business on Willamette

consolidate driveways

Family friendly infrastructure

increasing access for humans and human traffic as opposed to just cars

Movement of people

Other (please specify)

protect residential neighborhoods

reducing number of driveways

reducing number of driveways

sense of community

sense of place

slowing traffic to create a sense of place

"maximizing traffic flow" should include the increase in bicycle and pedestrian travel due to improved conditions (potentially) along the corridor. Not just maximizing AUTO traffic.

cut throughs in residential areas between W.S; Jefferson

middle turn lane

Option for future round abouts

Bus, trolly street car, EMX

better lighting

take down utility poles

Tree canopy

Utilities Underground (Very Important)

Utility Undergrounding

what improvements would happen to oak and portland st?

Online Survey Question 3: In your opinion, how important do you think each element listed below is to the community? The following are suggested important elements:

Add bus stop pull-out areas, if two travel lanes are eliminated.

Bicycles should be able to use the wider sidewalks even if bike lanes get put on Willamette St.

Bike routes on parallel streets make sense IF they have a smooth connection at each end to bike arterials

bike safety, trees, trees, trees

I would bike instead of car if I felt safe. Bikes on sidewalk aren't safe for cyclists oor pedis.

Not clear to me if bikes can ride on the wider sidewalks of the second option.

There are other alternatives for bicyclists; they don't need a specific lane on Willamette Street

We seriously need bike lanes. People bike there already and it's scary for cars and riders Make Woodfield Station business contribute to the cost of the new traffic light/access changes

Make it easier to get in and out of businesses

changes that are appropriate to each individual situation.

I would like to see a wonderful urban walking corridor began here

I would love large beautiful trees on Willamette Street but don't care for the cafe seating so much. I see most sidewalk amenities at encouraging bikes to zip through the pedesterian traffic helter skleter over the sidewalk. Bicycles are vehicles. If you want to truely encourage people to walk, create a very safe walking space like we have downtown with bikes walked on the sidewalk and skate boards in hand. #3 Even a longer delay than 120 seconds is AOK with me - it would give folks a few moments to look around and "see" the hood vs. zipping through it. #4 Amazon Parkway and Hilyard are meant to tak the traffic - so putting extra traffic on them is no big deal. The other streets that you listed are NOT meant to take the traffic. Beefing up the traffic calming on them SHOULD be part of the project.

I'm not sure what C means. Does it mean to make Willamette Street more efficient for the amount of traffic already there, or does it mean expanding to accommodate more motor vehicles than currently use the street? I'm all for efficiency, safety, and inclusiveness for all who use the street and businesses on Willamette Street.

Other (please specify)

Please note these are my perceptions of the community. They do not reflect the importance as I see them.

Protected bicycle lanes, utilities out of sidewalks, roundabouts, and access management Questions "a" and "j" are ridiculous in their presentation.

Reducing number of driveways.

Replace lights with ones that include a left hand turn for vehicles like by Roosevelt Middle Sch

The aesthetics of this area are very important. It is currently one of the uglier areas in South Eugene. Beautification of this area is a high priority to me as a property owner who lives nearby.

there's no straight across the board possibility, as the sidewalks differ in width and trees and businesses need to be considered. There no need for a straight-line approach.

Transforming this section of Willamette to become a unique and diverse place to shop, eat, drink, have fun, where pedestrian and cycling access modes are as important as vehicle modes.

Willamette is THE main N-S spine of the city. Leave it alone!

Bikes and pedestrians are an after thought with existing design-they should be considered important

Improvements that will make it welcoming to non-car/truck modes of transportation Pedestrian and bicyclist safety most important! They are most vulnerable. Other considerations secondary.

To encourage walking, biking, bus use through infrastructure changes is vital but uncomfortable.

As a resident reliant upon the Willamette Street corridor and a former long time commuter biker--I think it's important to balance development with an eye toward realizing bikes don't get the whole city. I can't drive my car on Amazon bikepath or along the river bike path; bikes also don't get every major artery. However, I realize here in Eugene we like to argue things to death. One critical thing that has not been brought up is that of emergency evacuation in case of catastrophic dam failure. At this time, Willamette Street is one of the arteries that could provide citizens with a route to higher ground--deciding to reduce Willamette to 1 travel lane either direction does not take disaster preparedness in mind. If "the big one" earthquake hits, and the dams break, are you going to ride your bike to higher ground or your car? People are going to be in cars and buses because it's most efficient. That's just one example of what is wrong with attempting to take a functional street artery and attempting to clog it by decreasing ease of use for cars.

Feel it would be very unsafe to have bicycle lanes with Ig number of driveways/businesses How many accidents and injuries have there been in the last two years? And on comparable streets?

Ensure vehicle don't move to neighborhoods. Our area does not have sidewalks and already narrow streets

I see few people walking along Willamette 24th - 30th--it is noisy with vehicle emissions--I don't think it is realistic to make sidewalks wider becaue they won't be heavily used. That street is busy enough now and requires enough vigilance driving that to add bike lanes increases the hazards. I'm convinced reduced vehicle lanes would slow traffic flow more than engineers have predicted. have calculated.

Nothing should be done to impede vehicular traffic on Willamette Street. It is already very congested.

VERY IMPORTANT--How is new "improved/controlled" traffic flow (vehicles, peds, & bikes) on Willamette going to affect traffic on other north-south arteries in South Eugene that run through residential areas??

convenient public transportation

Need bus pull outs to avoid blocking traffic.

Need to consider LTD buses blocking traffic and emergency vehicles traveling to South Eugene.

bury overhead power lines on east side and remove poles for aesthetic, safety, and circulation purposes

hide the power lines

Underground utilities

Forum Survey Question 11: You are welcome to share additional comments or questions. Please be concise and to the point.

Alt #1 does nothing to serve people the corridor currently does not serve.

Alternative #1 is not a change except for improved sidewalk. It does not create a multi-model; it does not create an appealing destination to shop and eat and meet fellow community members.

I am a truck driver and I don't believe bikes should be anywhere near a truck. Trucks sometimes need to stop in a lane of traffic (pet supply on Willamette) to load or unload. This is only possible in 4 lanes.

#3 would make me feel better walking with my daughter with bike lane. Now we drive 4 blocs to go to restaurants because of speeding close to us

1. Alternative 3 was stated to be safest (decreases current accidents), and also adds bicycle lanes as well as has the potential to improve sidewalk conditions. This is the obvious choice.

2. I live on 30th, a few blocks from Willamette. I have a small child and a growing family. I currently do not access the 24th - 29th corridor using any mode of transportation due to inhospitability/safety (biking/walking), and congested turning into business driveways. I do now, and will be always choosing biking/walking over car travel. I would like to see alternative 3 built. If it is, I see myself accessing the businesses much more.

A good point was made about alternative 5 tonight. It seems like the ideal option for pedestrians at first glance, but realistically there will likely be issues with pedestrian-cyclist conflicts on the sidewalk, making alternative 3 realistically safer for both cyclists and pedestrians

Alternative #3 with bike lanes will ease access to business driveways and not back up traffic when making a left turn

Alternative 3: I would surrender 1 foot from sidewalk to add 1 foot to outside traffic lanes. Alternative one maintains the status quo, which I don't think is acceptable, mostly from a safety standpoint. Alt 5. will be too restrictive to traffic, given inadequate room for buses and emergency vehicles. Only alt 3. will clam traffic, increase safety, and maintain reasonable access. It provides the BEST BALANCE for the whole community.

As a non-biking pedestrian, and sometime driver, I strongly support option 3 with bike lanes. A 30 second delay is not too much for increased safety for both pedestrians and bikes (and bike lanes increase pedestrian safety by keeping bikes off the sidewalk).

As a pedestrain, I actually prefer alt. #3. For mobility impared individuals, sidewalk cafes, etc. can actually be hazards/obstacles i.e. make being a ped harder. For "abled" (no disability) individuals, alt. 3 is fine + adequet for peds.

At several points tonight it was mentioned that the city wanted to follow a complete streets program. Option 3 seems to be the only one that meets this criteria

Currently, using a bike to access businesses on willamette is not a pleasant commute experience. I remain committed to supporting Alt #3

Dont't forget about kids crossing 24th to get to Roosevelt. Don't forget how hard it is already to cross 24th on oak even in midday. I'm not fond of bikes on willamette. I'd prefer most to use parallel routes. But the fact that bike lanes buffer pedestrians and allow for buses to stop without blocking traffic helps me be more comfortable with Alt #3

Given the goals of this project, the best alternative is alternative 3. Goals: Support Economic Development Balanced transportation share compact development complies with goals of ped/bike master plan

I am strongly in favor of Option #3. We need to be planning for increased pedestrian and bike traffic in order to reduce fossil fuel use and reduce greenhouse emissions. Create more equality for those who cant afford to drive and are choosing bike or walk

I presently drive alternate streets to avoid willamette. Too much weaving between lanes. I would use willamette if we had option 3

I think we missed a great opportunity to create an innovative design. The best option for cyclists, Alternative 3, is much like the current design on 18th. This is adequate for confident cyclists but intimidating for less experienced cyclists. If the city of Eugene is serious about improving the quality of living and adhering to its Climate Action Plan, we need to think outside the box and aim bigger.

I want to decrease the amount of traffic, I want cleaner air and a healthy community and healthy environment. I go for alternative 3.

I would favor alternative #3 if the bike lanes were removed.

I would like to see alternative 3 implemented with a speed limit of 25 MPH strictly enforced or lower it to 20 MPH!

I would love to know how a fire truck and bus could make it through alternative #3 with regular traffic including delivery trucks. How many bikes are expecting on Willamette? Im very much in favor of option #3.

Option 3 has lot of potential for all concerned. Like it! If there were some means to alert drivers upon infringing into bike lane i.e. raised divide, bumps on line dividing bike lane would add another safety comforts! Thanks for great job.

Re Pedestrian comfort - #3 is best since there will be no bicycles on the sidewalk and the bike lane will provide a buffer from the auto traffic, providing pedestrian comfort will increase pedestrian use leading to greater utilizing of Willamette ave. shops.

Although I'm over 60, (and i also own a vehicle), I find that i enjoy riding my bicycle whenever i can i.e. to work, shopping, pleasure, etc.

As a cyclist who would like the option to frequent willamette st and the businesses there, I am very partial to option #2, as I think most of the community is as well. It is the only option that is inclusionary to all users of the area.

Bike options MUST be apart of any realistic plan for the future of Eugene (and the Planet). Eugene represents eco friendly options anything with out a bike lane would be unexceptable and not representing Eugene's bike friendly demeanor that it is known for around the country.

I avoid riding a bike on willamette. I will use designated bike lanes on side streets, then cross to willamette at a cross light. I usually access businesses by walking and then only if the weather is good. I welcome greater accessibility for bikes including bike racks.

I like bike lanes as in option #3 but I have concerns about the comfort of this stye of bike lanes - it woud be more comfortable to have a buffer of some sort between cars and bikes, also I am concerned about buses stopping in bike lane, safety wise as a cyclist.

I patronize businesses in this corridor very often and am a bike rider. The current street configuration makes my visits VERY stressful. Bike lanes would make it more likely that i would visit these businesses

I would love for my children to access Willamette St. businesses by foot or bike on their own, but I absolutely won't allow it with the current design. Plus, Oregon law requires bike lanes. If sewers and utility work will be included, state law requires bike lanes.

I'd prefer a 2-way bike lane on sidewalks on east side of street.

If the alternative bike routes worked there would not be so many cyclists here begging to ride on Willamette. They don't the geography can't be fixed to make them work.

on a bicycle, it works well to use Oak St. to access one business, and cut over to Willamette when appropriate. However, when accessing more than one business, one must ride on the sidewalk currently.

Please put in bike lanes!!

Provide better access for bicycles to use oak and the street next to amazon. No plan is great, there just is not enough space along willamette. I worry about reducing lane width for buses and trucks. The younger generation may feel safe using willamette on a bike, I do not. I have encountered bikes on the sidewalks even where lane is available so I think many of them dont like to ride next to a car.

The city is knowingly breaking state law on a technicality by not providing a bike lane. There is no way to make bike riding on Willamette safe or pleasant. I will never use them (bike lanes). I will always take side streets.

We need to make the decision now to create bike lanes- we won't have this chance again - #1 does not solve any of the problems that are plaguing the street now - its time to stir the pot!

Aren't there statistics regarding the number of people that won't turn into a business because of traffic flow (alternative 1)? Doesn't having a dedicated turn lane increase business?

The business and property owners need people driving cars to have easy access to their businesses to thrive. Economic concerns for the corridor should be uppermost in the minds of all decision makers. Keep the area vital economically by keeping the 4 lanes for cars

You failed to list business access in your final consideration. That is a huge mistake.

In the future, please include information on economic impact (property values, retail sales, health care costs) of active transportation (bike, bus, walk) and health impact of increased physical activity from active transportation. Thanks!!

It would be useful to know how sensitive the model is to increasing fuel prices.

The price of gas will increase in the future due to peak oil. This means fewer motor vehicles on the road. Need to design for alternative transportation now. (e.g. bikes, bus)

1. Rather then a traffic signal on willamette at woodfield station, I would prefer a traffic signal along 29th by the Rite Aid. 2. Chris Henry answered a question about the affect of option 3 on bicycle transit rates through the corridor by suggesting such projections amount only to "guess work". Are there really no data available on cycling rates on sidewalks vs streets?

As the city reduces fossil fuel use, what will be the impact on commute times for cyclists and bus passengers in the corridor?

Focus on building community and avoid making a decision based on a focus of vehicular traffic flows and ADT numbers

For option #5 the wide sidewalk is of doubtful value if there aren't also bike lanes to buffer the adjacent 30 mph car traffic

Good Presentation, Well organized. Enjoyed comments from community, too. Because of bus routes and emergency vehicle use i prefer 4 lanes.

I am concerned about any decreasing width of car lanes that would impact firetruck, ambulance, and bus access. I believe the businesses in Willamette are unique and should not lose business with any of these changes. There should be a major push to have bikes use parallel streets.

I am torn between Alternatives 3 and 5 as one addresses bicycle concerns at the cost of pedestrian concerns and vice-versa. I wish that there was an alternative that did not put theses two concerns at odds.

i like to see information and statistics on the number of people (motorists/bikers/bussers) who commute through this corridor vs. the number who shop, work, or live along the corridor and the potential benefit to having fewer/slower vehicles commuting through.

I own a business and much of our busiest hours are the "Home-ward Bound" shoppers. I also sell 20lb bags of birdseed and my customers largely MUST drive to do business with us! Am torn: I want my business to get busier...but i also DO support Bike/Ped improvements too! So if the 3 (with bike lanes) happens, I'll need a business on the side delivering birdseed to our customers!

I was glad to hear 18th has higher ADT levels than willamette. 18th works well with three lanes and bike lanes. This means willamette would work as well. The increased delay is a tradeoff for improved bicycle and pedestrian environments. It is a delay we should accept.

I would like 3 lanes but no bike lanes. Route them near by on less busy streets. Keep cars moving slowly but moving. Turning lanes are very importent

If we can shift the mindset to wanting to be there and having it not just a pass through feeling.

Increases in delays on Willamette will do 2 things: 1) Encourage economic development - as shown in many oregon communitites! 2) Encourage more people to use alternative modes - reduce "VMT"! Both align with the city of Eugene's goals!

It might be helpful to see a short video of the area in question during these presentations, so we are all aware of the blocks in question here.

Let's think not just the needs for the day, we need a a vision for the next 20 years. Cost/Traffic/Safety = all important. Making the right choices for the next generation, even more important.

Open-Ended Response

Please consider possibilities beyond the right of way although this is potentially litigious, on the east side there is a relatively open corridor that could work!

Please double-side print and make smaller -> less paper! :)

Please include human beings i your definition of traffic. Please provide data on increased safety with 3 lanes. In delay please note amazon parkway is faster for cars right now with 1000s of comments willamette is clearly a top priority. Fix it sooner then five years from now!!

Really like the way this forum was run. I feel better informed and that I am partnering with the solution were cycle tracks considered on willamette for two way bicycle similar to what is on alder st in the university.

S. Willamette is one of only two major arterials to points beyond S. Eugene. Improvements to parallel streets and the existing bike path should be more than adequate for bicycles with access to shops from backside entrances.

Thanks for a good public process

The acceptance of speeding is difficult for me to live with happily. I think police and planners are 1. Male 2. Drive, dont't walk or bus

The city adopted plans and strategies for complete streets. Now is the chance to see these plans through!

The city should present a comparison study between the proposed alternatives #3 & #5, and 18th avenue, with number of cars, number of driveways, accident rate etc. so that we can see whether the three lane has worked historically

The way things are worded in this survey reflects biased assumptions that "traffic" and "travel" = cars only. Bikes are traffic- I personally do not own a car and bike everywhere. We seriously need bike lanes

There is no way to estimate how many fewer cars will use willamette under #3 and #5. the idea that traffic will only be delayed 30 secs between #1 and #3 seems really too little

There needs to be a stated, up front "understanding" that each proposal will benefit some and "harm" others (proportionally)- with each proposal having a different mix of those who end "up" and those who end "down." There can be no one size fits all.

This project would benefit from more examples of similar projects around the country where lanes were eliminated. Also, an operations model instead of a regional model run would be useful.

Vibrant urban corridors should not focus or prioritize auto flow in its study. It should focus on improving the vibrancy which no urban mayor or business owner would ask for fast, rapid auto flow. Option 1 will get you no where and truly makes this practice a waste of community time if it were choosen.

Would it be possible to have 5' bike lanes and 11' through lanes?

You are welcome to share additional comments or questions. Please be concise and to the point.

1. Reduce number of opportunities for left turns, both entering and leaving willamette. 2. Restrict pedestrian crossings, possibly with posts and claims as on agate at student housing crossing

Bike and Peds are a very important part of the street. The Effects of the 3 lane design will only make the street safer to all. "Traffic Shift" will only happen with people who don't care about the business on the street. they only use it as a through street. Bike and Peds will only continue to increase

I feel that if Willamette is more friendly to bikes and peds, then there will less need for drive along the (small section) of Willamette. Five blocks is NOT a lot. On major cities people walk much longer distances. If we CHANGE so will people. In Eugene, because distances are so short, people drive. Please make it easy for people to walk and BIKE.

I want to be encouraged and supported to ride bike and walk to t these establishments on Willamette St. I want a progressive decision on this issue- to help our community to become more healthy.

I'm a strong advocate for increasing alternative modes of transportation. I much prefer to ride my bike to shop and for recreating but don't feel safe with the current configuration on Willamette street. I would imagine that pedestrian and bike travel and shopping would improve if safety modifications were made.

My goal for improving willamette corridor is to reduce the emphasis on car travel and access and increase focus on multi-use and a sense of place! We'll want to come there if traffic is slowed......make it a shopping area

Slowing traffic and adding bike lane, making sidewalk better for peds ----> likely to increase business revenue. Alternative #3 best option to serve model

Slowing traffic and allowing safe movement for bicycles and pedestrians will improve the True economy - the sustainable, long term economy!

Will anyone seriously track 24hr/1wk # of bike, peds, wheelchair etc. on sidewalk and streets plus 3 of buses on/off

1. Bigger/wider sidewalks are not needed. We have at present an aging population. These people do not walk on willamette. 2. Have you studied at three different daily hours the number of people walking?

Shared 13' sidewalk works well on path around river and through amazon park, although I strongly prefer to continue the current configuration with improved sidewalks and 4 lanes

- slow it down - focus on quality of the street and the sidewalk; not on quantity of cars passing through the corridor

Do something to make street safe now, not 4-5 years from now.

I am concerned with Safety and it is not acceptable to rebuild what we know is an unsafe design for pedestrians and bicyclists.

I really think that it is important to talk more about the safety when talking about this project with the community, since this was ane of the major concerns of community members.

It is scary and very unsafe for pedestrians (especially walking w/ my two young kids). It is unsafe and frustrating to drive on Willamette when people stop to turn left. It is also unsafe to walk on sidewalks when bikes are on the sidewalks. You have to be crazy to bike in the street as is and you have to be rude to bike on the sidewalk. If parallel routes are "selected" for bikes, they will still travel on Willamette. Might as well make room for them.

reducing injuries and fatalities is the most important criteria for design.

Safe access to businesses trumps fast traffic flow for the good of the city.

Safety first! Alt 1 and 5 will continue to have bikes ride on the sidewalk, which is an unsafe place to ride. Alt. 3 works for pedestrians, bicycles and gets cars down the road safely. Safety for young people and elders should be a top priority. Reducing vehicular carbon emissions is also critical We need livable communities where it is safe to bike and walk safety is priority #1

Safety should be a deciding point. The design that increases safety for all modes is #3. Also, the only option that allows for ALL MODES to use street is #3. It is important to me that we design the street to MOVE PEOPLE no matter]how they are traveling - bike, walk, drive.

Safety should trump wait time!

The fact that alt 3 + 5 could reduce collisions by 10-30 % was not in the presentation of options. This is a significant omission and a fact the community should be aware of.

The safety aspect of options 3 & 5 reducing accidents by up to 30% far out weigh a 30 second delay for automobile traffic.

Amazon, Oak, Hillyard, Jefferson seem to have much more capacity than Willamette. please consider using civic stadium to connect downtown to amazon

I am quite convinced that a turn lane for left turning cars is a must on this section of the road. Its too heavily travelled to allow stacking in 50% of the road. I'd like to point out also that the assumption has been made in the above questions that all traffic will remain motorized. Given an opportunity to safely walk or cycle, it is likely that traffic may shift that way.

I prefer round abouts. Pedestrian light at 29th and Willamette place (powered by PU) and bike lanes with small barrier.

I would like a super sized round about on 29th and Willamette, taking property from the south side of the intersection only. The intersection traffic is already too much- backing up traffic at this for many blocks. It will get worse - affecting turn off blocks away -

If portland st is an alternate route, please consider a 4 way stop at 25th and portland. Portland is a straight shot now, folks zoom too fast already. Slow down the traffic!! Kitties and human pedestrians are at peril!

No round-abouts please! bus turnouts make sense to me

No roundabouts please!

No roundabouts! Danger to handicapped and elderly. Dont mix bike and walkers. Room for emergency vehicles!!

No roundabouts!! More signing and main crossing

One thing I appreciated is that alternatives 3 and 5 reduced collisions by 10-30%, while only increasing travel time through the orridor by 30 seconds. It seems like a no-brainer! Right now traffic signals are timed to allow left turns onto willamette from Tsunami Books,

Capella, True Value. How would this be affected? I fear negatively

The prediction that traffic would increase by 2018 regardless of the change made seems to multi-gate consensus of the business owners about reduced customers under planes 3 & 5

Traffic flow is quite important. Something to consider is that those future cars, driving down Willamette might be electric or hydrogen powered

YOU MUST ADDRESS putting a signal light by the Chase bank north of the Main entrance to Woodfield. Traffic WILL back up to the main entrance. Change the Chase bank entrance to the main entrance and what is currently the main entrance should be only 2 lanes (entrance and right turn).

All options should integrate measures to deter cut through traffic in adjacent single family neighborhoods. All options should make an effort to encourage efficient use of bigger transportation system, e.g shifting traffic to Hilyard and amazon which are designed to hold more traffic

bus turnouts!!

Concerned about being stuck behind buses on 3 lane options. Bus pullouts would be nice. Like the design modificator if an extra lane at 29th intersection

Aesthetics of the street are very important- plantings, trees, benches, etc. It is not a pleasant place to walk, park, bike, or drive now. Improving this would bring more people to the businesses.

any storm water amenities would be great!

I would lie to see 4 lanes of traffic continue on willamette with drainage improvements, sidewalks widening to 9'. Possibly closing some curb cuts that are doubled up at certain businesses

In the final landscape details, please don't add cutesy, 19th century Americana street lamps as they did in Sammamish, Washington.

Look carefully at multiple driveway consolidations, add planters/street trees with set-back sidewalks, underground the ugly utilities and make it beautiful

Please make it beautiful! Thanks

relocation of utility poles is of great concern to me. Willamette St. will always be ugly until these poles are removed.

Online Survey Question 11: You are welcome to share additional comments or questions. Please be concise and to the point.

Alternatives 3 and 5 are dangerous. Center turn lane likely result is head on accidents and increases the odds of bikes/pedestrians being hit by turning vehicles. 4 lanes with wider sidewalks is better and encouraging parallel streets.

Any alternative other than 4-lanes will result in grid lock and starve local businesses. As an avid cyclist and a shopping motorist, I feel that anything that reduces the flow, ie fewer lanes than 4, will be a detriment to all, particularly when 2 lanes does not allow for stopped buses which will grid it all to a halt. Even with 4 lanes, the lengthy stops made by buses will back traffic up to at least 18th. Amazon as an alternative only works for through traffic and using streets to the west is not at all practical. John Grant 171 W 52 nd Ave Eugene, 97405

I personally think that you are dreaming when you say there will only be 30 seconds delay for alternatives 3 and 5. Just look at the picture in the PDF presentation where you have additional vehicle queuing. Just that additional queuing would take probably 60 seconds at least. Also, people turning right (as into Woodfield station) will still have to slow down, especially with bicyclists whizzing by on the right (yeah, right, they aren't supposed to continue on with someone turning, but many of them do continue, so that is what you have to count on), so the whole traffic flow with just one lane will have to wait for that person, which will just add to the vehicle queuing, which will increase the delays even more. Alternative 3 had bike lanes, but wasn't wide enough for buses and trucks. And it would impede traffic flow way more than you think. Alternative 5 had larger sidewalks, but you'd still have the problem with bicyclists going on the sidewalk knocking down pedestrians or going on the one drive through lane and impeding drivers even more than they do now. Alternative 1 is the only sensible alternative - just make sure that you improve the sidewalks and provide parallel bicycle access. I drive daily between Willamette on 23rd (where my mother-in-law lives, to check on her), shop at Capella Market almost daily (26th & Willamette), and go to my house one block on Willamette on East 30th, so I am guite familiar with Willamette Street. Because of my stops, it is the only sensible route to take.

You show pedestrians walking by the 4 lane option in your illustration, There will probably be few pedestrian if you choose the 4 lane option.

Alternative 3 is the only alternative that provides safe conditions for the whole community. Furthermore, it supports the City's key planning goals and policies listed in the Climate and Energy Action Plan, Pedestrian and Bicycle Master Plan and the Eugene Transportation System Plan. Finally, the opposition to to alternative 3 bases their objections on an unsubstantiated fear of dire impacts on area businesses. However, the literature seems to indicate that the opposite might actually be more likely. In case after case where similar lane reconfiguration (two travel lanes and a center two-way-turn-lane) have been implemented, the resulting increases in pedestrian and bicycle access leads to increased retail activity. And it's not hard to imagine why. By creating a welcoming environment for people arriving on foot, by bike, and by car, a pleasant shopping experience is likewise created.

As a consumer, I am more likely to frequent the businesses along the South Willamette corridor if bike lanes were present. I think Option #3 is the only option that best matches the alternative transportation goals for the city of Eugene.

I think Alternative #3 is most viable from 24th to 27th, but from 27th to 29th on Willamette, there are routine bottlenecks and issues with left turns, access to Woodfield Station and Oregon Community Credit Union. This area should remain at four lanes, with clear markings for bicycles to take 27th on alternative southbound routes. Businesses on the east side of Willamette between 27th and 29th do have alternative access from Oak Street. The other issue is the lack of a northbound bike lane between 29th PI and 29th St on Willamette. Taking 29th for bicycles offers access to Amazon bikepaths, but the long block between 29th PI and 29th St offers limited safe access for bicycles to Woodfield Station and surroundings.

I'm concerned with the narrowness of the streets for buses & trucks in option 3.

If the goal is really to provide safe access for all modes on Willamette Street and not three blocks away, and not along another, parallel, street, but ON Willamette Street you'll see that Alternative #3 is the clear choice.

Alternative 5 creates a pedestrian friendly district and I think is the best commercial option, with the best commercial gain, as well a creating a safer streetscape. Alternative 1 doesn't solve many problems. Willamette is not the best place for bike lanes and is not a good commuter rout for bikes or cars. I think commuting trafic should be loaded onto Amazon Parkway.

As a bicyclist that frequents the south willamette area businesses regularly, it is amazing to me that there aren't more bicycling design options. Most of the reason why I don't go to the area more often is because of the ridiculously horrible bike access. It's very unsafe and there are no good direct routes at all. If eugene wants to be a bicyclist friendly place, we ned to have more options for both bikes and pedestrians.

Bike lanes are a must. If not, you'll be revisiting this in a couple years.

Bike lanes are the best way to encourage this mode of transportation and the best way to keep motorists, cyclists, and pedestrians safe.

Bike lanes on Willamette street are a necessity. It is dangerous to ride your bike there now. The options that do not include bike lanes are unacceptable.

Bikes need to be in bike lanes not on the sidewalk. The design should provide access for bikes as they are vehicles too. I do not bike to Willamette often because of the dangerous route I am forced to take-on the sidewalk. The sidewalks are too narrow and should be reserved to pedestrians or people in wheelchairs or walkers.

Emphasizing parallel/nearby bicycle access makes more sense than trying to accommodate bikes on Willamette. There are many streets in Eugene which one wouldn't sanely ride a bike on but there is nearby bike access. This works just fine. I am very concerned about slowing emergency vehicles if the street is reduced from 4 lanes of car traffic.

For any alternative that does not include bicycle lanes, a detailed plan for alternative bicycle routes, including access to businesses, is necessary for earning full evaluation of the alternative.

I am a bike commuter to work but will not take Willamette due to safety issues no matter what decision is made. I take the Amazon bike path for safety and because it's much nicer. I am an avid cyclist. There are ancillary streets I can take. Don't ruin the livelihood of the

Willamette Street businesses. Leave the street the way it is.

I believe that the option to increase vehicle throughput and bicycle safety is to remove bicycle lanes from main thoroughfares and turn the next adjacent street into a 'bicycle street', as Alder Street is. Having lived in big and small cities, born and raised in EUG, I have never seen a peaceful solution with bikes and cars on the same road. Alder street is a wonderful example. Bicyclists do not ride on Hilyard/Patterson because they have a safe and pleasant place to be that is not inconvenient to use. Ceeding a small ancillary street to bicycles makes no impact on vehicle traffic.

I commuted by bike along Willamette to the Village School for three years. I was forced to ride on the sidewalk for portions of the trip because there is no room and no visibility on the street. Obviously bikes riding on the sidewalk raises safety concerns. For those of us who are not uphill bike riders, Willamette St. is the only real north-south corridor to So. Eugene. Please add bike lanes to Willamette St.

I don't think biking on Willamette will ever feel safe - even with bike lanes. I don't ever use the lanes on Coburg Rd., for example. So - extra wide sidewalks work best to accommodate a variety of activities; ie. #5. Onward

I live and work in this general area and I can not express to the powers that be how much we need bike lanes on Willamette. We also need to make this thriving area more pedestrian friendly. Although I personally would go farther with more/bigger bike lanes and buffer zones, Alt. #3 at least offers bike lanes. It would be foolish at this current juncture in our history to redesign a major thoroughfare without bike lanes. Honestly, I can't believe 2 of the designs don't include them. Seven generations people!

I love the shops and restaurants on this section of Willamette, however, I don't feel safe riding my bicycle in this stretch of Willamette so I rarely shop/eat in this area, which is unfortunate since I only work a few blocks away. Increasing usability for vehicles without providing bike lanes would not increase my usage of the area. Please provide designated bike lanes.

I must walk due to vision problems. My biggest problem is bicycles that travel at high speed and that do not slow of alert when approaching. Bike lanes will not help unless the riders actually USE them.

I understand that it requires an attitudinal shift for people to consider the importance of bikes and pedestrians, but I believe it is very important that we do so. Eugene has an opportunity to set a standard for livability and bikeability and projects such as the Willamette redesign can help set that trend.and define what we hold important. I strongly urge you to choose a redesign option that includes the needs of pedestrians and cyclists as you move forward, Regards, Robert Ault 541-337-9765

I would be curious about a divider between the bike lane and car lane on alt. 3 - a 7' sidewalk seems acceptable to me at this location. Bike access is really necessary here, since biking on side streets is very different (far more difficult) than car shift to those other streets (an engine makes a big difference. I frequently avoid the area if I'm on bike but it has many businesses I'd visit if it was more friendly. An option that doesn't include bike lanes is very shortsighted.

I would be willing to use parallel streets for bike travel (and already do). However, I frequent businesses on Willamette on my bike that can only be accessed by actually biking on Willamette street itself. I would be willing to support wider sidewalks rather than bike lanes if bikes could safely use the sidewalks as well - if the "design features" did not block bikes travelling on the sidewalks.

I would frequent the businesses on Willamette more often if it were easier to travel by bike on the street and find decent bike parking. Fewer driveways (ex: glenwood, play it again sports, mini pet mart consolidation) would also benefit bike, pedestrian and auto traffic.

I would like to see European-style bike lanes installed, that are curbed and raised a few inches from the car street level, to protect bicyclers from car 'slosh'; and also structurally defined from the pedestrian sidewalk to discourage casual over-lapses. Thanks for making an effort to humanize Willamette Street from its current harshness toward everything but conduiting cars and bigger vehicles.

If cutting Willamette to 2 lanes plus a turn gets you bike lanes and only a 60 second increase in delay, I'd say that's well worth it. More cyclists means fewer cars on the road.

If Eugene prides itself on being a "green" city, we need to do everything we can to encourage biking!

If I could safely ride in a bike lane, I would bike instead of drive to businesses that I frequent along Willamette. If there are fewer cars because folks could safely bike, that would help others who must drive or take buses. If there is a dedicated center turn lane, I would also be more inclined to frequent businesses because I could actually turn into their properties. Now it is not easy to do so.

Ignoring one whole group of users is unacceptable now and totally misguided for future needs. Lets do it right the first time. While cities across the country are prioritizing bike accessibility, Eugene fails to do so, even on bike corridors like Blair. Pathetic!

Not clear if bikes could use the wider sidewalks in Alt#4. If there are no bike lanes on Willamette St, then I would probably prefer to ride on the wider sidewalks than go to a parallel street.

PLEASE ADD BIKE LANES! I'm happy to pay taxes towards this!

Ridiculous to consider any plan that does not include bike lanes! Do you think bicyclists will just disappear? I believe that if the city consciously chooses an option without bike lanes and a cyclist dies in this corridor, you should be held liable.

Very concerned about adding bicyclists to the mix with all of the driveways & drivers crossing lanes of traffic when there is an "opening" especially if only 1 lane in each direction fewer gaps for people to get into the "flow."- people tend to focus on cars more than they do cyclists (I do cycle). I would like to see cyclists on parallel streets for safety concerns. By rights cyclists should be able to use Willamette. We are dealing with an EXISTING LIMITED amount of space - thinking of SAFETY now. Thanks for being so inclusive in this process.

We have lived and traveled in the Willamette corridor for many years. We feel strongly that Willamette street is too narrow and dangerous for addition of bicycle lanes. Not to mention the danger of cars and bikes sharing the multiple driveways and turns into and out of businesses. When we bicycle we use our city's fantastic bicycle paths and alternative routes with less traffic for our safety and sanity. The amount of people that would convert and use bicycles to shop at Willamette street businesses would be a very small number. We vote for alternative #1 due to above and concern regarding poor traffic flow with alternative #5.

With 4 traffic lanes and narrow sidewalks, walking and biking through this area feels unsafe. As my family travels primarily by bicycle, we sometimes choose to avoid the area. When I am shopping in the area, I often need to travel 4 blocks or less down Willamette. Using parallel streets would double the length of my trip, so I make the choice to take a lane on what feels like a highway (another option is the sidewalks, but cars turn in and pull out without seeing cyclists on so many driveways that it's safer to be seen by drivers and impact traffic out in a lane). A better design than any of these would include protected bike lanes (on the "sidewalk" side of the curb, and separate from a walking path), or at least a slightly elevated grade in the same position as in Design 3.

wide sidewalks could accommodate bicyclists better with signage akin to bike paths to encourage pedestrian traffic to stay right and bicyclists to signal when passing.

As a business owner on the east side of Willamette, I am very concerned about the city's idea of putting up a stop light and immediately affecting the buildings and businesses on the east side. Car and travel access is very important up and down Willamette Street. the 29th area is already a little hidden from the rest of Eugene, and needs the access so that shoppers can get to and fro without a hassel. I am curious to see what the percentage ratio is between drivers and bike riders along this area? I have seen no statistics on this. This HAS to play some kind of part? What are the daily traffic counts here? I bet pretty high? We need to celebrate the businesses along this stretch, not make it hard for them!

I don't think businesses will buy into any plan that closes their driveways on Willamette (that's really not fair to them as there will already be hardships during construction, folks using alternative routes, etc.). I'm guessing that with the turn lane installed we won't need a stoplight at Willamette and the WoodField Station.

I feel businesses on Willamette should have the most say in this, as most are independent businesses and it wouldn't be good to have their business effected in a negative way from this. I would like to see more crosswalks--maybe at 24th Place and between 27th and 30th.

More bicycle and pedestrian access on Willamette St. will improve the neighborhood and I believe businesses will ultimately benefit from this increased access.

Please check out the studies that show that increased bicycle access leads or a rise in business. We need to move away from a car oriented corridor; Businesses should know that reducing cars and providing more pedestrian and bicycle infrastructure here is the best thing for them.

I co-teach an energy science & policy course at UO, and we see further evidence every year that gasoline prices are likely to continue to increase at or above the rate of the past ten years (in which they've approximately tripled). Please help Eugene adapt to this future by giving commuters, shoppers, and businesses a great set of alternatives to car-only transportation! Thanks for all your hard work!

I think doing construction without adding a bike lane is a waste of both time and money. I also think that taking five years to fix a road is way too long. (I am getting this time frame from question three which give statistics for 2018). I don't know too much about construction, but I do know that it should not take that long to fix a road.

• I like #5, if there would be enough breaks in the traffic, that a car could actually turn. I missed the mtg but still am concerned that I'd be in the center lane and not able to turn with all the traffic funneled into 1 lane. • As a bike rider, I am fine carefully sharing the sidewalk if it is wider. That could help pedestrians and bike riders. • I love the idea of more trees. • I like the idea of cafes but having gone to Holy Cow and sat outside many a time, I think Willamette St is way too noisy to enjoy a cafe environment. I may be more sensitive to noise than others * The artists renditions are lovely but don't realistically represent how that street really is with MANY more cars than shown during most of the day and night. * Making a left from Woodfield station onto Willamette is difficult much of the time. I leave via 29th and that helps some.

10' lanes in Alternative 3 are too narrow. Most drivers can not deal with them, and will tend to crowd the bike lanes anyway. In addition, in Alternative 3, when busy, turning movements across the bike lanes is dangerous. Therefore Alternative 5 is better for bikes. Again, there are turning movements, but a lot more time for bikes and cars to react. Alternative 1 is a terrible idea all together.

Are we trying to make Willamette along this stretch into a lingering street (i.e. wide sidewalks) or a better commuter street, or both? I'd hate to roll the dice on nice wide sidewalks that no one uses. If bike lanes were along Willamette, I still wouldn't use them like I don't bike Willamette now, prefering a quieter/less busy street instead, esp. with the narrow travel lanes. Are we considering this street too much in isolation from neighboring parallel street opportunities? Probably not, but poor little Willamette can only do so much. Also, all I can see with a light at Woodfield AND 29th is a major nightmare. And some decent street trees and plantings outside businesses would go a long way towards making things look better. I hope this is in whatever plan.

As a frequent pedestrian, it is dangerous to be sharing narrow sidewalks with fast-moving bicyclists. And it is dangerous for the bicyclists to be in the same lanes as faster-moving cars. They really need bike lanes for everyone's safety. As we plan for the future we should be prioritizing walking, biking and public transportation FAR over individual cars, not only for the sake of the environment but because eventually it will become too expensive to fuel personal cars, and people will end up needing those alternatives.

I don't see how alternative #1 is the best option for vehicles. People drive too fast and everyone knows it's not currently working well. Because of this, I don't go to businesses on S. Willamette very often. I think alternative #3 will not only be better for bicyclists, but also cars and trucks. It will slow traffic down to the speed limit and improve safety for all users. If this is done, I'll be more likely to visit businesses on S. Willamette by both car and bicycle.

I live on Portland St near 26th (the street parallel to and just west of Willamette). We get a ton of bikers, walkers, and runners going by our house and I think a lot of these folks are diverted off Willamette because it's so crappy to bike or walk down. I wish that any south Willamette businesses who oppose bike lanes because they think it will hurt business could see the amount of pedestrian traffic they are missing out on when people take side streets instead. Those who are walking or biking are more likely to notice a new restaurant or see that a store has a sale and actually stop at a business rather than people who are trying to get to their destination quickly in a car. The center turn lane with bikes lanes makes a ton of sense for Willamette. Seems like it works fine for 18th, and it's a lot easier and more pleasant to ride, drive, and get in and out of business on 18th compared to Willamette (which I try to avoid as much as possible using any form of transportation, even though I live right off it). Make it a street that works well for pedestrians, bikes and cars and it will be much better for the businesses located there.

I think it's important to make that section of Willamette more bike and pedestrian friendly, because of the types of business and proximity to schools and neighborhoods. Since it narrows to two lanes anyway, it isn't a great arterial. It would be better to have most through traffic funnel into Amazon.

I travel this area. IF you go three lanes the middle lane must go into city in the AM work travel times and out in the PM times. Any study showing only a 60 second delay at these time cannot be accurate and is a typical misuse of any study.

I was riding the bus along Jefferson Street between 18th and 13th this morning. I noticed that if a truck and the bus came to the same spot in the road and that spot had either a car on each side of the street or a wider vehicle parked on one side of the street, at least one lane of traffic had to stop and make way for the other lane of traffic. At times both lanes stopped to decide just who would go first... So, while I support option #3, I also support the outside lane being a full 11 feet wide to carry the bus & truck traffic load and I support the bike lane being being 1 foot wider also. That would but us back to the 9 foot sidewalks, make the auto/truck/bus lanes safer, and the bike lanes safer. Then, asking bikes & skate boards to have limited access (like it is downtown = walking bikes and carrying skate boards) to the sidewalk is not unreasonable. Everyone gives a little something and everyone gets a little something. WIN/WIN/WIN.

I would have the sidewalk and a bicycle path separate from the street on just one side of the street, make Willamette one way.

I've lived here 38 years and use Willamette all the time. I've never seen an accident, or near accident. I drive Willamette at all different time of the day so I think I can gauge the situation well. I'm a bikerider also. Willamette is not for bike and I know it; so I don't use it for that. If I must use it when on the bike, I use only that part that takes me to where I'm going. Some streets are just not meant for bicycles. As far as peds. are concerned, walking should be an issued if you stop, look and then walk. Willamette St. is a major thoroughfare and the present street configuation shouldn't be dimenished in any way. If we want to keep our taxes lower, we should encourage the use of this street and its businesses not decrease its usage.

I've lived off Willamette at east 39th place for over 31 years, with over 17 of those years as a commuter to downtown Eugene. If we could start from scratch, and if Willamette wasn't such a busy and critical transportation route, then I would be fully supportive of a 3-lane option with wide sidewalks and bike paths. However, with the reality we have it is just not practical or safe to give up the 4 lanes to accommodate those changes. I don't believe the traffic people's estimate of only a 30-second delay if we go to 3 lanes. I agree with many of the points raised in R-G letters to the editor from those who are against the 3-lane options. I also think you would see people using the center turn lane as an extra lane to get around traffic that is backed up. One idea I like that I've seen elsewhere is to have signs at the stoplights saying No Left Turns 4-6 pm (or something along those lines). I believe bicycles should be prohibited from being on the streets on that stretch of Willamette because it is very unsafe both for them and for the drivers, and they clog up the traffic terribly. They should be directed to alternate bike routes as close to Willamette as possible. We have wonderful businesses in that corridor, and the natural downside of that is a lot of in and out traffic. I often do all my errands going no further south than Capella. The businesses in that area have only gotten better and better over the years, and to me it captures the very nature of Eugene, with a wide mix of people and places, conventional and alternative. I love south Eugene very much and don't want to see its vibrancy compromised to accommodate a very small but very vocal group of bike riders. I am sympathetic to their issues but they are a small user group. In all my years of driving Willamette I have also witnessed some very bad bike behavior which has endangered the bikers and the drivers. The one thing that has been extremely important to me for many years but that I haven't officially advocated for is to please, please, please plant large, beautiful trees along that stretch of Willamette (and from Willamette east on 29th to the Parkway also). It has been one of the ugliest roads compared to many others in Eugene and it deserves better! I know just that improvement alone would have a tremendous aesthetic impact and give the area more of a relaxed, neighborhood feel than a concrete/asphalt "hurry through" feel. Thanks for listening!

in all presentations, motor vehicles are the priority and everything else is an afterthought. the plans are disappointing, lack originality and miss the point of creating a go to place for people. every question is based on some aspect of using a car in the area. question 1 pays slight lip service to other concerns but i can't even answer the rest because not being car focused and wishing i could actually drive less, there's not much choice.

Let Woodfield Station traffic enter ONLY from the Willamette entrance (East edge of property) and ALL Traffic exit only on the South boundary onto 29th with timed traffic lights at 29 & Will. North-South and East bound drivers use the exit closest to the old bank and West bound drivers exit nearest Rite Aid. OR...buy the newly abandoned Wendy's, tear it down, grade the property and create an entrance/exit to Woodfield Station there with a light at 28th and Will.

Many residents such as myself travel Willamette Street because they stop to shop. Currently it is already a nightmare with 4 lanes of traffic to try to turn left due to the traffic at peak times, add buses and it's a mess. We really need lights that include opportunities to turn left to not tie up traffic. Trying to get in and out of Oregon Community CU is next to impossible at times already. It concerns me that if the lanes go to 3, there will be traffic both going South and North trying to turn left in that one middle lane increasing risks of accidents. I highly recommend that should the City still think this 3 lane idea is a good one to pre-test it first to get the reality before investing. Crest Drive is a nightmare to drive that someone thought a good idea. Please do not do the same to Willamette Street.

My preference was eliminated from consideration by city staff. The best alternative in my opinion is two lanes southbound, one center left turn lane, and one lane northbound. There is a wonderful north-south bike path in Amazon and Tugman parks that I use when I bike north from my home in south Eugene so bike lanes on Willamette would be redundant. If alternative 3 or 5 are implemented, the businesses on Willamette from 6th to 29th will lose a lot of business because homeowners in south Eugene will no longer travel Willamette street to return home from work and stop to make purchases along the way. Much of that business will go to the Hilyard corridor and Edgewood shopping center. If you further limit motor vehicle accessibility to and from south Eugene you make it a less desirable neighborhood just as reducing bus routes has done. Don't allow Wildish to bid on this project because they ruined the street drainage on Hilyard street and south Willamette the last time they touched it.

Not-to-distant future autos will be smaller, electric, driver-less, and majorly public-owned and shared. Alt 1 and 5 would suffice, Alt 3 will be adequately satisfied with already-planned adjoining-routes.

On the sidewalk on wilammette, right behind the civic stadium is a very pretty two way staircase- on college hill. In Europe such two way staircases are made very charming by putting canopy on it with creepers/climbers on it...very very charming- this one has so much potential to be done like that. I have seen that in India as well, in Hill stations. That should really be considered, right now its been destroyed by grafittis

Please consider the demographics of the people living in South Eugene. I have family members also living in South Eugene and many friends and neighbors who are either retired or approaching retirement. I have no interest biking down the hills (or back up) lugging groceries or just for a leisure trip meeting friends for lunch, etc. I appreciate that bicycle riders want better access, but the reality is, how many are going to use those lanes in downpours, or other inclement weather? They already have other choices. I'm very concerned about fire and ambulance runs blocking traffic or not being able to get through fast enough. Is a 30 - 60 second delay worth it to someone who has an emergency?

Please remember that you only hear from a certain segment of the public. In fact, in vocal minorities often drive decision making in this town.

Question #3 implies I'm in a car. Please stop making that assumption. The commute time via bicycle would be incredibly improved with on-street bike lanes.

Question 3 seems heavily biased and does not take into account Eugene's large number of dedicated commuter cyclists, whose commute times could be cut drastically by alternative 3. Prioritizing through car traffic is a good way to make any street hostile to both cyclists and pedestrians (important to keep in mind that both cyclists and drivers instantly become pedestrians as soon as they leave their vehicles, and that all business patronage occurs when people are operating in this mode).

Really, anything would be an improvement. I am scared to walk with small children on Willamette because the cars are so close to the sidewalk (which is narrow in parts). It would be great to have biking lanes!

Thank you, Chris, for such excellent transparency and communication with the citizenry of S. Eugene!

The idea of alternate streets for bikes and peds does not mention that the sidewalks are not continuous on either Portland or Oak. Also that traffic going west will often go 24th to Portland to 27th which increases the traffic making walking in the street when necessary unsafe. Thanks for asking for input.

This is a key chance to improve an area blighted by automobile dominance.

This is a major arterial route to South Eugene. Let's route bike lanes to other areas, and preserve this as is. Let's reinforce the speed limit to reduce accidents, and provide better pedestrian crossings.

This project needs to prepare for the next 25+ years. It's important to keep a vision of a more inviting streetscape, allowing cars to pass through but balancing needs of the other potential users as well. It will also be helpful to provide for users in cars to approach the corridor from each side, park, and walk to multiple destinations. The existing corridor is too narrow to accommodate everyone optimally. It seems bicycles are the most reasonable to displace to parallel streets IF their needs are met with good connecting points.

To me, leaving the current configuration is the only sensible option. Put bike lanes on other parallel streets, let Willamette be for thru car traffic. Thanks. (Do feel it's already a done deal & the city will reduce the traffic lanes) Some questions are "leading" - designed for only 1 desired outcome.

We need to make it safer for people to walk and bike amongst businesses, as well as maintain a smooth flow. I would like to see the utilities put underground to make the whole environment more friendly to restaurants and people. If there are slight delays, more people will use Amazon, which is a good thing.

Your illustrations of the alternatives were pitiful, all of them. You know what the streetscape is; that is what you should have been presenting, not the fanciful pile of rubbish you showed instead. Cars parked parallel to Willamette, right next to the street, I don't think so. Accurate representation of the scale of the buildings, I don't think so. Where is the representation of conflict between drivers from opposite directions both wanting to make left turns? You've shown this area as perfectly flat. The west side of the street is as far from level as you could possibly get. Fantasyland! Those of us who live in this area are going to be greatly affected by these decisions. Your representations of all three of the alternatives all show how disconnected you are from what is actually happening in this neighborhood, and what it actually looks like, and how it actually functions.

Any design that does not provide facility for all transit modes: motor vehicle, bicycle, pedestrian, skateboards, public transit and wheelchairs and electic wheelchairs would be a total failure on the part of the Willamette Street Project.

Anything less than a street that serves all of the citizens who use it is a failure of process. Thank you to everyone for your hard work and concerted efforts devoted to improving my neighborhood. While I would prefer (in an ideal world) to see bike lanes directly on Willamette St. (as I am a regular bike commuter and use my bike for shopping in this area) I would prefer to share a wide sidewalk (like I do in Amazon Park) so that the buses and trucks would have the 11' lanes they need.

The 30-seconds longer trip figure is highly suspect. The westbound lane reconfiguration a few years ago on 29th between Oak and Willamette added much more than 30 seconds to the transit time for the 29th and Willamette intersection during busy traffic periods. For the proposed #3 and #5, just getting stuck behind a bus stopped for passengers anywhere between 24th and 29th would certainly add more than 30 seconds. I am very much in favor of increased bike lanes, wider pedestrian sidewalks and "design elements" but can't help but think that #3 would slow down vehicle traffic significantly and #5 would do the same but with almost no public benefit. The obvious smart thing to do is a trial of the #3 by just re-striping the existing street without making any other modifications to the route.

The more Eugene can accommodate motor vehicle travel with the needs of pedestrian, bus, and bicycle travel the better. All four modes of transportation are important for the city's quality of life. Currently the city caters to motor vehicle travel more than the other modes of transportation. I am fine with the city diminishing motor vehicle accessibility if it significantly increases the accessibility of the other modes of transportation. The Willamette St. renovation is a great opportunity to significantly increase residents' ability to bus, walk, and bike through the city while only minimally reducing the ease of motor vehicle travel.

This corridor should be more highly considered a destination rather than a through-fare for commuters, and ALL modes should be balanced to maximize the safety and comfort of ALL corridor users. Demote the importance of this corridor meeting typical "Performance" standards for auto flow.

A redesign of southern Willamette St. is not useful or practical in isolation. Without consideration of the one-way portion south of 18th and the connections of Willamette St. with Amazon Parkway, a change in traffic pattern is of little use.

A 30 second delay seems a small price to pay for increased safety. I never bike on Willamette Street because it is just too dangerous, but I would do so frequently if there were bike lanes.

As a resident south of 32nd and a parent I want to stress how important it is for me to have a SAFE access to businesses on Willamette St. for both my son and me. We would bike far more often but because of the current alignment I drive adding to the congestion. Think long term: more people will be biking as the cost of operating a motor vehicle gets more expensive. Build the bike lanes.

I feel it is very unsafe to allow bike riders on Willamette St. if the sidewalks were wider, pedestrians and cyclists could share.

It is not safe for bikes and peds the way it is and it is ugly and has no character, we can do better.

Make it safe and fun for peds and bikes, and discourage people from driving. People will flock there. This worked like a charm in the Pearl District of Portland. Driving in that neighborhood is a nightmare but the businesses are all packed with pedestrian and bicycling customers.

Make this stretch friendly for walkers and cyclists, please!!

Safety first: designated bike and turn lanes. Grew up and currently live on 26th & Willamette since the 80s. Lack of safe bike lanes has been a deterant for me to choose my bike over my car. Bet I'm not the only one.

Safety should be the highest priority issue, please stress the safety aspects of alt 3 & 5.

The only way to make bike riding safe is to have a physical separation between the bike lane and car traffic. That is really the only option that is acceptable to me. It can just be as little as a six inch wide curb.

This section of Willamette is one of the scariest places in the whole city to be on a bike, and I would love to see that change.

Vehicle traffic should be shifted to Amazon Parkway, that was the whole idea of putting in Amazon Parkway and access to Amazon Parkway should be improved. Without bike lanes, Willamette will continue to be unsafe for everyone.

Willamette is miserable to travel by foot, and dangerous by bike...I shop elsewhere on a regular basis to avoid biking with my children on Willamette. Our family loves to walk over to shop, and we'd do this more often if it wasn't such a yucky walk once we got there!

Willamette Street is busy enough that adding bike lanes will DECREASE safety

For "Sidewide amenities" you listed "Cafe Seating"--but there are no restaurants or other establishments abutting the sidewide so that appears to be a completely misleading statement. In addition, if the adopted concept is for bicycles and pedestrians to share the sidewalk, then (a) tables, sandwich boards, and etc. should prohibited because they would interfere, (b) curb cuts should be level (not sloping) to improve safety and (c) something needs to be done to remind vehicles to stop and look before crossing the sidewalk (it is an uncommon practice now).

Alternatives 3 & 5 should only be considered any further if they were subjected to at least a full year of testing. Refusing to test is an admission of their potential for creating grid-lock during times of high traffic volumes: in the morning, around noon, 'school out' and end of conventional work days. Where would the cost of developing 'off-site' improvements - such as pedestrian/bicycle crossings - would come from. Were they identified in the bond prospectus? Thank you for the opportunity to participate in this survey.

As a long time former commuter and resident of South Eugene (I live at 43rd and Hilyard), I'm appalled by the considerations to potentially reduce Willamette to fewer lanes in order to provide greater access to bikes. As I've mentioned elsewhere--there are sufficient bike paths, protected routes, bike lanes to enable bikers to travel where they need to (and when I was a commuter biker, I utilized these routes of travel just fine). There are not sufficient alternate paths for cars, and the suggested increase in congestion will drive many of us to begin our morning commutes driving through *neighborhoods* seeking a path around the congestion points. Bikers don't get to "have" every functional travel artery reworked simply to cater to that specific lifestyle choice. I don't get to drive my car on the river bike path, the Amazon bike path, nor do I get to short cut to Valley River or Autzen using any of the bike bridges over Willamette. So it should go with major arteries that provide ease of travel and maximum business utility for cars. Willamette is functional as is, and anything that reduces ease of travel for cars seems profoundly short sited as well as wasteful. Additionally, no where in any of these plans do I see discussion of how to include disaster preparedness in these development plans. In event of a catastrophic dam breach, Willamette Street is one of the few travel arteries leading to higher ground. Attempting to reduce car efficiency flies in the face of emergency preparedness--if the "big one" earthquake hit and the dams failed would you be riding your bike to the Butte or would you be trying to drive a car or a busload of citizens to higher ground?

Especially if traffic is to be shifted to Jefferson, it would be nice to have a traffic signal where Jefferson runs into 28th. It can be very hard to turn left (east) from Jefferson. Ideally, it would be a light that is only triggered when a car is there.

I had an accident turning left out of WoodField station, so a stop light would be VERY welcome! I avoid that intersection as result of the accident. I like the idea of wide sidewalks allowing bikers to be removed from traffic lanes. I know that would slow commuter bikers and increase risks to peds, but would certainly encourage casual bikers to use Willamette, and set the basis for an enriched environment for cafe's, etc.

I live on 28th and High St. Many people do and will continue cut through my neighborhood at high speed. I would like this issue to be addressed if lanes are cut from Willamette.

I live on Portland. Currently no stopsigns on Portland between 24th and 27th. Local traffic likely to shift to Portland. Please plan for more Stopsigns on Portland. Your Alt #5 illustrations show the utility poles on the curb edge, when they would remain in place with a curb extended out past them. Please revise drawings. Also put bicyclists in perspective of #5, as they will use the wider sidewalk too presumedly. Please include striped crosswalk at 24th place across Willammette. Slower traffic/congestion could very well mean more opportunities for businesses to catch driver's eyes = more customers! The congestion = loss of business argument just does not hold water...

Most traffic is automobile. Any plan which gives priority to foot, bike traffic is inconsistent with most travelers' priorities.

Motor vehicles comprise the vast majority of users of this corridor. It makes no sense to hinder thousands of drivers to accommodate dozens of alternative users.

On what do you base that Alternatives 3 & 5 would add 30 seconds to the travel time? If a new traffic light is placed at Woodfield Station, many cars coming from the South would wish to turn left there, blocking the "through lane" in Alternative 1. If no light is placed there, it is only a matter of time before a pedestrian will be killed there trying to cross the street.

Please change the three way stop at Olive and 23rd to a 4 way stop. This will slow down traffic in the neighborhood where there are no sidewalks and hopefully keep traffic from moving into the narrow street. Traffic is already going way to fast on Olive.

Question #3 is not well-written. I checked additional delay of "average of 60 seconds" per car trip, but this is not really my limit. AS LONG AS THE TRAFFIC FLOWED SMOOTHLY, I would be fine with a much longer delay (3 minutes?)

Regressive development is not the answer. Traffic has to move for commercial and residential by automobile not bikes or busses. Maybe a new north double lane street, just below Willamette St would accommodate all aspects of transportation. Thus, Willamette traffic could flow one way south and the new Street would accommodate proper traffic flow north. Bicycles need to go down the Amazon pathway not on highly needed automobile travel routes.

Repaving Willamette with 4 lanes of automobile traffic is not an acceptable alternative. The traffic on Willamette St in this corridor is already congested and difficult to travel. There are also a lot of really wonderful, oft-frequented businesses there! I cannot IMAGINE having *fewer* lanes for cars. And I think diverting traffic away from those important storefronts would be terrible. I do like to ride my bike, and I never use Willamette St. I am perfectly content to use the lovely bike paths that go through Amazon Park.

This area is a major bottle neck for all N-S traffic. Diverting traffic is not fessible or acceptable. You need to accept that diverting traffic is not acceptable. The best plan for bikes is to let them use the existing lanes and provide alternate routes.

Turning option at Woodfield Station would better consist of signage encouraging right turn only during peak traffic times, and signs directing pedestrians to walk 1/2 block to crosswalks.

Why isn't changing to two-way, the one way block south of 18th part of this master traffic plan? What happened to bus pull-outs. Anything less than 4 lanes makes them manditory. Keep the nikes off the roadway. Put them on a safer, wider sidewalk. Streets between Willamette and Jefferson are too residential to handle through traffic.

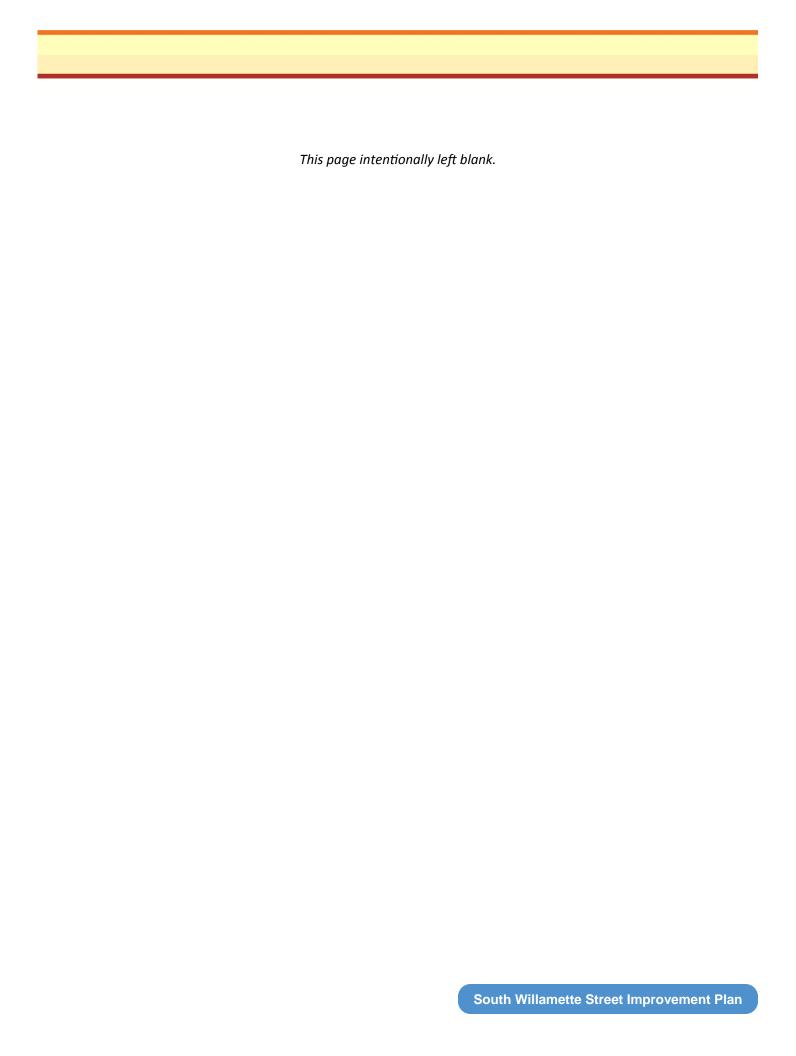
Willamette needs a center turn lane to function. Bike lanes make economic and environmental sense in S. Eugene. Willamette isn't as much a commuter street as a destination / shopping area and would be even more of a destination if it were more pedestrian and alt transport friendly - keeping more of our economy local, decreasing distances people travel to shop. Better sidewalks would mean people could park, stroll & shop rather than driving in and out of each business, which creates the awful bottle necks.

Willamette Street needs to remain 4 lanes. Bike lane is already provided on Amazon Parkway. Where are you planning for buses and bus stops and pickups? In the bike lane?? Then bikers will pull into the car lane to go around the buses and this is a recipe for disaster! The multitude will be in cars. Bikers are a few among many. I have used Willamette Street for 40 years. It needs to remain 4 lanes.

Wouldn't that additional signal (Question #2) be way too close to the existing signal? "Through-bikers" can continue to divert over to Amazon Parkway and to the off-street bike path in Amazon Park. The wider sidewalks and outside traffic lanes in Alternative #5 preserve more future options for transit-related improvements (BRT, trolley, etc.)

I lived on Lincoln st during the days of "dragging the gut". It was a nightmare to maneuver Willamette st. during the evenings. I do not want to go back! Put the utilities underground, carve out some more space for bicyclists on the wider sidewalks if they are so determined to use Willamette st. instead of parallel street bike lanes, and leave Willamette with 4 lanes and restrict turns to facilitate flow.

ALTERNATIVES DEVELOPMENT AND TIER 1 SCREENING





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MEMORANDUM #5

DATE: January 17, 2013

TO: Project Management Team

FROM: Scott Mansur, P.E., PTOE

Mat Dolata, P.E., PTP Peter Coffey, P.E.

SUBJECT: South Willamette Street Improvement Plan

Alternatives Development and Tier 1 Screening

P10086-012

This memorandum summarizes the development of draft alternatives for the South Willamette Street Improvement Plan and includes a preliminary evaluation (Tier 1 Screening) of six proposed alternatives. The analysis is focused on conceptual cross-sections that illustrate alternative uses of the available right-of-way. The memorandum also presents a variety of design treatments for consideration, as the alternatives are further refined.

Overview

The South Willamette Street Improvement Plan will explore options for people to easily and safely walk, bike, take the bus, or drive in an eight-block study area from 24th Avenue to 32nd Avenue. The goal of the study is to help South Willamette Street become a vibrant urban corridor accessible by bicycle, foot, car, and bus. Six conceptual alternatives have been developed to illustrate potential configurations for the available right-of-way. Also identified are alternative design elements that may be incorporated into alternatives as they are further refined in subsequent tasks.

Alternatives will be evaluated using a two-tier process. Tier 1 screening is a qualitative assessment based on criteria and scoring methodology identified in Technical Memorandum #1 (South Willamette Street Improvement Plan – Evaluation Criteria). This screening evaluates community priorities and identifies broad level tradeoffs that exist within a constrained right-of-way. The Tier 1 screening process will be used to identify up to three alternatives to be considered and further evaluated in the Tier 2 screening. Tier 2 screening assessments will include more detailed description and rigorous analysis of the facility design. Traffic analysis and multimodal measures will be included in the Tier 2 screening. Traffic operations will be evaluated for the 2018 horizon year.



Corridor Segments

The study area can be separated into three segments of Willamette Street (Figure 1). The north corridor segment, from 24th Avenue to near 29th Avenue, has a 60 foot right-of-way consisting of four travel lanes and no dedicated bike lanes. The "transition zone" is a short segment near 29th Avenue where the right-of-way widens to 75 feet. This segment currently has five travel lanes to accommodate left-turn lanes at the 29th Avenue intersection, and no dedicated bike lanes. The south corridor segment begins south of 29th Avenue. The right-of-way returns to approximately 60 feet, with three travel lanes and bike lanes available in both directions. Figures 2a, 2b, and 2c illustrate the existing cross-sections for the three Willamette Street segments.



Figure 1 – Study Corridor Segments

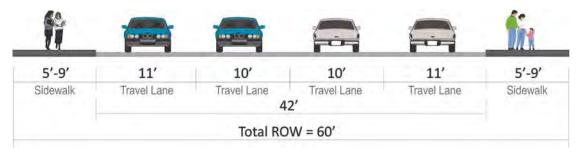


Figure 2a: 4-Lane Cross-section (north corridor segment)



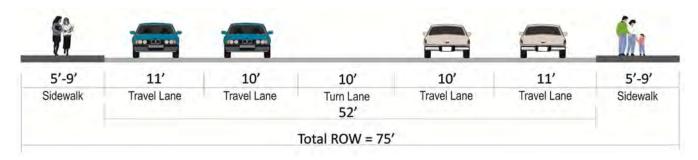


Figure 2b: 5-Lane Cross-section (transition zone)



Figure 2c: 3-Lane Cross-section (south corridor segment)

Alternative Cross-Sections

The six proposed alternatives are illustrated via conceptual cross-sections and overhead plan views (Figures 3 through 9). Although the three study corridor segments differ in existing design and surrounding land use characteristics, the alternative cross-section concepts attempt to create a foundation for a continuous and cohesive corridor while balancing needs and broad objectives.

The north segment of the corridor has the widest variety of possible configurations and the most influence on the overall corridor design due to length, proximity to commercial areas, and the availability of parallel travel routes. While all six alternatives may be considered for the north corridor segment, Alternatives 1, 2, and 5 would not apply to the south corridor segment because they do not included any dedicated bicycle facilities and no parallel facilities are available near Willamette Street, south of 30th Avenue.

The transition segment near 29th Avenue will be designed to be as consistent as possible with the north and south sections, while taking into consideration multi-modal needs across the corridor. Differences may exist in roadway configurations in the north segment and south segment for alternatives. The north corridor segment generally has a 42 foot curb-to-curb width, while the south corridor segment is 41 feet wide, resulting in some differences in how space may be allocated, particularly if curbs are not reconstructed as part of the alternative. Each corridor segment will be further detailed in the Tier 2 screening process for selected facility design alternatives.

The proposed alternatives are focused on developing a design for short term improvements; while also supporting a long-term corridor vision. To facilitate development of a design plan that can be adopted and

Alternatives Development and Tier 1 Screening January 17, 2013 Page 4 of 23



implemented in the short-term, an effort was made to minimize the costs related to right-of-way acquisition and curb reconstruction. Because any alternative that maintains the existing right-of-way cannot provide optimal facilities for all modes and design for all elements, priorities and tradeoffs must be carefully considered in selecting how space is allocated. Each of the conceptual cross-sections maintains existing right-of-way and only two of the six cross-sections would require curbs to be relocated.

Other design treatments identified later in this memorandum may include curb relocation and right-of-way expansion. These general design treatments are not associated with specific alternative cross-sections, but may be incorporated into their final design, as alternatives are refined. Right-of-way expansions may require property acquisition. Property acquisition and construction costs should be considered as part of the tradeoffs and priorities associated with each of the alternatives considered. The following section identifies each of the six proposed cross-section alternatives along with alternative-specific considerations for key elements of the facility design.

Alternative 1: 4-Lane

Alternative 1 maintains the existing (curb-to-curb) roadway configuration for the north corridor segment (Figure 3). Sidewalks would be expanded to their maximum width (approximately nine feet) within the existing right-of-way. Alternative 1 is not being considered for the south corridor segment because it does not include any dedicated bicycle facilities and no parallel facilities are available near Willamette Street, south of 30th Avenue.

Alternative 1 Considerations		
Motor Vehicle Mobility	Maintains existing four travel lanes	
	 Left-turning vehicles block travel lanes 	
Walkability	Consistent nine-foot sidewalks	
	 Sidewalks narrower than ten-foot standard width¹ 	
	 Sidewalk width is not sufficient to support active commercial streetscape² 	
Bicycle Facilities	No on-street bike lanes	
	 Improved bike access would occur via parallel route improvements and 	
	crossing enhancements (see Figure 10)	
	Bike sharrows possible on curbside lanes	
Transit Service	Maintains eleven-foot outside travel lane for buses	
Cost	Relatively low cost to maintain current cross-section	

¹ Minimum width defined for curbside sidewalks in pedestrian-oriented commercial areas.

Design Standards and Guidelines for Eugene Streets, Sidewalks, Bikeways and Accessways, City of Eugene, November 1999.

² A concept for the "Heart of the Walkable Business District" characterized by a "Safe, Attractive Pedestrian Experience for Business, Shopping and Entertainment" was identified in the *South Willamette Area Draft Concept Plan*, City of Eugene, October 2012.

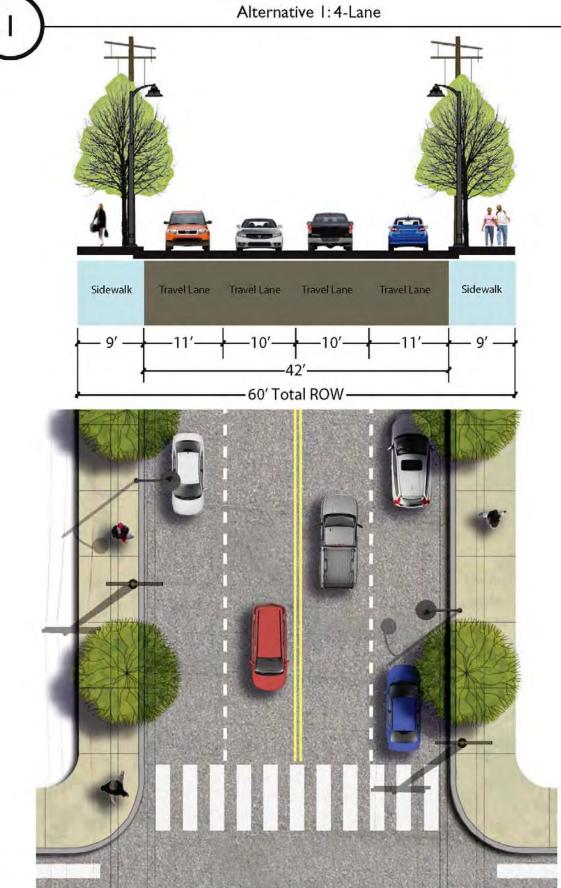


Figure 3 – Alternative 1 Concept



Alternative 2: 4-Lane with Center Left-turn Lane

Alternative 2 maintains four travel lanes in the north corridor segment, with one of the northbound lanes converted to a two-way center left-turn lane (Figure 4). The roadway would include two southbound through lanes, one northbound through lane, and a two-way center left-turn lane.

Sidewalks would be expanded to their maximum width (approximately nine feet) within the existing right-of-way. Alternative 2 is not being considered for the south corridor segment because it does not include any dedicated bicycle facilities and no parallel facilities are available near Willamette Street, south of 30th Avenue.

	Alternative 2 Considerations
Motor Vehicle Mobility	 Four total travel lanes maintained (2 Southbound, 1 Northbound, and 1 center turn lane) Provides center left-turn lane Southbound capacity increased Northbound capacity reduced Northbound buses stopped in a single through lane will have impact on northbound travel
Walkability	 Consistent nine-foot sidewalks Sidewalks narrower than ten-foot standard width³ Sidewalk width is not sufficient to support active commercial streetscape⁴
Bicycle Facilities	 No on-street bike lanes Improved bike access would occur via parallel route improvements and crossing enhancements (see Figure 10) Bike sharrows possible on curbside lanes
Transit Service	Maintains eleven-foot outside travel lane for buses
Business Accessibility	 Improves motor vehicle access during PM period, when commercial traffic is highest Center turn lane improves access for turning vehicles Does not significantly change accessibility for transit and bicycle modes
Cost	 Relatively low cost to convert lane direction north of 29th Avenue Intersections and traffic signals would need to be reconfigured north of 29th Avenue

³ Minimum width defined for curbside sidewalks in pedestrian-oriented commercial areas.

Design Standards and Guidelines for Eugene Streets, Sidewalks, Bikeways and Accessways, City of Eugene, November 1999.

⁴ A concept for the "Heart of the Walkable Business District" characterized by a "Safe, Attractive Pedestrian Experience for Business, Shopping and Entertainment" was identified in the South Willamette Area Draft Concept Plan, City of Eugene, October 2012.

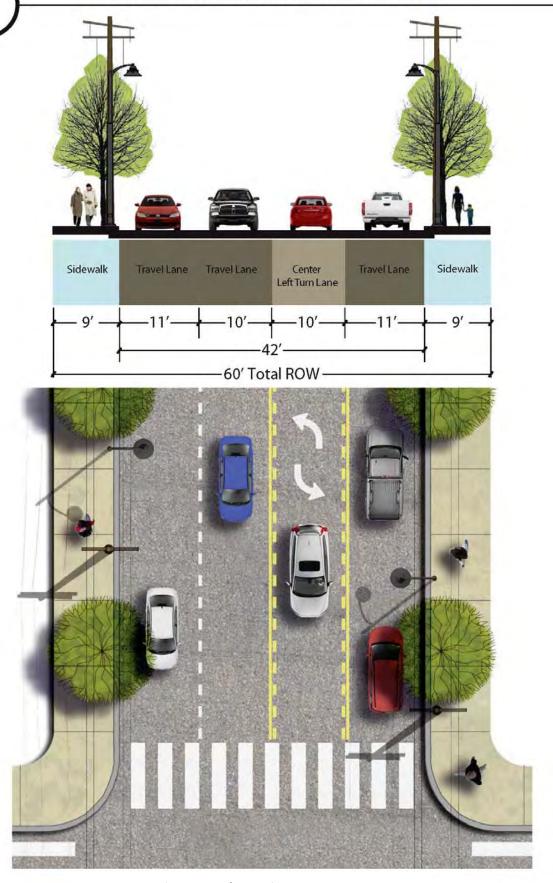


Figure 4 – Alternative 2 Concept

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Alternative 3: 3-Lane with Bike Lanes

Alternative 3 would provide one northbound through lane, one southbound through lane, a two-way center left-turn lane, and a bike lane in each direction (Figure 5). This configuration would convert the north corridor segment from four motor vehicle lanes to three, while adding two bike lanes. Three travel lanes would be maintained in the south segment of the corridor with one of the southbound lanes converted to a two-way center left-turn lane.

Sidewalks would be expanded to the maximum available width within the remaining right-of-way. Sidewalk and lane widths may vary across the corridor depending on the existing curb-to-curb width.

	Alternative 3 Considerations
Motor Vehicle Mobility	 Reduces number of travel lanes from four to three, north of 29th Avenue Capacity reduced and travel time increased for through-traveling vehicles Ten-foot travel lanes are narrow for trucks and less than the eleven-foot standard width⁵
Walkability	 Consistent nine-foot sidewalks Sidewalks narrower than ten-foot standard width⁶ Bike lanes provide separation from motor vehicle lanes Sidewalk width is not sufficient to support active commercial streetscape⁷
Bicycle Facilities	Includes six-foot bike lanes
Transit Service	Ten-foot travel lanes are narrow for busesPotential conflicts with bike lanes
Business Accessibility	Center turn lane improves access for turning vehiclesImproved bicycle access
Cost	 Moderate cost to provide center left-turn lane and bike lanes Intersections and traffic signals would need to be reconfigured

⁵ Minimum travel lane width on Minor Arterials is 11 feet

Design Standards and Guidelines for Eugene Streets, Sidewalks, Bikeways and Accessways, City of Eugene, November 1999

⁶ Minimum width defined for curbside sidewalks in pedestrian-oriented commercial areas.

Design Standards and Guidelines for Eugene Streets, Sidewalks, Bikeways and Accessways, City of Eugene, November 1999.

⁷ A concept for the "Heart of the Walkable Business District" characterized by a "Safe, Attractive Pedestrian Experience for Business, Shopping and Entertainment" was identified in the South Willamette Area Draft Concept Plan, City of Eugene, October 2012.

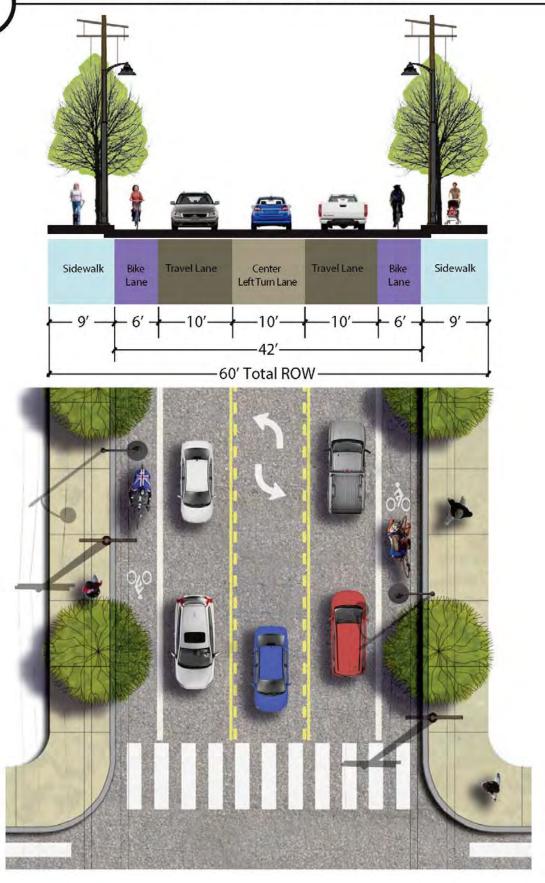


Figure 5 – Alternative 3 Concept



Alternative 4: 3-Lane with Buffered Bike Lanes

Alternative 4 would include one northbound through lane, one southbound through lane, a two-way center left-turn lane, and a buffered bike lane in each direction (Figure 6). The roadway would need to be reconstructed to expand curb-to-curb width to 47 feet. The alternative may apply to the north and/or south corridor segment.

Sidewalks would be expanded to the maximum available width within the remaining right-of-way. However, with the 47 foot curb-to-curb width, sidewalk width would be limited to approximately six and one-half feet on both sides of the street, unless additional right-of-way is acquired.

	Alternative 4 Considerations
Motor Vehicle Mobility	 Reduces number of travel lanes from four to three, north of 29th Avenue Capacity reduced and travel time increased for through-traveling vehicles Maintains eleven-foot outside travel lanes
Walkability	 Sidewalks only 6.5 foot in width Curbside sidewalks far narrower than ten-foot standard width⁸ Buffered Bike lanes provide separation from motor vehicle lanes Sidewalk width is not sufficient to support active commercial streetscape⁹
Bicycle Facilities	 Includes five-foot bike lanes with two-foot buffers Bike lanes painted green to distinguish from motor vehicle lanes
Transit Service	 Maintains eleven-foot travel lanes for buses Potential conflicts with bike lanes
Business Accessibility	Center turn lane improves access for turning vehiclesImproved bicycle access
Cost	 Higher cost for reconstruction to expand existing curb-to-curb width With reconstruction, utilities should be relocated for ADA compliance Intersections and traffic signals would need to be reconfigured
Other	 Center left-turn lane offers opportunities for design elements including raised median treatments (e.g., landscaping, pedestrian refuge, access management) Sidewalk and right-of-way width may be widened with redevelopment (i.e., as a condition of development approval) Narrow width limits sidewalk design treatments (e.g., landscaping, lighting)

⁸ Minimum width defined for curbside sidewalks in pedestrian-oriented commercial areas.

Design Standards and Guidelines for Eugene Streets, Sidewalks, Bikeways and Accessways, City of Eugene, November 1999.

⁹ A concept for the "Heart of the Walkable Business District" characterized by a "Safe, Attractive Pedestrian Experience for Business, Shopping and Entertainment" was identified in the South Willamette Area Draft Concept Plan, City of Eugene, October 2012.

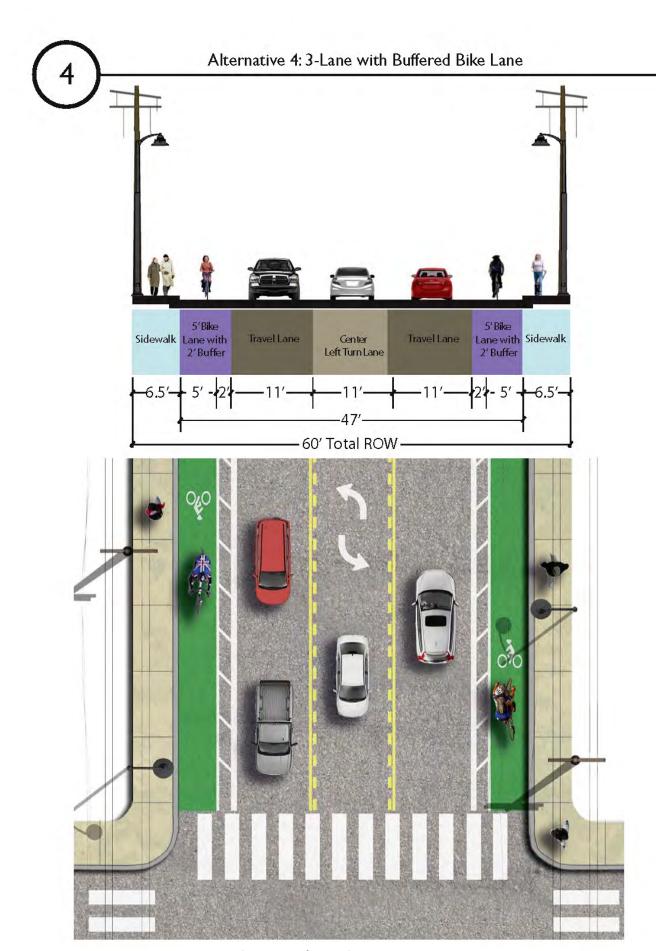


Figure 6 – Alternative 4 Concept

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Alternative 5: 3-Lane with Wide Sidewalks

Alternative 5 would convert the roadway from four motor vehicle lanes to three, north of 29th Avenue (Figure 7). The roadway would be reconstructed to expand sidewalks, resulting in a narrower curb-to-curb width (34 feet instead of the current 41 to 42 foot width.) No new bike lanes would be included on Willamette Street.

Sidewalks would be expanded to the maximum available width within the remaining right-of-way. With the 34-foot curb-to-curb width, sidewalks could be extended up to 13-feet. Alternative 5 is not being considered for the south corridor segment because it does not include any dedicated bicycle facilities and no parallel facilities are available near Willamette Street, south of 30th Avenue.

	Alternative 5 Considerations
Motor Vehicle Mobility	 Reduces number of travel lanes from four to three, north of 29th Avenue Capacity reduced and travel time increased for through-traveling vehicles Maintains eleven-foot outside travel lanes
Walkability	 Provides wide (13-foot) sidewalks to facilitate a transformative pedestrian environment including design treatments (e.g., storefront displays, café seating, landscaping)
Bicycle Facilities	 No on-street bike lanes Improved bike access would occur via parallel route improvements and crossing enhancements (see Figure 10) Bike sharrows possible on curbside lanes Potential to provide raised bike facility if additional right-of-way acquired for sidewalk widening and reconstruction
Transit Service	Maintains eleven-foot travel lanes for buses
Business Accessibility	 Center turn lane improves access for turning vehicles Wide sidewalks provide opportunities for design treatments to support commercial development, aesthetic treatments, and walkability
Cost	 Higher cost to reconstruct curbs to expand/reconstruct sidewalks Intersections and traffic signals would need to be reconfigured
Other	 Center left-turn lane offers opportunities for design elements including raised median treatments (e.g., landscaping, pedestrian refuge, access management) Wide sidewalks support "Green Street" design treatments

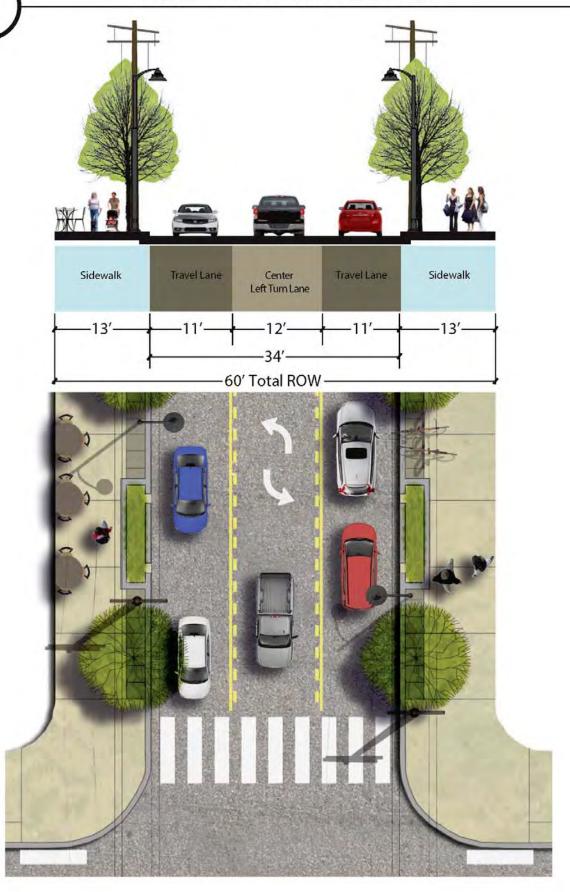


Figure 7 – Alternative 5 Concept



Alternative 6: 2-Lane with Bike Lanes, Median & Roundabouts

Alternative 6 would convert the corridor to two motor vehicle lanes with bike lanes in each direction (Figure 8). A raised median would be constructed in the middle of the roadway, with roundabouts at intersections. The curb-to-curb roadway width would not need to be modified outside of intersections. Sidewalks would be expanded to the maximum available width within the remaining right-of-way. Sidewalk and lane widths may vary across the corridor depending on the existing curb-to-curb width.

	Alternative 6 Considerations
Motor Vehicle Mobility	 Reduces number of travel lanes from four (or three) to two
	 Capacity reduced and travel time increased for through-traveling vehicles
	 Median would restrict turns at many driveways to right-in-right-out
	 Intersections with roundabouts would provide opportunities for U-turns
	 Maintains eleven-foot outside travel lanes
	 Medians and roundabouts would greatly improve corridor safety
Walkability	Consistent nine-foot sidewalks
	 Sidewalks narrower than ten-foot standard width¹⁰
	 Bike lanes provide separation from motor vehicle lanes
	 Wide median provides opportunities for pedestrian crossing refuges
	 Sidewalk width is not sufficient to support active commercial streetscape¹¹
Bicycle Facilities	Includes six-foot bike lanes
Transit Service	 Maintains eleven-foot travel lanes for buses
	Potential conflicts with bike lanes
Business Accessibility	Right-in-right-out limits motor vehicle access to driveways
	Improved bicycle access
Cost	Very high cost to construct medians and roundabouts
	 Property acquisition needed to construct appropriately-sized roundabouts
Other	 Raised median offers opportunities for streetscape design elements (e.g.,
	landscaping, pedestrian refuge, access management)
	 Impact on properties near intersections due to constructing roundabouts
	 More consistent cross-section throughout the corridor

Potential right-of-way expansions may be necessary to construct roundabouts at intersections on Willamette Street. The potential impacts would be most significant at 29th Avenue, which has traffic volumes that would likely necessitate a multi-lane roundabout. At other key intersections (e.g., 24th Avenue, 25th Avenue), single lane roundabouts may provide effective traffic control, however the right-of-way impacts of the roundabouts has yet to be determined. Due to concerns regarding right-of-way impacts, the 29th Avenue intersection could be traffic signal controlled, while roundabouts could be provided at other key intersections. The appendix includes a report ¹² excerpt that provides illustrations and a general discussion of roundabout types.

 $^{^{10}}$ Minimum width defined for curbside sidewalks in pedestrian-oriented commercial areas.

Design Standards and Guidelines for Eugene Streets, Sidewalks, Bikeways and Accessways, City of Eugene, November 1999.

¹¹ A concept for the "Heart of the Walkable Business District" characterized by a "Safe, Attractive Pedestrian Experience for Business, Shopping and Entertainment" was identified in the *South Willamette Area Draft Concept Plan*, City of Eugene, October 2012.

¹² NCHRP Report 672 - Roundabouts: An Informational Guide, 2nd Edition, National Cooperative Highway Research Program, 2010.

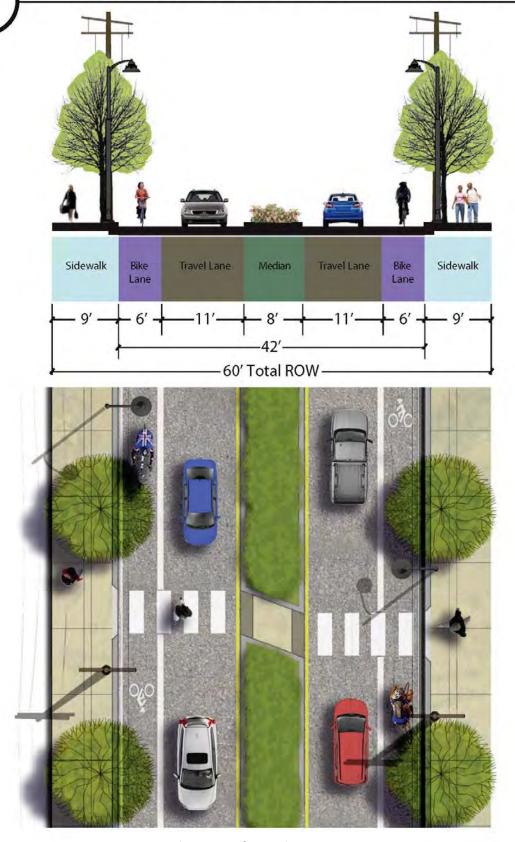


Figure 8 – Alternative 6 Concept



Alternative Facility Design Treatments

This section describes design concepts that can be incorporated into many or all of the six conceptual cross-section alternatives previously described. The design concepts are intended to better balance comfort, safety, and appeal for all users. The design elements aim to improve service for cars, trucks, buses, bikes and/or pedestrians.

Bike Facilities

Figure 9 illustrates potential bicycle facility improvements nearby, connecting to, and crossing Willamette Street. These improvements may be combined with bike lanes on Willamette Street or considered independently. The improvements could include improved bicycle access on local streets, with a variety of bike boulevard treatments applied. Crossing improvements could be provided such as intersection priority areas (i.e., "Green Boxes") or rider-activated push-button signals for crossing at intersections with traffic signals.

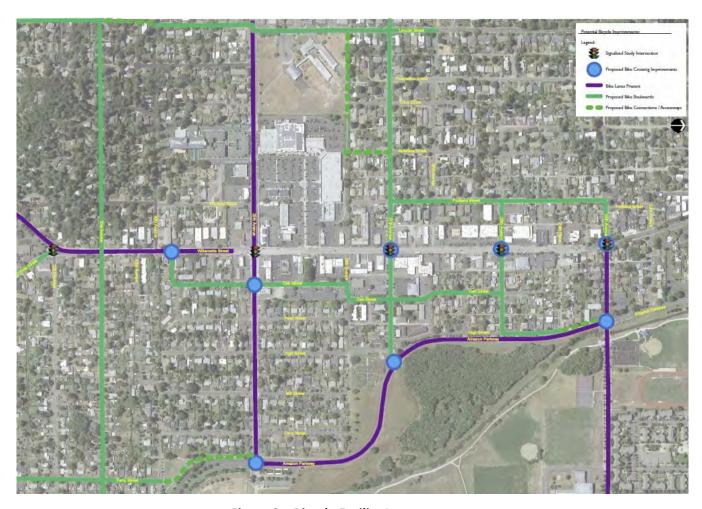


Figure 9 - Bicycle Facility Improvements



Sidewalk Widening

Existing sidewalks on Willamette Street are generally narrow with numerous obstructions and no separation from travel lanes. Each of the alternatives presented assumes sidewalks will be widened to construct the maximum allowable width within the existing right-of-way. Wider sidewalks that extend beyond the existing right-of-way may be constructed incrementally as properties redevelop.

Widening sidewalks will provide a more comfortable pedestrian environment that is accessible to more users and offers substantially greater support for the success of future businesses as the area redevelops. Wider sidewalks may also provide opportunities for landscaping, vegetation, storm water/drainage elements (e.g., bioswales), café seating, overhead signing, decorative lighting, bike parking, etc.



Figure 10 – Bioswales Source: OTAK



Figure 11 – Vegetation/Landscaping
Source: OTAK



Figure 12- Decorative Lighting



Figure 13 – Café Seating Source: OTAK



Utility Relocation

Utilities (poles, hydrants, pedestals, etc.) currently located along the sidewalks result in an inconsistent and obstructed pedestrian environment. Relocating the utilities underground would improve the sidewalk environment by removing some barriers to pedestrian access and making the corridor more aesthetically pleasing. Similar opportunities, as were identified for widened sidewalks, would become available with utility relocation, since the available sidewalk space would be increased.



Figure 14 - Utilities In Sidewalk Example

Pedestrian Crossings

A variety of design treatments can be implemented to enhance the pedestrian environment for crossings along Willamette Street.

- Signing and striping: pedestrian accessibility may be emphasized through signing or striping near intersections
- Modified pavement surface: physical differences such as raised pavement or textured crosswalks provide a visual signal to drivers to watch for pedestrians.
- Median pedestrian crossing refuges (i.e., island): pedestrians may cross a roadway in stages when a median pedestrian refuge is available. This is especially beneficial for users who require more time for crossings.



Figure 15 - Median Pedestrian Crossing Refuge

• Leading pedestrian interval: pedestrians at signalized intersections could be provided with a three- to four-second head start for entering into the crossing, before parallel traffic is given a green light. Leading pedestrian intervals allow for pedestrians to be more visible to turning vehicles.



Mid-block crossings: Opportunities for pedestrian crossings outside of existing intersections may be
provided at mid-block crossing locations. Mid-block crossings improve pedestrian access by decreasing
the distance between destinations that require crossing the roadway. A variety of design treatments
exist for mid-block crossings including rectangular rapid flashing beacons and overhead flashing
beacons.







Figure 17 - Rectangular Rapid Flashing Beacon

Driveway Consolidation

There are currently over 70 driveways on Willamette Street from 24th Avenue to 32nd Avenue. This creates numerous conflict points for motor vehicles, pedestrians and bicyclists. Managing access points along the corridor requires finding an appropriate balance between safety, mobility, and access. Consolidating driveway access points will be considered as part of each alternative if there are safety benefits that result. Reducing conflict points is likely to result in fewer accidents along the corridor. Those alternatives where a raised median may be included will need to carefully consider interaction between driveway access and the roadway design.



On-Street Parking

On-street parallel parking provides convenient access for adjacent businesses and a buffer between pedestrians and motor vehicles. On-street parking would likely have a very favorable benefit to the pedestrian environment. On-street parking may have a small impact to roadway capacity as parking maneuvers occur.

To provide on-street parking along Willamette Street, either travel lanes will need to be eliminated, or the right-of-way will need to be expanded to relocate sidewalks further from the roadway travel lanes. On-street parallel parking spots are typically eight feet wide. Figure 18 illustrates one concept regarding how on-street parking may be incorporated into the corridor. The concept effectively swaps off-street private parking for on-street public parking. This strategy may be applied along the length of the corridor or along individual blocks.

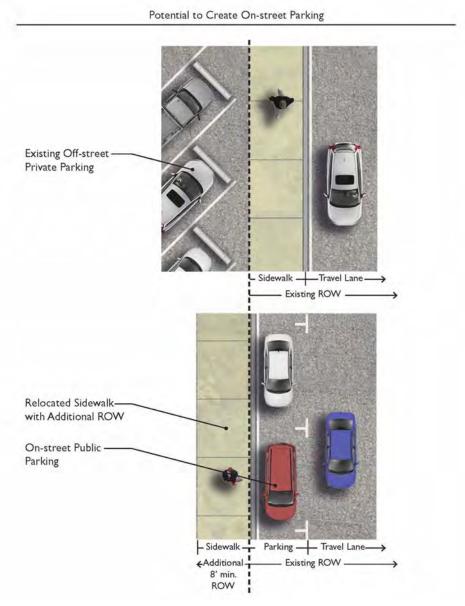


Figure 18 – Conceptual Illustration of On-Street Parking on Willamette Street



Bus Stops

Two bus routes currently provide service along Willamette Street. Improving bicycle and pedestrian access to transit stops would support transit usage along the corridor. If sidewalks are expanded there may be space available for improved bus stop amenities such as covered benches (shelters), real-time arrival information, or other transit stop amenities.

Buses currently stop on the street and block the curbside travel lane during passenger boardings. Constructing bus pullouts would remove stopped vehicles from travel lanes, but would likely require right-of-way acquisition and would also require buses in the pullouts to pull back into the traffic stream.



Figure 19 - Bus Shelter

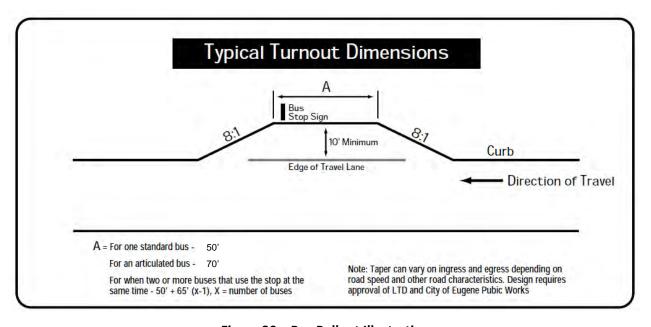


Figure 20 - Bus Pullout Illustration

Source: LTD Standards and Design

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Evaluation of Alternatives

The following section provides Tier 1 screening evaluation of the six proposed alternative cross-section concepts. The alternatives were scored using the evaluation criteria and methodology previously detailed in Technical Memorandum #1 (South Willamette Street Improvement Plan – Evaluation Criteria). Evaluation criteria were established to assess the potential of alternatives to best meet the transportation needs of the users of Willamette Street. The criteria are based on the goals and objectives in the Draft Eugene Transportation System Plan.

Individual criteria were scored as 1, 0, or -1 representing improvement, no change, or degradation, respectively. The scoring weighs all criteria equally, with the total evaluation score representing the sum of the individual criteria scores. Figure 21 documents the results of the scoring evaluation. Alternatives 3 and 7 provide the two highest scores while Alternatives 1 and 2 have the lowest scores. The same result occurs whether equal weighting is given to the eight goal categories or the 23 individual criteria. Criteria and scoring for each alternative are further detailed in the appendix.

Next Steps

The preliminary evaluation of alternatives (Tier 1 screening) and all information included in this memorandum will be distributed to project stakeholders and presented to the community. The Tier 1 screening process will be used to identify up to three alternatives to be considered and further evaluated in the Tier 2 screening. Tier 2 screening assessments will include more detailed description and rigorous analysis of the facility design.



	Ac	cess &	& Mobil	ity	Safe	ty & He	ealth		cial uity	Ed	conomi	c Bene	efit	Co Effectiv		CI	imate (& Ener	gy	Ecolo Fund	ogical ction		nunity	
Alternative	Reliability	Neighborhood Connectivity	Motor Vehicle Travel Time	Alternative Mode Travel Time	Safety	Security	Emergency Access	Equity	Economic Access	Freight Mobility	Walkable/Bikeable Business District	Business Vitality	Fundability	Asset Management	Project Benefits	Reduce Vehicle Miles Traveled	Pedestrian Facilities	Bicycle Facilities	Transit Facilities	Storm water Design	Landscape Design	Community Vision and Land Use	Transportation Planning Compatibility	TOTAL
Alternative 1 - 4-Lane	0	0	0	0	0	0	0	0	0	0	0	0	1	1	1	0	0	0	0	0	0	0	0	3
Alternative 2 - 4-Lane with Center Left-turn Lane	0	0	0	0	0	0	0	0	0	0	0	1	0	1	1	0	0	0	0	0	0	0	0	3
Alternative 3 - 3-Lane with Bike Lanes	0	1	-1	1	1	1	-1	1	1	-1	1	0	0	1	1	0	0	1	0	0	0	0	0	7
Alternative 4 - 3-Lane with Buffered Bike Lanes	0	1	-1	1	1	1	-1	1	1	-1	1	0	-1	1	1	0	-1	1	0	0	0	-1	0	4
Alternative 5 - 3-Lane with Wide Sidewalks	0	0	-1	0	0	1	-1	1	1	-1	1	0	-1	1	1	0	1	0	1	0	0	1	0	5
Alternative 6 - 2-Lane with Bike Lanes, Median & Roundabouts	0	1	-1	1	1	1	-1	1	1	-1	1	-1	-1	1	1	0	0	1	0	0	0	0	0	5

FIGURE 21 – Evaluation Criteria Scoring



South Willamette Street Improvement Plan – Tier 1 Screening Evaluation Notes

P10086-012

This document describes the evaluation criteria and scoring approach used to analyze the alternatives developed for the South Willamette Street Improvement Plan. A point-based rating methodology is used to evaluate how well proposed design alternatives meet measure of effectiveness criteria. In this way, a consistent method is used to evaluate and rank the alternatives based on how well they meet identified goals and objectives. The ranking informs the Technical Advisory Committee, stakeholders and appointed and elected officials; however, the final recommended alternative will be based on feedback and direction from these parties.

The goals and objectives in the Draft TSP provided a basis for the development of the evaluation criteria, which are intended to assess an alternative's potential to meet the needs of the people using Willamette Street. The evaluation criteria were refined based on a review of planning documents more specific to the study area, such as the South Willamette Concept Plan. The criteria are summarized in Table 1 according to the broad goal category they support.

Criteria scoring could differ for the south and north corridor segments due to differences in existing configurations (e.g., 4 lanes in north compared to 3 in south). For clarity in comparing scoring across all alternatives, the scoring was focused on changes relative to the existing configuration on the north segment of the corridor. The north corridor applies to all scenarios, makes up the longest segment in the corridor, and may be considered the most critical segment due to the surrounding commercial land uses.

The overall evaluation approach assumes an equal weight for each criterion. The evaluation scores for all criteria are summed to determine the overall evaluation score. This method allows a goal category with more supporting criteria to have a larger influence on the overall score.

The scoring methodology may be modified (or combined) to be applied in different ways. Two alternative methodologies:

- Equal weight for each goal category

 Each of the eight categories receives an equal weight. In
 this method, evaluation scores for each criterion under a particular goal category would be averaged
 to determine one score for each goal category. They would then be summed to arrive at an overall
 evaluation score.
- Stakeholder feedback to determine weight
 — Feedback from stakeholders would be solicited to
 help determine the weight of each goal category. Criteria scores for a particular category would be
 averaged and the weight would then be applied.

Table 1 summarizes the criteria, describes the evaluation score and notes how the scoring was applied to the seven alternatives considered in the Tier 1 screening.



Table 1: Evaluation Criteria and Scoring

Criteria	Evaluation Score	Scoring Notes
Access and Mobility		
Reliability Improves trip reliability, consistency, comfort and convenience for all modes (walk, bike, transit, cars).	+1. Improves trip reliability 0. No change -1. Reduces trip reliability	Each alternative represents tradeoffs within limited space. No alternative improves or degrades all modes.
Neighborhood Connectivity Increases the number of households that can safely walk, bike, or use transit services to meet basic (non-work) daily needs.	+1. Increases # of connected households 0. No change -1. Decreases # of connected households	Alternatives that provide bike lanes were considered to represent improvement. Sidewalk width was not considered to significantly affect number of connected households.
Motor Vehicle Travel Time Reduces travel time between key origins and destinations for motor vehicles.	 +1. Decreases travel time for motor vehicles 0. No change -1. Increases travel time for motor vehicles 	Alternatives with less than 4 motor vehicle lanes where scored '-1' due to reduced capacity.
Alternative Mode Travel Time Reduces travel time between key origins and destinations for alternative modes.	+1. Decreases travel time for alternative modes 0. No change -1. Increases travel time for alternative modes	Alternatives that provide bike lanes were considered to represent improvement. Sidewalk width was not considered to significantly affect travel time.
Safety and Health		
Safety Improve safety and security for all users, especially for the most vulnerable; strive for zero fatalities.	+1. Improves safety for all modes 0. No change -1. Reduces safety for all modes	Alternatives with 3 vehicle lanes are expected to lower vehicle speeds. Bike lanes provide a dedicated facility and a buffer for pedestrians. Safety impacts of wider sidewalks without bike lanes (Alternative 5) are unclear.
Security Improve actual and perceived sense of security (i.e. Safe driving, getting to and riding transit, walking and biking).	+1. Improves sense of security 0. No change -1. Decreases sense of security	Wider sidewalks and bike lanes provide a buffer between motor vehicle lanes, improving sense of security for users.



(Continued) Table 1: Evaluation Criteria and Scoring

Criteria	Evaluation Score	Scoring Notes		
Emergency Access Improves or maintains emergency response times within and through the corridor.	 +1. Improves emergency response times 0. No change -1. Reduces emergency response times 	Alternatives with less than 4 motor vehicle lanes where scored '-1' due to narrower space for vehicle lanes and greater likelihood of vehicle blockage/congestion.		
Social Equity				
Equity Contributes to closing the transportation access gap between the general user and populations with limited choices, such as the elderly, low income, minority populations, and people with disabilities.	 +1. Specifically benefits populations with limited choices 0. No Change -1. Negatively impacts populations with limited choices 	Alternatives that provide dedicated bike lanes or sidewalks beyond 10' are considered to benefit users with limited choices.		
Economic Access Improves access from residences to employment and neighborhood centers within a 20-minute walk, bike, or transit trip.	+1. Improves employment access 0. No change -1. Decreases employment access	Alternatives that provide dedicated bike lanes or sidewalks beyond 10' are considered to improve access within 20-minute walk, bike, or transit trip.		
Economic Benefit				
Freight Mobility Provides safe, efficient, and continuous motor vehicle operation to allow timely freight movement along Willamette Street.	 +1. Improves corridor's freight movement 0. No Change -1. Negative impact on freight movement 	Alternatives with less than 4 motor vehicle lanes where scored '-1' due to reduced capacity.		
Walkable/Bikeable Business District Promotes a "Safe, Attractive Pedestrian Experience for Business, Shopping and Entertainment."	 +1. Improves business district pedestrian and bicycle experience 0. No change -1. Reduces business district pedestrian and bicycle experience 	Alternatives that provide dedicated bike lanes or wider sidewalks are considered to improve experience for pedestrians and people on bikes.		
Business Vitality Supports access and visibility of businesses that rely on drive-by traffic by balancing congestion with economic vitality	+1. Supports economic vitality0. No change-1. Negative impact on economic vitality	Adding a center turn lane is a benefit to business traffic. Reducing lanes is considered negative. Doing both is neutral.		



(Continued) Table 1: Evaluation Criteria and Scoring

Criteria	Evaluation Score	Scoring Notes
Fundability Available funding sources exist to implement projects in a timely fashion.	 +1. Funding sources are available 0. Feasible costs, but no identified funding -1. High costs and no funding expected 	Available funding for maintaining existing alignment (Alt 1. Maintaining curb-to-curb width is considered feasible. Moving curbs is considered high cost.
Cost Effectiveness		
Asset Management Favors the enhancement and maintenance of existing systems over system expansion.	+1. Enhances existing transportation system 0. Minimal enhancement or expansion -1. Expands transportation system	All alternatives are considered enhancements of the existing transportation system.
Project Benefits Optimizes benefits relative to public, private and social costs over the life-cycle of the project	+1. Provides maximum benefits 0. Minimal benefits -1. Provides no benefits	Each alternative provides benefits, but reflects tradeoffs necessary withir limited space.
Climate and Energy		
Reduce Vehicle Miles Traveled (VMT) Improves the corridor as an attractive area without having to drive. Increases mode share for walk, bike, and transit thus reducing greenhouse gases and fossil fuel consumption.	+1. Reduces VMT 0. No change -1. Increases VMT	VMT impacts are unclear. Bike lanes may reduce VMT. Fewer lanes may result in out-of-direction travel. VMT Impact of median and roundabout is also unclear.
Pedestrian Facilities Adds sidewalks and crosswalks that fill in system gaps, improve system connectivity, removes obstructions and are accessible to all users.	+1. Improves pedestrian facilities0. No change-1. Negative impact on pedestrian facilities	Pedestrian improvements are expected to be included in each scenario due to widening of sidewalks and other desig treatments. Therefore, criterion is applied relative to change from "baseline" enhanced 9' sidewalk.



(Continued) Table 1: Evaluation Criteria and Scoring

Criteria	Evaluation Score	Scoring Notes		
Bicycle Facilities Adds bikeways that fill in system gaps, improve system connectivity, and are accessible to all users.	 +1. Improves bicycle facilities, including bike lanes 0. No change -1. Negative impact on bicycle facilities 	Alternatives that provide dedicated bike lanes are considered '+1', otherwise '0'.		
Transit Facilities Improves transit facilities and accessibility to transit stops (for all users) along and near the corridor.	 +1. Improves transit facilities 0. No change -1. Negative impact on transit facilities 	Wider sidewalk (Alt 5) is only alternative considered to have a significant benefit to transit stop accessibility		
Ecological Function				
Stormwater Design Transportation improvements lower the rate of storm water runoff and improve water quality.	+1. Minimizes storm water runoff 0. No change -1. Increases storm water runoff	No storm water runoff impacts have been identified.		
Ecological Function (continued)				
Landscape Design Reduces the urban heat island through landscape design, less pavement, and increased tree canopy.	+1. Reduces heat island 0. No change -1. Increases heat island	No landscape design impacts have been identified.		
Community Context				
Community Vision and Land Use Supports implementation of Envision Eugene land use and growth management goals and A Community Climate and Energy Action Plan for Eugene.	+1. Supports Envision Eugene0. No change-1. Conflicts with Envision Eugene	Criterion is applied based on sidewalk width (relative to "baseline" enhanced 9' sidewalk).		
Transportation Planning Compatibility Compatible with City's transportation plans (TSP, Long Range Transit Plan, and Pedestrian and Bicycle Master Plan [PBMP])	 +1. Compatible with City transportation plans 0. Has little or no impact (or has offset impacts) -1. Not compatible with City transportation plans 	Each alternative represents tradeoffs within limited space. No alternative is considered to significantly differ in overall compatibility.		

NCHRP REPORT 672

NATIONAL COOPERATIVE HIGHWAY RESEARCH PROGRAM

Roundabouts: An Informational Guide

Second Edition

In Cooperation with



U.S. Department of Transportation

Federal Highway Administration

TRANSPORTATION RESEARCH BOARD
OF THE NATIONAL ACADEMIES

Exhibit 1-9 Roundabout Category Comparison

Design characteristics of the three roundabout categories.

Mini-roundabouts can be
useful in low-speed urban
environments with right-of-way
constraints.

Exhibit 1-10 Features of Typical Mini-Roundabout

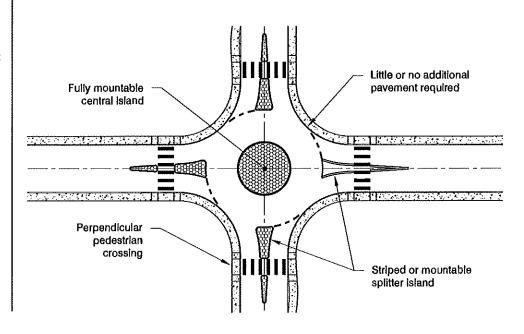
Design Element	Mini-Roundabout	Single-Lane Roundabout	Muitilane Roundabout
Desirable maximum entry design speed	15 to 20 mph (25 to 30 km/h)	20 to 25 mph (30 to 40 km/h)	25 to 30 mph (40 to 50 km/h)
Maximum number of entering lanes per approach		1 (1)	2+
Typical inscribed circle diameter	45 to 90 ft (13 to 27 m)	90 to 180 ft (27 to 55 m)	150 to 300 ft (46 to 91 m)
Central island treatment	Fully traversable	Raised (may have traversable apron)	Raised (may have traversable apron)
Typical daily service volumes on 4-leg roundabout below which may be expected to operate without requiring a detailed capacity analysis (veh/day)*	Up to approximately 15,000	Up to approximately 25,000	Up to approximately 45,000 for two-lane roundabout

^{*}Operational analysis needed to verity upper limit for specific applications or for roundabouts with more than two lanes or four legs.

In most cases, roundabouts in all three categories are designed with pedestrian and bicycle facilities; however, in some instances a jurisdiction may choose to not provide these features if these types of users are not anticipated or can be better served in another location.

1.3.1 MINI-ROUNDABOUTS

Mini-roundabouts are small roundabouts with a fully traversable central island. They are most commonly used in low-speed urban environments with average operating speeds of 30 mph (50 km/h) or less. Exhibit 1-10 shows the features of typical mini-roundabouts, and Exhibit 1-11 provides an example. They can be useful in such environments where conventional roundabout design





Dimondale, Michigan

is precluded by right-of-way constraints. In retrofit applications, mini-roundabouts are relatively inexpensive because they typically require minimal additional pavement at the intersecting roads and minor widening at the corner curbs. They are mostly recommended when there is insufficient right-of-way to accommodate the design vehicle with a traditional single-lane roundabout. Because they are small, mini-roundabouts are perceived as pedestrian-friendly with short crossing distances and very low vehicle speeds on approaches and exits.

A fully traversable central island is provided to accommodate large vehicles and serves one of the distinguishing features of a mini-roundabout. The mini-roundabout is designed to accommodate passenger cars without requiring them to traverse over the central island. The overall design of a mini-roundabout should align vehicles at entry to guide drivers to the intended path and minimize running over of the central island to the extent possible.

1.3.2 SINGLE-LANE ROUNDABOUTS

This type of roundabout is characterized as having a single-lane entry at all legs and one circulatory lane. Exhibit 1-12 shows the features of typical single-lane roundabouts, and Exhibit 1-13 provides examples. They are distinguished from mini-roundabouts by their larger inscribed circle diameters and non-traversable central islands. Their design allows slightly higher speeds at the entry, on the circulatory roadway, and at the exit. The geometric design typically includes raised splitter islands, a non-traversable central island, crosswalks, and a truck apron. The size of the roundabout is largely influenced by the choice of design vehicle and available right-of-way.

1.3.3 MULTILANE ROUNDABOUTS

Multilane roundabouts have at least one entry with two or more lanes. In some cases, the roundabout may have a different number of lanes on one or

Exhibit 1-11 Example of Mini-Roundabout

Exhibit 1-12Features of Typical Single-Lane Roundabout

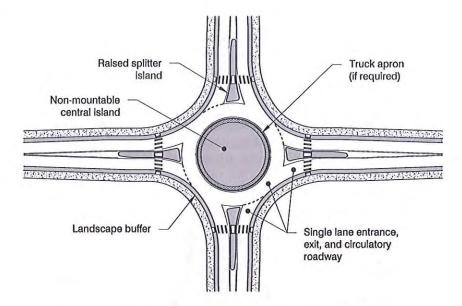


Exhibit 1-13 Examples of Single-Lane Roundabouts



(a) Dublin, Ohio



(b) Skagit County, Washington

more approaches (e.g., two-lane entries on the major street and one-lane entries on the minor street). They also include roundabouts with entries on one or more approaches that flare from one to two or more lanes. These require wider circulatory roadways to accommodate more than one vehicle traveling side by side. Exhibit 1-14 through Exhibit 1-16 provide examples of typical multilane roundabouts. The speeds at the entry, on the circulatory roadway, and at the exit are similar or may be slightly higher than those for the single-lane roundabouts. The geometric design will include raised splitter islands, truck apron, a non-traversable central island, and appropriate entry path deflection.

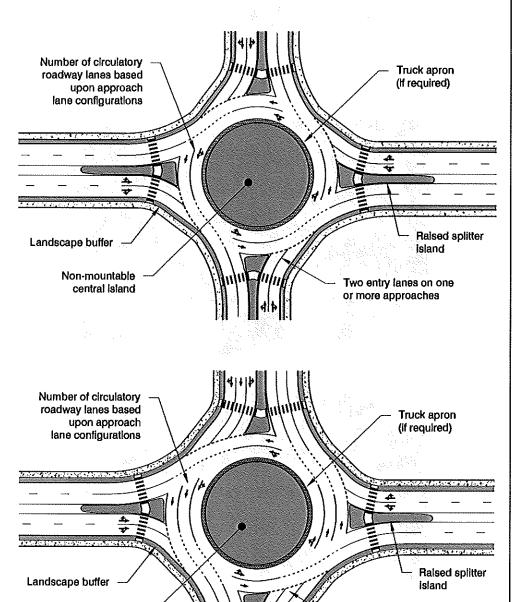


Exhibit 1-14Features of Typical Two-Lane Roundabout

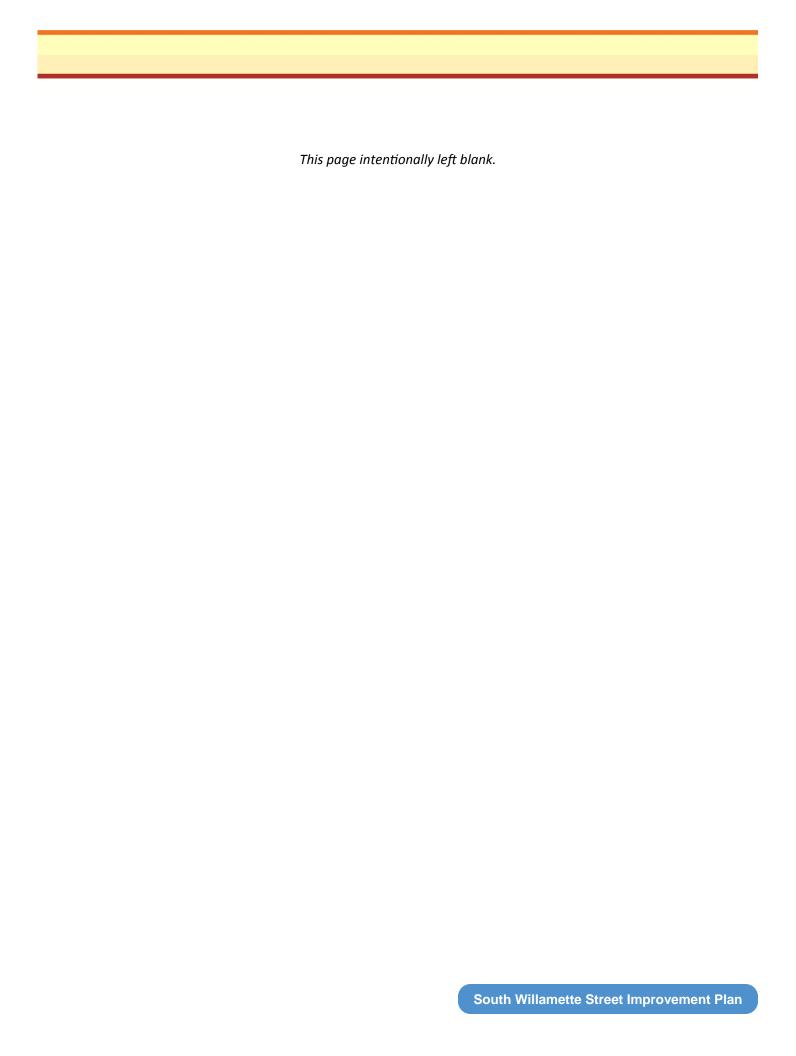
Exhibit 1-15Features of Typical
Three-Lane Roundabout

Non-mountable

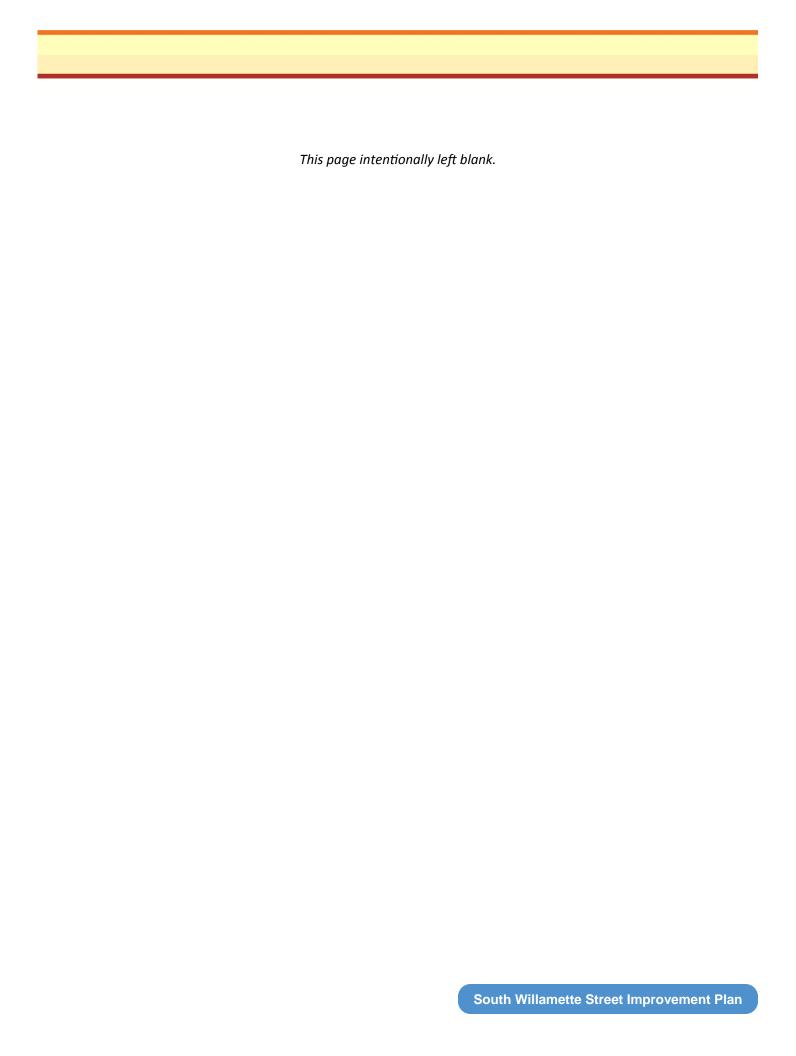
central island

Three entry lanes on one or more

approaches



SECTION I FACILTY DESIGN ALTERNATIVES







117 Commercial Street NE Suite 310 Salem, OR 97301 503.391.8773 www.dksassociates.com

MEMORANDUM #7

DATE: June 12, 2013

TO: Project Management Team

FROM: Scott Mansur, Mat Dolata and Peter Coffey - DKS Associates

Tom Litster and Kaitlin North - OTAK

SUBJECT: South Willamette Street Improvement Plan

Facility Design Alternatives

P10086-012

This memorandum summarizes facility design options for three alternatives advanced for the South Willamette Street Improvement Plan. The alternative configurations are illustrated through cross-section diagrams and overhead plan views that show configurations for travel lanes, bike lanes, sidewalks, and other roadway elements. The memorandum also presents the (Tier 2) screening evaluation results of three alternatives, as well as cost estimates and discussion of streetscape elements, bicycle and pedestrian connections to the corridor, and other design considerations.

Overview

The South Willamette Street Improvement Plan will explore options for people to easily and safely walk, bike, take transit, or drive in an eight-block study area from 24th Avenue to 32nd Avenue. The goal of the study is to help South Willamette Street become a vibrant urban corridor accessible by bicycle, foot, car, and bus. Seven conceptual alternatives have been refined to three based on direction from the City of Eugene staff after receiving community input and reviewing the results of the Tier 1 Screening.

The Tier 1 screening evaluated community priorities and identified broad level tradeoffs that exist within a constrained right-of-way. The screening provided a qualitative assessment for each alternative based on criteria and scoring methodology identified in Technical Memorandum #1 (South Willamette Street Improvement Plan – Evaluation Criteria).

A broad level of public involvement was vital to the screening process. Public input was received through letters, phone calls, emails, and in-person at stakeholder outreach meetings, focus groups, meetings with Planning Commission and City Council, and two community forums. Community Forum #2 (February 27th, 2013) was attended by more than 300 people, who provided input on the seven initial alternative concepts and facility design elements.

The three alternative configurations advanced to the next screening phase are a 4-lane (Alternative 1), 3-lane with bike lanes (Alternative 3) and 3-lane with wide sidewalks (Alternative 5.) This memorandum, together with Technical Memorandum #8 (Traffic Analysis for Roadway Alternatives), provides the more detailed description and rigorous analysis of the facility design needed to perform the next level (Tier 2) screening.

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With the information presented in these memorandums and the Tier 2 screening results, this process will progress toward a selected design for the corridor.

Alternative Cross-Sections

The three alternatives are illustrated via cross-sections and overhead views (Figures 1 through 3). Plan views for the entire corridor (from 24th Avenue to 32nd Avenue) are included in the appendix. Most significant differences among the alternatives occur in the corridor segment between 29th Avenue and 24th Avenue.

Summary of Potential Changes by Segment

Some planned improvements are desired throughout the corridor and will be assumed for each alternative. These improvements include new pavement, improved drainage, wider sidewalks, and enhancements to pedestrian and bicycle access around Willamette Streets. Other improvements may vary depending on the location and alternative configuration. The following section describes an overview of potential differences by roadway segment.

24th **Avenue to 28**th **Avenue Roadway Configuration:** The cross-sections and overhead views in Figures 1 to 3 represent the roadway configuration between 24th Avenue and near 28th Avenue. Alternative 1 maintains the existing 4-lane roadway. Alternative 3 illustrates a 3-lane roadway (two travel lanes and a continuous center turn lane) and continuous bike lanes. Alternative 5 is also a 3-lane alternative, but with widened sidewalks rather than continuous bike lanes.

24th **Avenue to 28**th **Avenue Sidewalk Configuration:** All three alternatives attempt to maximize the sidewalk width within the existing right-of-way. For Alternative 1 and Alternative 3, the sidewalks would be reconstructed to approximately 9-feet wide. For Alternative 5, the sidewalk widths would expand to approximately 13-feet wide by replacing the bike lanes illustrated for Alternative 3 with additional sidewalk space.

28th Avenue to 30th Avenue Roadway Configuration: This section is a "transition area" from the proposed cross-sections identified for each conceptual alternative, through the 29th Avenue intersection to near 30th Avenue. Alternative 1 would maintain the existing roadway configuration, which widens from one northbound motor vehicle lane to two (and a left-turn pocket at 29th Avenue) and widens between the Woodfield Station Driveway and 29th Avenue to add a southbound left-turn pocket to the two existing southbound motor vehicle through lanes. The northbound bike lane would end at 29th Place and the southbound bike lane would begin south of 29th Avenue, as currently configured.

In Alternative 3, the existing bike lanes would be extended northward through the 29th Avenue intersection in order to provide continuous bike lanes between 32nd Avenue and 24th Avenue. Adding bike lanes would require either expanding the curb-to-curb width of the roadway or removing a motor vehicle lane. Widening the curb-to-curb width would likely require narrower sidewalks or additional right-of-way near the 29th Avenue intersection. A proposed design modification presented for Alternative 3 (and Alternative 5) would add a

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second southbound travel lane just north of the Woodfield Station Driveway, but not include a second northbound through travel lane (included in Alternative 1).

The configuration of travel lanes for Alternative 5 would be similar to Alternative 1 for bike lanes and Alternative 5 for motor vehicle lanes. Bike lanes would begin (southbound) and end (northbound) south of the 29th Avenue intersection. A single northbound motor vehicle through lane would be included, instead of the two existing lanes. The additional space made available by potentially not including a second northbound travel lane in this section would accommodate wider sidewalk space rather than the bike lanes provided in Alternative 3.

28th **Avenue to 30**th **Avenue Sidewalk Configuration:** Sidewalk widths in this "transition area" could vary depending on the specific design of motor vehicle lanes, turn pocket lengths, bike lanes, etc. In general, Alternative 5 provides the narrowest curb-to-curb width and therefore the most space for sidewalks and pedestrian amenities within the existing right-of-way.

29th **Place to 32**nd **Avenue Roadway Configuration:** No changes to the existing travel and bike lane configurations are proposed in any alternative between 32nd Avenue and near 29th Place (where the existing northbound bike lane ends).

29th **Place to 32**nd **Avenue Sidewalk Configuration:** All three alternatives would expand sidewalk widths to approximately 8.5 feet, or the maximum available within the existing right-of-way.

The following sections provide illustrations and a descriptive overview of each alternative configuration.

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Alternative 1: 4-Lane (No-Build)

Alternative 1 maintains the existing (curb-to-curb) roadway configuration. Sidewalks would be expanded to their maximum width (approximately nine feet) within the existing right-of-way. The cross-section illustration shown for Alternative 1 is not being considered for the south corridor segment because it does not include any dedicated bicycle facilities and no parallel facilities are available near Willamette Street, south of 30th Avenue. Plan views for the entire corridor (from 24th Avenue to 32nd Avenue) are included in the appendix.

	Alternative 1 Considerations
Motor Vehicle Mobility	 Maintains existing four travel lanes
	 Left-turning vehicles block travel lanes
Walkability	Consistent nine-foot sidewalks
	 Sidewalks narrower than ten-foot standard width¹
	 Sidewalk width is not sufficient to support active commercial streetscape²
Bicycle Facilities	No on-street bike lanes
	 Improved bike access would occur via parallel route improvements and
	crossing enhancements (see Figure 5)
	Bike sharrows possible on curbside lanes
Transit Service	Maintains eleven-foot outside travel lane for buses
Cost	Relatively low cost to maintain current cross-section

¹ Minimum width defined for curbside sidewalks in pedestrian-oriented commercial areas.

Design Standards and Guidelines for Eugene Streets, Sidewalks, Bikeways and Accessways, City of Eugene, November 1999.

² A concept for the "Heart of the Walkable Business District" characterized by a "Safe, Attractive Pedestrian Experience for Business, Shopping and Entertainment" was identified in the South Willamette Area Draft Concept Plan, City of Eugene, October 2012.

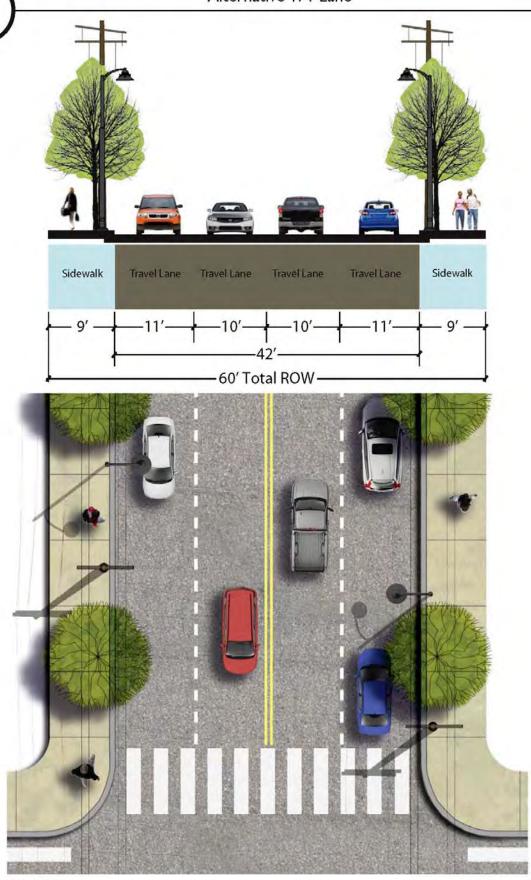


Figure 1 – Alternative 1 Concept

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Alternative 3: 3-Lane with Bike Lanes

Alternative 3 would provide one northbound through lane, one southbound through lane, a two-way center left-turn lane, and a bike lane in each direction (Figure 2). This configuration would convert most of the segment north of 29th Avenue from four motor vehicle lanes to three, while adding two bike lanes. Plan views for the entire corridor (from 24th Avenue to 32nd Avenue) are included in the appendix.

Sidewalks would be expanded to the maximum available width within the remaining right-of-way. Sidewalk and lane widths may vary across the corridor depending on the existing curb-to-curb width.

Alternative 3 Considerations	
Motor Vehicle Mobility	 Reduces number of travel lanes from four to three, north of 28th Avenue Capacity reduced and travel time increased for through-traveling vehicles Ten-foot travel lanes are narrow for trucks and less than the eleven-foot standard width³
Walkability	 Consistent nine-foot sidewalks Sidewalks narrower than ten-foot standard width⁴ Bike lanes provide separation from motor vehicle lanes Sidewalk width is not sufficient to support active commercial streetscape⁵
Bicycle Facilities	Includes six-foot bike lanes
Transit Service	 Ten-foot travel lanes are narrow for buses Potential conflicts with bike lanes
Business Accessibility	 Center turn lane improves access for turning vehicles Improved bicycle access
Cost	 Moderate cost to provide center left turn lane and bike lanes Intersections and traffic signals would need to be reconfigured
Other	 Center turn lane offers opportunities for design elements including median treatments (e.g., landscaping, pedestrian refuge, access management)

³ Minimum travel lane width on Minor Arterials is 11 feet

Design Standards and Guidelines for Eugene Streets, Sidewalks, Bikeways and Accessways, City of Eugene, November 1999

⁴ Minimum width defined for curbside sidewalks in pedestrian-oriented commercial areas.

Design Standards and Guidelines for Eugene Streets, Sidewalks, Bikeways and Accessways, City of Eugene, November 1999.

⁵ A concept for the "Heart of the Walkable Business District" characterized by a "Safe, Attractive Pedestrian Experience for Business, Shopping and Entertainment" was identified in the South Willamette Area Draft Concept Plan, City of Eugene, October 2012.

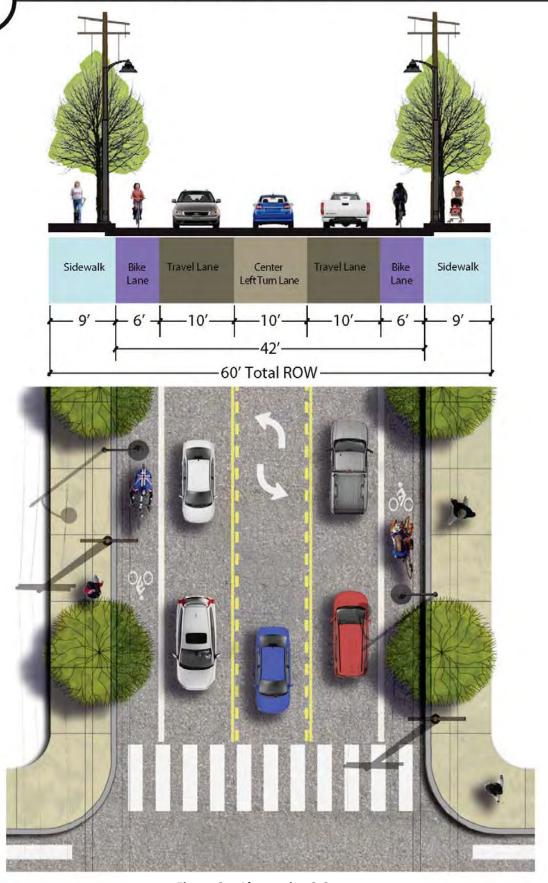


Figure 2 – Alternative 3 Concept





Alternative 5: 3-Lane with Wide Sidewalks

Alternative 5 would convert most of the roadway segment north of 29th Avenue from four motor vehicle lanes to three (Figure 3). The roadway would be reconstructed to expand sidewalks, resulting in a narrower curb-to-curb width (34 feet instead of the current 41 to 42 foot width.) No new bike lanes would be included on Willamette Street. Plan views for the entire corridor (from 24th Avenue to 32nd Avenue) are included in the appendix.

Sidewalks would be expanded to the maximum available width within the remaining right-of-way. With the 34-foot curb-to-curb width, sidewalks could be extended up to 13-feet. Alternative 5 would not result in any changes to roadway configuration south of 30th Avenue because it does not include any dedicated bicycle facilities and no parallel facilities are available near Willamette Street, south of 30th Avenue.

	Alternative 5 Considerations
Motor Vehicle Mobility	 Reduces number of travel lanes from four to three, north of 28th Avenue Capacity reduced and travel time increased for through-traveling vehicles Maintains eleven-foot outside travel lanes
Walkability	 Provides wide (13-foot) sidewalks to facilitate a transformative pedestrian environment including design treatments (e.g., storefront displays, café seating, landscaping)
Bicycle Facilities	 No on-street bike lanes Improved bike access would occur via parallel route improvements and crossing enhancements (see Figure 5) Bike sharrows possible on curbside lanes Potential to provide raised bike facility if additional right-of-way acquired for sidewalk widening and reconstruction
Transit Service	Maintains eleven-foot travel lanes for buses
Business Accessibility	 Center turn lane improves access for turning vehicles Wide sidewalks provide opportunities for design treatments to support commercial development, aesthetic treatments, and walkability
Cost	 Higher cost to reconstruct curbs to expand/reconstruct sidewalks Intersections and traffic signals would need to be reconfigured
Other	 Center turn lane offers opportunities for design elements including median treatments (e.g., landscaping, pedestrian refuge, access management) Wide sidewalks support "Green Street" design treatments

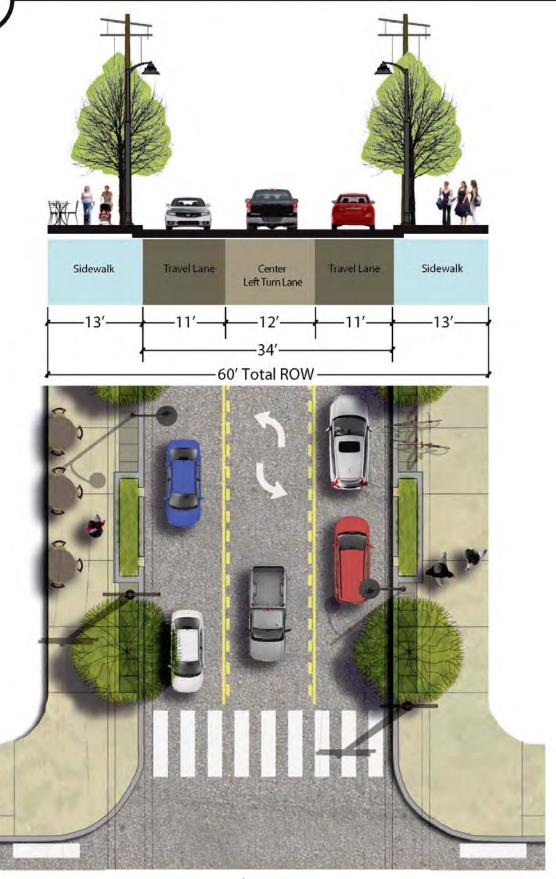


Figure 3 – Alternative 5 Concept

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Street Intersection Impacts

The following section describes how each alternative would be accommodated at the study intersections. Plan views displaying intersection configurations for each alternative are included in the appendix. Traffic analysis showing intersection operations for each alternative is included in Technical Memorandum #8 (Traffic Analysis for Roadway Alternatives).

24th **Avenue Intersection:** No changes to right-of-way or curb-to-curb width are proposed at the intersection in Alternatives 1 or 3. In Alternative 5, the south leg of Willamette Street would be reconstructed with curb-to-curb width narrowed to accommodate wider sidewalks. In Alternative 3 and Alternative 5, the south leg of Willamette Street would be reconfigured from four travel lanes to three lanes (one lane in each direction with a center left turn lane in the middle). The space gained from removing one of the four travel lanes would be used for either bicycle lanes (Alternative 3) or wider sidewalks (Alternative 5). The north leg of Willamette Street would convert from two through lanes to one through lane and a dedicated left turn lane. The traffic signal would also need to be modified in Alternatives 3 and 5. No changes to right-of-way are proposed at the intersection in any alternative.

25th **Avenue Intersection & 27**th **Avenue Intersection:** The configuration of these intersections would appear as depicted in the overhead views shown in Figures 1-3. No changes to right-of-way or curb-to-curb width are proposed in Alternatives 1 or 3, while sidewalks are expanded in Alternative 5. Traffic signals would need to be reconfigured to accommodate the 3-lane configuration identified in Alternative 3 and Alternative 5. No changes are identified for 25th Avenue or 27th Avenue approaches at Willamette Street.

Woodfield Station Driveway Intersection: It is recommended that a traffic signal at this intersection be considered as a design option in all alternatives. A traffic signal would provide better access for turning vehicles and an additional pedestrian crossing opportunity. No changes to the existing lane configuration would be needed in Alternative 1. In Alternative 3 and Alternative 5, there would be a left turn lane on the northbound approach, and a single northbound through travel lane. Southbound, one travel lane would widen to two approximately 100 feet north of the intersection. Driveway modifications would likely be necessary on the east side of Willamette Street, across from the Woodfield Station Driveway. No right-of-way changes are anticipated in any of the alternatives. Sidewalks will be extended within the existing right-of-way.

29th **Avenue Intersection:** Compared to other study intersections, 29th Avenue has significantly higher traffic volumes (see Table 1). To adequately serve the traffic demand at the intersection and meet City of Eugene traffic operations performance standards, the Willamette Street approaches require more than a single through lane on each approach. The plan view figure for Alternative 1 illustrates a 5-lane cross-section at 29th Street, as exists currently. For Alternative 3 and 5, the proposed design option would include a 4-lane cross-section at 29th Avenue including a single northbound travel lane. Removing one of the two existing northbound travel lanes may be considered to accommodate bike lanes or wider sidewalks, respectively. Without reducing the number of vehicle lanes, additional right-of-way would be required to provide bike lanes or wider sidewalks. Further discussion of the alternative configurations at 29th Avenue is included in Technical Memorandum #8 (Traffic Analysis for Roadway Alternatives).





32nd **Avenue Intersection:** No changes are proposed in any alternative to this intersection.

Table 1: Intersection Traffic Volume (2012 p.m. peak hour)

Intersection	Traffic Entering Volume
Willamette Street/24 th Avenue	1,834
Willamette Street/25 th Avenue	1,668
Willamette Street/27 th Avenue	1,914
Willamette Street/Woodfield Station Driveway	1,706
Willamette Street/29 th Avenue	2,732
Willamette Street/32 nd Avenue	1,613

Roundabout Compatibility

Roundabouts can improve traffic flow and reduce overall delay at many roadway intersections. Roundabouts generally reduce the number of overall collisions and fatalities when they are installed and are less expensive to operate and maintain compared to traffic signals. However, emergency vehicle and freight users may be opposed to roundabouts in sensitive areas.

Roundabouts would need to be constructed with multiple lanes to serve the four travel lines included in Alternative 1. The three-lane configurations (Alternatives 3 and 5) could be constructed with single lane roundabouts; however, the traffic analysis results (shown in Technical Memorandum #8) indicate that single lane roundabouts may not comfortably accommodate peak hour traffic demand at several intersections. Multilane roundabouts could be considered but would require a larger intersection configuration.

These larger configurations would require property acquisition to provide the right-of-way needed to construct the appropriately-sized roundabouts. Right-of-way acquisition can have significant costs and impacts to adjacent properties, particularly in a developed commercial area. Figure 4 illustrates an example, showing a potential layout for the intersection of 29th Avenue and Willamette Street with a multi-lane roundabout. While other intersections on Willamette Street could be configured with smaller layouts, the impacts and costs for the corridor may be significant even if the 29th Avenue intersection remained as currently configured. Roundabouts are not explicitly included in the facility design of any alternative but may be considered further as potential design refinements.





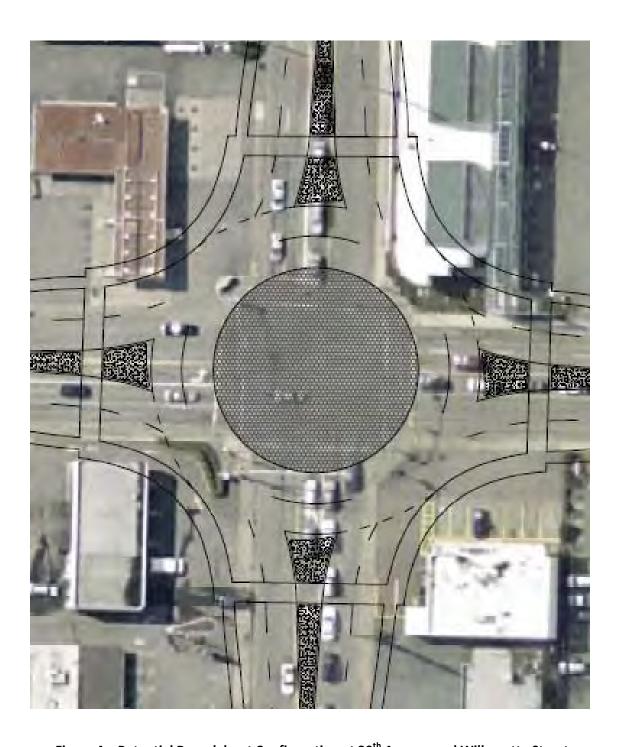


Figure 4 – Potential Roundabout Configuration at 29th Avenue and Willamette Street





Public and Private Approaches/Access Management Strategies

There are currently over 70 driveways on Willamette Street from 24th Avenue to 32nd Avenue. This creates numerous conflict points for motor vehicles, pedestrians and bicyclists. Reducing conflict points is likely to result in fewer accidents and increased capacity along the corridor. Managing access points along the corridor requires finding an appropriate balance between safety, mobility, and access. Consolidating driveway access points will be considered as part of each alternative, particularly where specific safety benefits would result.

The following strategies should be considered for the Willamette Street corridor:

- Consider removing and consolidating access points to existing businesses
- Consider shared accesses between adjacent property owners
- Implement turn lanes at driveways

Enhanced Bicycle Connections

Figure 5 illustrates potential bicycle facility improvements nearby, connecting to, and crossing Willamette Street. These improvements may be combined with any of the Willamette Street alternatives or considered independently. The improvements could improve bicycle access on local streets with a variety of bike boulevard treatments applied. Crossing improvements could be provided such as intersection priority areas (i.e., "Green Boxes") or rider-activated push-button signals for crossing at intersections with traffic signals.

To support development of the surrounding bicycle network as well as improving bicycle access to the corridor, two crossing improvements are proposed for the alternatives:

- Combined bike/turn lane on 24th Avenue: a bike lane would be striped with a dashed line within the inside portion of the existing right turn lane. Signage would be used to identify the combined lane and guide users toward the proper positioning. This would extend the existing bike lane on 24th Avenue (which currently drops away) and improve comfort for some riders who wish to travel through to the proposed Bike Boulevard on Portland Street. A local example of this configuration is located on 13th Avenue at Patterson Street. For Alternative 3 (which includes bike lanes on Willamette Street) a green bike box may be added to improve access for bicycle riders making a left turn from 24th Avenue to Willamette Street.
- Crosswalk with Pedestrian Hybrid Beacon at 29th Place: a Pedestrian Hybrid Beacon is a traffic control device that stops roadway traffic to allow pedestrians or bicycles to cross safely. The beacon is activated only when a pedestrian or bicyclist pushes the button to cross. By locating a safe crossing where the current northbound bike lane ends north of 30th Avenue (at the driveway/path connecting to 29th Place), safe access will be provided for southbound bicycle riders wishing to connect to Willamette Street from Oak Street, via 29th Place. The beacon would be most beneficial in Alternatives 1 and 5, where there are no continuous bike lanes on Willamette Street, but may also be considered as part of Alternative 3.





These improvements are illustrated in the plan view drawings included in the appendix.

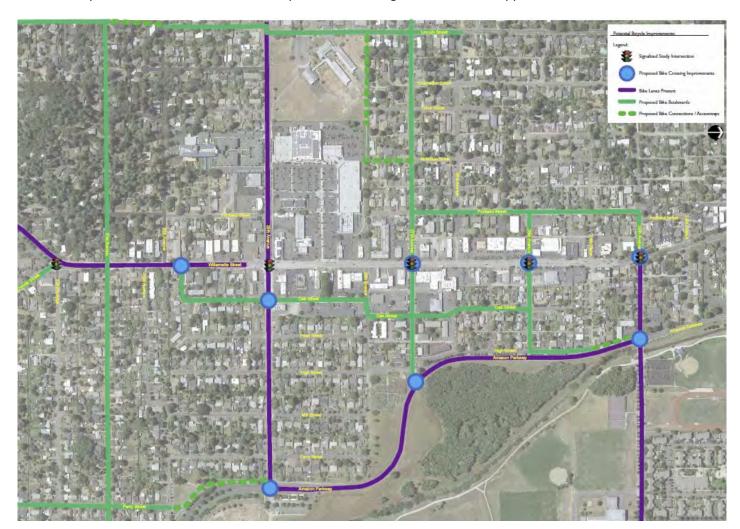


Figure 5 - Bicycle Facility Improvements

Enhanced Pedestrian Connections

The pedestrian environment on Willamette Street will be improved with wider sidewalks that are included in each alternative. To further enhance the pedestrian experience, crossing opportunities should be improved along Willamette Street. Currently the two largest distances between signalized crossings on the corridor are over 1400 feet (between 32nd Avenue to 29th Avenue) and over 900 feet (between 29th Avenue and 27th Avenue.) Two crossing improvements are proposed for the corridor:

• Traffic signal with crosswalks at Woodfield Station Driveway: a traffic signal at this location would provide a safe crossing for pedestrians between commercial areas and transit stops on both sides of the street. The intersection could be designed with a median pedestrian crossing refuges (i.e., island) on the north crosswalk in Alternatives 3 and 5, which include a center turn lane. The median refuge allows

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pedestrians to cross a roadway in stages, which is especially beneficial for users who require more time for crossings.

• Crosswalk with Pedestrian Hybrid Beacon at 29th Place: a Pedestrian Hybrid Beacon could be located south of 29th Avenue to provide a safe crossing for both pedestrians and bicycle riders. The signal would be most beneficial in Alternatives 1 and 5, where there are no continuous bike lanes on Willamette Street, but may also be considered as part of Alternative 3.

These improvements are illustrated in the plan view drawings included in the appendix.

Streetscape Elements

Travel lanes, sidewalks, bike lanes, intersection design and transit stops are fundamental facility design elements. Each has a function and must provide safety and comfort for the intended users. The configuration of these elements will play a part in the streetscape design of Willamette Street, as the perceptions of ease of travel and the sense of safety and comfort may change for different users with each alternative. The experiences of all users of the street as a place to be are affected by the facility design and streetscape. It is important that the facility design alternatives (1-3) and the potential elements of a unified streetscape design be considered together and not as separate evaluations.

Most of the right-of-way design elements that will be experienced and appreciated as a streetscape occur within the sidewalk corridor. The sidewalk corridor is defined by the roadway curbs and the back of sidewalks. When that corridor has been well-designed, it accommodates three primary functions, with design treatments to support those functions. Figure 6 illustrates conceptual sidewalk corridors and how the streetscape elements and the pedestrian experience may be affected.

Through Pedestrian Zone: Comfortable and unobstructed walking is the primary function of the sidewalk corridor. Draft federal guidelines developed by the Public Rights-of-Way Access and Advisory Committee (PROWAAC), require a minimum width of 4-feet and a preferred width of 5-feet. A useful urban design standard is the ability of two people to walk comfortably side-by-side, which typically requires at least 6-feet.

Furnishings Zone: Accommodates streetscape elements such as utility poles, street lights, planters, trees, benches, bike racks and bus shelters. It may also accommodate Low-Impact Development (LID) features such as flow-through stormwater planters. Pedestrian activities include transit boarding at designated stops, access to bike racks and access to on-street parking. The minimum desired width is 4-feet, with preferred widths of 5-feet to 7-feet.

Building Front Zone: For streets that support a significant amount of pedestrian-oriented retail, with buildings set close to sidewalks, an additional 1-foot to 2-feet is desirable to support storefront displays and window shopping.





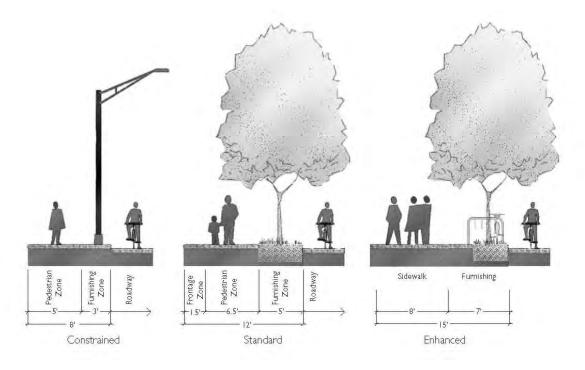


Figure 6 – Sidewalk Corridor Design

Developing a Design Theme for Willamette Street

Potential elements of a streetscape design theme for Willamette Street are described in the following section. Graphic representations of the potential elements are included in the appendix.

Unifying Streetscape Elements

Typical unifying elements of a streetscape are texture, color and form, along with other distinctive elements that create a unique functional or art-based character. Each of these elements can play an important role in the eventual transformation of Willamette into a signature street for the district.

Texture: Texture can be a unifying element by using a consistent palette of materials such as paving, walls, columns and railings. Opportunities for Willamette Street include sidewalk reconstruction and textured crosswalks at intersections, formalized mid-block pedestrian crossings or distinctive pavements for bike lanes.

Color: Color is a unifying element visually linked to texture. Colors can tie together places separated by distance and by function. Opportunities include any of the above elements that have special textures, as well as street furnishings such as bike racks, benches and bus shelters, and landscape materials with distinctive flowers or foliage colors.

Form: Form can provide both visual unity and visual distinction. Both unity and distinction have a place in a well-designed streetscape. Form also provides a sensed of orientation within the public realm and can provide visual





landmarks for the district. Opportunities include site furnishings, pedestrian-scale lighting, signage and bus shelters.

Additional Distinctive Elements — Green Street

Green Streets are primarily thought of as innovative facilities to treat and manage stormwater within the right-of-way. Those facilities create an ecological function for our streets, in addition to the traditional mobility and access functions. There are a number of Green Street facilities for stormwater. The selection of one or more facilities for Willamette Street will require detailed engineering analysis and consistency with existing City of Eugene stormwater standards. The choice of techniques will also be affected by the width of the sidewalk

corridor in a preferred alternative. Typical facilities include the following:

Flow-Through Planters: Flow-through stormwater planters are a common bioretention facility in urban areas. They provide a distinctive architectural feature for the sidewalks of an urban Green Street where sidewalk widths are 12 feet or greater, with a minimum 5-foot furnishing zone available. The design and location of planters should consider other sidewalk uses, such as outdoor seating storefront displays, as well as maintenance of adequate passenger loading/unloading space for on-street parking.



Figure 7 – Flow-Through Planters



Figure 8 - Basins

Basins: Because of their larger size, basins are usually located behind the sidewalk. They are an alternative to planters in the furnishing zone if the sidewalk width is too constrained to accommodate both the planter and a comfortable walking space for pedestrians. In those instances, the overall street right-of-way need may be greater, or a stormwater management easement required since the width of a basin is greater than a planter due to side slopes.





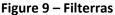
Filterras: Proprietary devices that treat stormwater through a physical process using amended soil and bioretention media combined with small street tree or a shrub. These devices can fit within the furnishing zone of a sidewalk corridor of 12-feet or greater in width.



Street Tree 2" Depth 50/50 7' CIP Concrete Sidewalk Compost/Bark 6"x12" 8" Aggregate Base Course Scupper 3'-7 Raingarden Trench Backfill Silva Cells Soil Mix 2'x4'x16" Subgrade Drain Rock 200 20 Joint Utility Trench 6" Drain Pipe to Street SD Sys 4" Aggregate Subbase Geotextile Fabric

Figure 10 - Sidewalk Silva Cells

Permeable Paving: Many of the impermeable surfaces within the sidewalk corridor could be constructed using permeable paving material such as landscape planting, permeable concrete or porous paving blocks. This requires well-draining native soil. The disadvantages of permeable paving include difficulties with maintenance and repair, higher cost, and limited infiltration effectiveness of streets with a gradient over five percent. Permeable pavement can be used in conjunction with other Green Street features and will help reduce the required size of these facilities by lessening the amount of runoff coming off the paved surface.



Sidewalk Silva Cells: This technique creates a sidewalk rain garden along the roadway and partially under the sidewalk. Rain falls directly on permeable pavers and planters. The silva cells extend the rain garden underneath the sidewalk and into a soil media that treats stormwater and nurtures the landscaping.



Figure 11 - Permeable Paving





It should be noted that Green Street principles are not limited to stormwater management. Other key elements of a Green Street are:

- Safe and appealing pedestrian environment
- Multimodal travel choices
- Maximizing opportunities for trees and landscaping
- Visual and physical connections to public spaces and open spaces
- Renewable energy for public signs and lighting

<u>Additional Distinctive Elements — Public Art</u>

Public art becomes another means for people to interact with each other and with the urban context. Creating a lively public realm with intrigues, challenges and inspires us as it becomes part of our larger goal of improving the quality if civic life. Within the unifying elements of streetscape, it is also another opportunity to explore texture, color and form. Implementing a public art program should include assessing the potential for city and regional funding support and coordination with local businesses. Examples of public art within or along a street right-of-way have been included in the appendix.

Streetscape Design Matrix

Figure 12 provides a summary matrix of how easily some of the typical amenities of a streetscape can be accommodated within the sidewalk corridors depicted in the alternatives. It is based on design principles described in the Streetscape Design Basics for Willamette Street Figure (included in the appendix) and the accompanying narrative.





Willamette Street Amenities Matrix

	Alterative 1	Alterative 3	Alterative 5
Bus Stop Amenities			
Enhanced Bus Shelters	\boxtimes	\otimes	
Sidewalk Character			
Wide Sidewalks (10' or greater)	\otimes	\otimes	
Paved furnishing zone			
Planter strip			
Outdoor seating/retail focus	\otimes	\boxtimes	
Textured Crosswalk			
Sidewalk Furnishings			
Bike Racks	\otimes	\otimes	
Benches	⊗	\otimes	
Trash receptacles			
Lighting			
Pedestrian scale (18' tall or shorter)			
Landscaping			
Deciduous tree canopy	\otimes	\otimes	
Street corner planting			
Landscaped tree wells			
Stormwater facilities	\otimes	\otimes	



Figure 12 – Amenities Matrix

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Utility Relocation

Relocating the utilities underground would improve the sidewalk environment by removing some barriers to pedestrian access and making the corridor more aesthetically pleasing. Utilities (poles, hydrants, pedestals, etc.) currently located along the sidewalks result in an inconsistent and obstructed pedestrian environment. Alternative 1 and Alternative 3 have the most constrained sidewalk conditions (approximately 9-feet width with reconstruction). Even minor adjustments of utility poles locations to be fully within the Furnishings Zone represents a significant cost, but would increase the Through Pedestrian Zone to minimum widths. Reconstruction of the sidewalk corridor to 13-feet in Alternative 5 would require relocation of all above-ground utilities to the new Furnishings Zone location created by moving the curb lines into the current roadway area. In this scenario, ample pedestrian circulation space would be available.

The planning-level cost estimate for utility relocation on Willamette Street between 24th Avenue and 32nd Avenue is \$2.6 Million⁶.

On-Street Parking

On-street parallel parking provides convenient access for adjacent businesses and a buffer between pedestrians and motor vehicles. On-street parking would likely have a very favorable benefit to the pedestrian environment, however, given the constrained right-of-way and community priorities, on-street parking is not considered in any of the three design alternatives. On-street parking may be reconsidered as part of long-term enhancements to the corridor.

Bus Stops

Two Lane Transit District (LTD) bus routes currently provide service along Willamette Street. Buses currently stop on the street and block the curbside travel lane during passenger boardings. Constructing bus pullouts would remove stopped vehicles from travel lanes, but would likely require right-of-way acquisition and would also require buses in the pullouts to merge back into the traffic stream. Figure 13 illustrates the dimensions of a potential bus pullout along Willamette Street. The traffic impacts of bus pullouts are further discussed in Technical Memorandum 8.

No bus pullouts are recommended for the corridor given the frequency of bus uses (five per hour south of 29th Avenue and two per hour north of 29th Avenue), right-of-way impacts, and increased delay for merging transit vehicles.

⁶ The cost estimate is based on 2013 dollars. The cost shown is a preliminary high-level estimate, subject to change. Estimate was received by email on June 11, 2013 from Mark Oberle, Eugene Water & Electric Board.





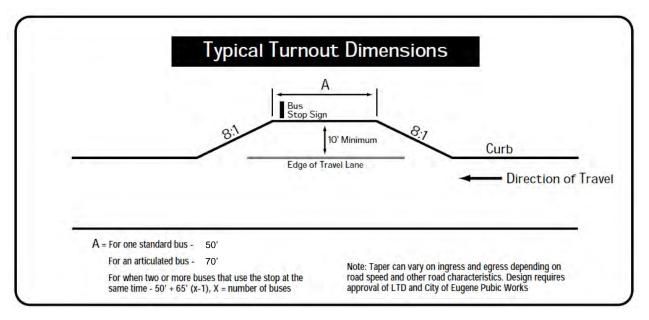


Figure 13 - Bus Pullout Illustration

Source: LTD Standards and Design

Improving bicycle and pedestrian access to transit stops would support transit usage along the corridor. If sidewalks are expanded there may be space available for improved bus stop amenities such as covered benches (shelters), real-time arrival information, or other transit stop amenities. No additional transit stop amenities are suggested for the corridor. Ridership should be monitored to identify potential future improvements as the Willamette Street corridor is redesigned and the surrounding land uses change over time.

Cost Estimates

Planning-level cost estimates were developed for each alternative, with the facility designs specified in this memorandum. The cost estimates are shown in Table 2. All costs shown are planning-level estimates in 2013 dollars and are subject to change. Details and assumptions for the cost estimates are shown in the appendix. The costs listed previously for utility relocation are not included in the estimates shown in Table 2.





Table 2: Planning-Level Cost Estimates (Million Dollars, in 2013 \$)

Alternative	Pavement Project	24 th to 29 th Ave	29 th to 32 nd Ave	Total
1	\$2.1	\$1.7	\$0.3	\$4.1
2	\$2.1	\$1.8	\$0.3	\$4.2
3	\$2.1	\$2.4	\$0.3	\$4.8

Pavement Project – City of Eugene project is planned to include paving, ADA accessibility, and stormwater improvements from 24th to 29th Avenue

Evaluation of Alternatives

The following section provides Tier 2 screening evaluation of the three proposed alternatives. The alternatives were scored using the evaluation criteria and methodology previously detailed in Technical Memorandum #1 (South Willamette Street Improvement Plan – Evaluation Criteria). Evaluation criteria were established to assess the potential of alternatives to best meet the transportation needs of the users of Willamette Street. The criteria are based on the goals and objectives in the Draft Eugene Transportation System Plan.

Individual criteria were scored as 1, 0, or -1 representing improvement, no change, or degradation, respectively. The scoring weighs all criteria equally, with the total evaluation score representing the sum of the individual criteria scores. Figure 14 documents the results of the scoring evaluation which rate Alternative 3 as the highest scoring and Alternative 1 the lowest. An explanation for the criteria and scoring for each alternative are further detailed in the appendix.

^{24&}lt;sup>th</sup> to 29th Avenue – Additional costs vary by alternative 29th to 32nd Avenue – Additional costs same for all alternatives

^{*}All costs are planning-level estimates subject to change



	Ac	cess &	k Mobil	ity	Safe	ety & Ho	ealth		cial uity	Econ	omic B	enefit	Cost	Effectiv	eness	C	limate	& Ener	gy	Ecolo	ogical ction		nunity	
Alternative	Reliability (For All Modes)	Neighborhood Connectivity	Motor Vehicle Travel Time	Active Mode Travel Time	Safety	Security	Emergency Response	Equity	Economic Access	Freight Mobility	Walkable/Bikeable Business District	Business Vitality	Fundability	Asset Management	Project Benefits	Reduce Vehicle Miles Traveled	Pedestrian Facilities	Bicycle Facilities	Transit Facilities	Storm water Design	Landscape Design	Community Vision and Land Use	Transportation Planning Compatibility	TOTAL
Alternative 1 - 4-Lane	0	0	0	0	0	0	0	0	0	0	0	0	1	1	1	0	0	0	0	0	0	0	0	3
Alternative 3 - 3-Lane with Bike Lanes	0	1	-1	1	1	1	-1	1	1	-1	1	0	0	1	1	0	0	1	0	0	0	0	0	7
Alternative 5 - 3-Lane with Wide Sidewalks	0	0	-1	0	1	1	-1	1	1	-1	1	0	-1	1	1	0	1	0	1	0	0	1	0	6

FIGURE 14 – Evaluation Criteria Scoring

S. Willamette Street Improvements - Cost Estimates Cost Estimates for Alternative Concepts

Alternative 1 (24th to 29th Avenue)					
4-Lane, 9' Sidewalk	Qty	Unit	Unit Cost	Total Cost	Notes
Demolition and Clearing	1	LS	\$32,329	\$32,329	3% of Total Construction Cost
Mobilization and Erosion Control	1	LS	\$129,317	\$129,317	12% of Total Construction Cost
Construction Survey	1	LS	\$21,553	\$21,553	2% of Total Construction Cost
Temporary Traffic Control	1	LS	\$32,329	\$32,329	3% of Total Construction Cost
Concrete Sidewalks, 9' width	33,250	SF	\$8	\$266,000	Includes agg base
Concrete Driveways, 9' width	12,100	SF	\$12	\$145,200	Includes agg base
Driveway transitions	2,689	SF	\$5	\$13,444	Includes agg base, 2' beyond driveway
Street Trees	80	EA	\$600	\$48,000	2.5" caliper, includes root barrier, irrig
Street Lights Ornamental	40	EA	\$8,000	\$320,000	
Drainage and Utility Adjustment Allowance	1	LS	\$10,000	\$10,000	Assumes use of existing system, Adj sidewalk utility boxes, Rel bus stops
Traffic Signal Modifications (ADA Ped Poles)	3	EA	\$25,000	\$75,000	Assumes Three Signal Modifications for new ADA pedestrian push buttons
Traffic Signals	1	EA	\$200,000	\$200,000	Assumes Woodfield Station Signal

Total: \$1,293,173

Construction Contingency 30%:
Alt 1 Total: \$387,952 **\$1,681,125**

Alternative 3 (24th to 29th Avenue)					
3-Lane with Bike Lanes, 9' Sidewalks	Qty	Unit	Unit Cost	Total Cost	Notes
Demolition and Clearing	1	LS	\$35,119	\$35,119	3% of Total Construction Cost
Mobilization and Erosion Control	1	LS	\$140,477	\$140,477	12% of Total Construction Cost
Construction Survey	1	LS	\$23,413	\$23,413	2% of Total Construction Cost
Temporary Traffic Control	1	LS	\$35,119	\$35,119	3% of Total Construction Cost
Concrete Sidewalks, 9' width	33,250	SF	\$8	\$266,000	Includes agg base
Concrete Driveways, 9' width	12,100	SF	\$12	\$145,200	Includes agg base
Driveway transitions	2,689	SF	\$5	\$13,444	Includes agg base, 2' beyond driveway
Street Trees	80	EA	\$600	\$48,000	2.5" caliper, includes root barrier, irrig
Street Lights Ornamental	40	EA	\$8,000	\$320,000	
Drainage and Utility Adjustment Allowance	1	LS	\$10,000	\$10,000	Assumes use of existing system, Adj sidewalk utility boxes, Rel bus stops
Traffic Signal Modifications (ADA Ped Poles)	3	EA	\$50,000	\$150,000	Assumes Three Signal Modifications (left turn phasing) for new ADA pedestrian push buttons
Traffic Signals	1	EA	\$200,000	\$200,000	Assumes Woodfield Station Signal
Mid-block Pedestrian Crossing	450	SF	\$40	\$18,000	45' long by 10' wide, concrete in median

\$1,404,773 Total:

Construction Contingency 30%:

Alt 3 Total: \$421,432

\$1,826,205

Alternative 5 (24th to 29th Avenue)					
-Lane, 13' Sidewalks, Storm water planters	Qty	Unit	Unit Cost	Total Cost	Notes
Demolition and Clearing	1	LS	\$59,978	\$59,978	4% of Total Construction Cost
Mobilization and Erosion Control	1	LS	\$179,933	\$179,933	12% of Total Construction Cost
Construction Survey	1	LS	\$37,486	\$37,486	2.5% of Total Construction Cost
Temporary Traffic Control	1	LS	\$74,972	\$74,972	5% of Total Construction Cost
Concrete Sidewalks, 13' width	45,000	SF	\$8	\$360,000	Includes agg base
Concrete Driveways, 13' width	18,250	SF	\$12	\$219,000	Includes agg base
Driveway Transitions	2,808	SF	\$5	\$14,038	Includes agg base, 2' beyond driveway
Street Trees	80	EA	\$600	\$48,000	2.5" caliper, includes root barrier, irrig
Stormwater Treatment Planters	4,680	SF	\$30	\$140,400	Assume 5' width with curbs, treat 6% of half street plus sidewalk
Street Lights Ornamental	40	EA	\$8,000	\$320,000	

Drainage and Utility Adjustment Allowance	1	LS	\$30,000	\$30,000	Assumes recon of storm inlets only, Adj sidewalk utility boxes, Rel bus stops		
Traffic Signal Modifications (ADA Ped Poles)	3	EA	\$50,000	\$150,000 Assumes Three Signal Modifications (left turn phasing) for new ADA pedestrian push buttons			
Traffic Signals	1	EA	\$200,000	\$200,000	Assumes Woodfield Station Signal		
Mid-block Pedestrian Crossing	450	SF	\$40	\$18,000	45' long by 10' wide, concrete in median		

\$1,851,807 Total:

Construction Contingency 30%:
Alt 5 Total:

\$555,542 **\$2,407,348**

29th to 32nd Avenue

	Qty	Unit	Unit Cost	Total Cost	Notes
Demolition and Clearing	1	LS	\$4,800	\$4,800	3% of Total Construction Cost
Mobilization and Erosion Control	1	LS	\$19,200	\$19,200	12% of Total Construction Cost
Construction Survey	1	LS	\$3,200	\$3,200	2% of Total Construction Cost
Temporary Traffic Control	1	LS	\$4,800	\$4,800	3% of Total Construction Cost
Signing and Striping	1	LS	\$10,000	\$10,000	Estimated from ODOT values
HAWK Signal	1	EA	\$150,000	\$150,000	Includes cross walk striping

Total:

Construction Contingency 30%: 29th to 32nd Total

\$192,000 \$57,600 **\$249,600**

Assumptions

Includes removal of structures, obstructions,	tree removal					
Includes ERC materials, plan, and patrol						
Includes signage, barrels, barricades, and pa	itrol					
Excavation includes surfacing removal, rock/s	soil base excavation					
Pavement is all PCC and not being reconstru	cted or repaired					
Includes aggregate, Demolition and replacement due to poor condition						
Sidewalk cost includes aggregate						
Pavement is all PCC, pavers installed on op	of PCC Recon					
Includes root barrier and irrigation						
Includes curbing, soil media, plantings, edge	treatment					
Includes pole, lamp, ballast, conduit						
Includes lane striping, parking stalls/ticks, sig	nage					
Estimates new pipe connections, structures						
	Includes signage, barrels, barricades, and pa Excavation includes surfacing removal, rock/ Pavement is all PCC and not being reconstru Includes aggregate, Demolition and replacen Sidewalk cost includes aggregate Pavement is all PCC, pavers installed on op Includes root barrier and irrigation Includes curbing, soil media, plantings, edge Includes pole, lamp, ballast, conduit Includes lane striping, parking stalls/ticks, sig					



South Willamette Street Improvement Plan – Tier 2 Screening Evaluation Notes

P10086-012

This document describes the evaluation criteria and scoring approach used to analyze the alternatives developed for the South Willamette Street Improvement Plan. A point-based rating methodology is used to evaluate how well proposed design alternatives meet measure of effectiveness criteria. In this way, a consistent method is used to evaluate and rank the alternatives based on how well they meet identified goals and objectives. The ranking informs the Technical Advisory Committee, stakeholders and appointed and elected officials; however, the final recommended alternative will be based on feedback and direction from these parties.

The goals and objectives in the Draft TSP provided a basis for the development of the evaluation criteria, which are intended to assess each alternative's potential to meet the needs of the people using Willamette Street. The evaluation criteria were refined based on a review of planning documents more specific to the study area, such as the South Willamette Concept Plan. The criteria are summarized in Table 1 according to the broad goal category they support.

The primary intent of the scoring is to provide a relative comparison between alternatives. Criteria scoring could differ for different segments within the corridor (i.e., north of 29th Avenue compared to south of 29th Avenue) due to differences in existing configurations and surrounding land uses. For clarity in comparing scoring across all alternatives, the scoring was generally focused on changes relative to the existing configuration and primarily on the north segment of the corridor. The north corridor makes up the longest segment in the corridor, has the most relative change between alternatives, and may be considered the most critical segment due to the surrounding commercial land uses.

The overall evaluation approach assumes an equal weight for each criterion. The evaluation scores for all criteria are summed to determine the overall evaluation score. This method allows a goal category with more supporting criteria to have a larger influence on the overall score.

Table 1 summarizes the criteria, describes the evaluation score and notes how the scoring was applied to the three alternatives considered in the Tier 2 screening.



Table 1: Evaluation Criteria and Scoring

Criteria	Evaluation Score	Scoring Notes
Access and Mobility		
Reliability (For All Modes) Improves trip reliability, consistency, comfort and convenience for all modes (walk, bike, transit, cars).	+1. Improves trip reliability 0. No change -1. Reduces trip reliability	Each alternative represents tradeoffs within limited space. No alternative improves or degrades all modes.
Neighborhood Connectivity Increases the number of households that can safely walk, bike, or use transit services to meet basic (non-work) daily needs.	+1. Increases # of connected households 0. No change -1. Decreases # of connected households	Alternatives that provide bike lanes were considered to represent improvement. Sidewalk width was not considered to significantly affect number of connected households.
Motor Vehicle Travel Time Reduces travel time between key origins and destinations for motor vehicles.	 +1. Decreases travel time for motor vehicles 0. No change -1. Increases travel time for motor vehicles 	Alternatives that reduced motor vehicle lanes where scored '-1' due to reduced through-capacity.
Active Mode Travel Time Reduces travel time between key origins and destinations for active modes (pedestrian and bicycle).	 +1. Decreases travel time for alternative modes 0. No change -1. Increases travel time for alternative modes 	Alternatives that provide bike lanes were considered to represent improvement. Sidewalk width was not considered to significantly affect travel time.
Safety and Health		
Safety Improve safety and security for all users, especially for the most vulnerable; strive for zero fatalities.	+1. Improves safety for all modes 0. No change -1. Reduces safety for all modes	Alternatives with 3 vehicle lanes are expected to improve safety by lowering vehicle speeds and reducing some types of collisions. Bike lanes provide a dedicated facility and a buffer for pedestrians.
Security Improve actual and perceived sense of security (i.e. Safe driving, getting to and riding transit, walking and biking).	+1. Improves sense of security 0. No change -1. Decreases sense of security	Wider sidewalks and bike lanes provide a buffer between motor vehicle lanes, improving sense of security for users.

Table Continued on next page.



(Continued) Table 1: Evaluation Criteria and Scoring

Criteria		Evaluation Score	Scoring Notes
Emergency Response Improves or maintains emergency response times within and through the corridor.	+1.	Improves emergency response times	Alternatives with less than 4 motor vehicle lanes where scored '-1' due to narrower space for vehicle lanes and greater likelihood of vehicle blockage/congestion.
	0.	No change	
	-1.	Reduces emergency response times	
Social Equity			
Equity Contributes to closing the transportation access gap between the general user and populations with limited choices, such as the elderly, low income, minority populations, and people with disabilities.	+1	Specifically benefits populations with limited choices	Alternatives that provide dedicated bike lanes or sidewalks beyond 10' are considered to benefit users with limited choices.
	0.	No Change	
	-1.	Negatively impacts populations with limited choices	
Economic Access Improves access from residences to employment and neighborhood centers within a 20-minute walk, bike, or transit trip.	+1.	Improves employment access	Alternatives that provide
	0.	No change	dedicated bike lanes or sidewalks beyond 10' are
	-1.	Decreases employment access	considered to improve access within 20-minute walk, bike, or transit trip.
Economic Benefit			
Freight Mobility Provides safe, efficient, and continuous motor vehicle operation to allow timely freight movement along Willamette Street.	+1.	Improves corridor's freight movement	Alternatives with less than 4 motor vehicle lanes where scored '-1' due to reduced capacity.
	0.	No Change	
	-1.	Negative impact on freight movement	
Walkable/Bikeable Business District Promotes a "Safe, Attractive Pedestrian Experience for Business, Shopping and Entertainment."	+1.	Improves business district pedestrian and bicycle experience	Alternatives that provide dedicated bike lanes or wider sidewalks are considered to improve experience for pedestrians and people on bikes.
	0.	No change	
	-1.	Reduces business district pedestrian and bicycle experience	
Business Vitality	+1.	Supports economic vitality	Adding a center turn lane is a benefit to business traffic. Reducing lanes is considered negative. Doing both is neutral.
Supports access and visibility of businesses that rely on drive-by traffic by balancing congestion with economic vitality	0.	No change	
	-1.	Negative impact on economic vitality	



Table Continued on next page.

(Continued) Table 1: Evaluation Criteria and Scoring

Criteria	Evaluation Score	Scoring Notes
Cost Effectiveness		
Fundability Available funding sources exist to implement projects in a timely fashion.	+1. Funding sources are available	Funding for maintaining existing alignment (Alt. 1) and curb-to-curb width is considered feasible. Moving curbs (Alt. 5) is considered high cost.
	Feasible costs, but no identified funding	
	-1. High costs and no funding expected	
Asset Management Favors the enhancement and maintenance of existing systems over system expansion.	+1. Enhances existing transportation system	All alternatives are considered enhancements of the existing transportation system.
	Minimal enhancement or expansion	
	-1. Expands transportation system	
Project Benefits Optimizes benefits relative to public, private and social costs over the life-cycle of the project	+1. Provides maximum benefits	Each alternative provides benefits, but reflects tradeoffs necessary within limited space.
	0. Minimal benefits	
	-1. Provides no benefits	
Climate and Energy		
Reduce Vehicle Miles Traveled (VMT) Improves the corridor as an attractive area without having to drive. Increases mode share for walk, bike, and transit thus reducing greenhouse gases and fossil fuel consumption.	+1. Reduces VMT	VMT impacts are unclear.
	0. No change	Bike lanes may reduce VMT. Fewer lanes may
	-1. Increases VMT	result in out-of-direction travel.
Pedestrian Facilities Adds sidewalks and crosswalks that fill in system gaps, improve system connectivity, removes obstructions and are accessible to all users.	+1. Improves pedestrian facilities	Pedestrian improvements
	0. No change	are expected to be included in each scenario
	-1. Negative impact on pedestrian facilities	due to widening of sidewalks and other design treatments. Therefore, criterion is applied relative to change from "baseline" enhanced 9' sidewalk.



(Continued) Table 1: Evaluation Criteria and Scoring

Criteria	Evaluation Score	Scoring Notes
Bicycle Facilities Adds bikeways that fill in system gaps, improve system connectivity, and are accessible to all users.	 +1. Improves bicycle facilities, including bike lanes 0. No change -1. Negative impact on bicycle facilities 	Alternatives that provide dedicated bike lanes are considered '+1', otherwise '0'.
Transit Facilities Improves transit facilities and accessibility to transit stops (for all users) along and near the corridor.	+1. Improves transit facilities 0. No change -1. Negative impact on transit facilities	Wider sidewalk (Alt 5) is only alternative considered to have a significant benefit to transit stop accessibility
Ecological Function		
Stormwater Design Transportation improvements lower the rate of storm water runoff and improve water quality.	+1. Minimizes storm water runoff 0. No change -1. Increases storm water runoff	No storm water runoff impacts have been identified.
Landscape Design Reduces the urban heat island through landscape design, less pavement, and increased tree canopy.	+1. Reduces heat island 0. No change -1. Increases heat island	No landscape design impacts have been identified.
Community Context		
Community Vision and Land Use Supports implementation of Envision Eugene land use and growth management goals and A Community Climate and Energy Action Plan for Eugene.	+1. Supports Envision Eugene 0. No change -1. Conflicts with Envision Eugene	Criterion is applied based on sidewalk width (relative to "baseline" enhanced 9' sidewalk).
Transportation Planning Compatibility Compatible with City's transportation plans (TSP, Long Range Transit Plan, and Pedestrian and Bicycle Master Plan [PBMP])	 +1. Compatible with City transportation plans 0. Has little or no impact (or has offset impacts) -1. Not compatible with City transportation plans 	Each alternative represents tradeoffs within limited space. No alternative is considered to significantly differ in overall compatibility.

Alternative 1

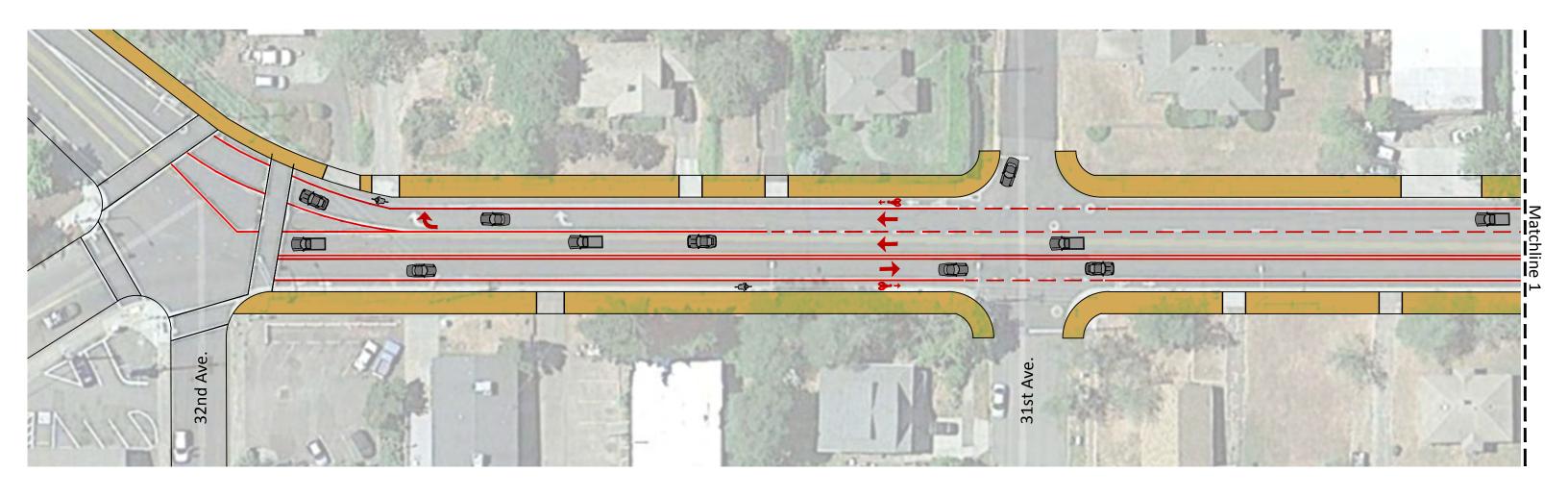


Alternative 3



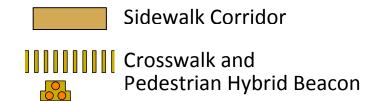
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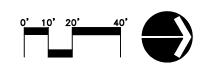




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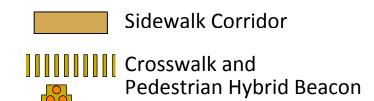




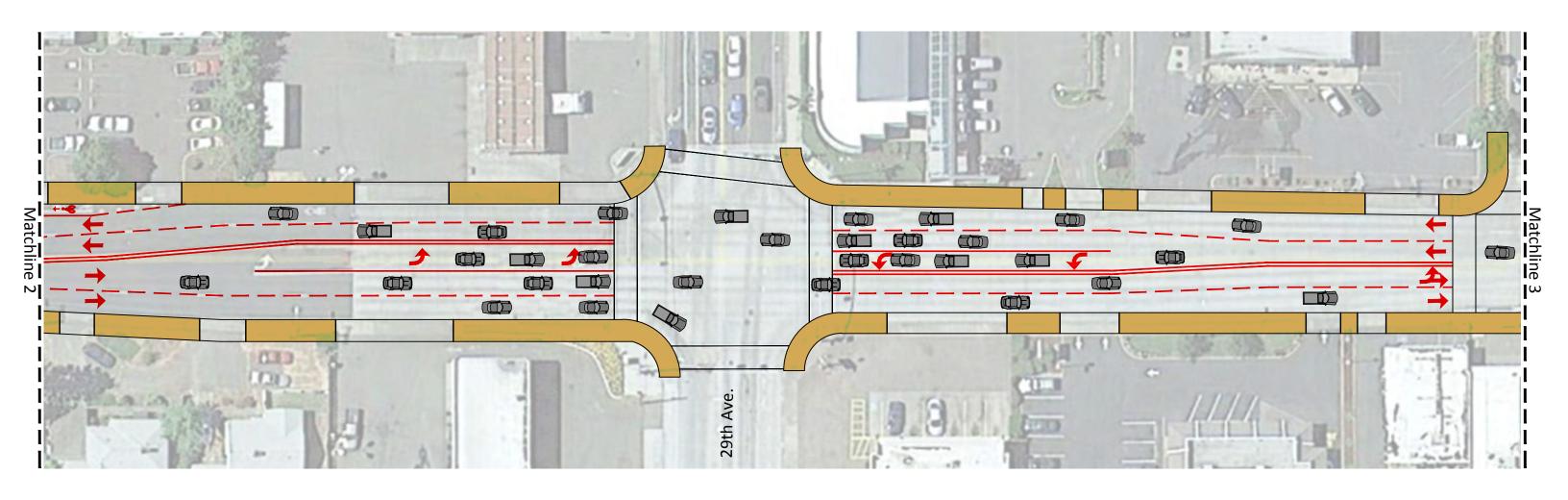




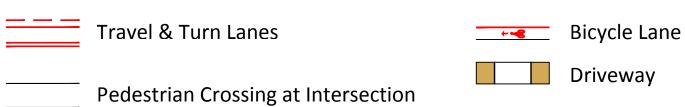


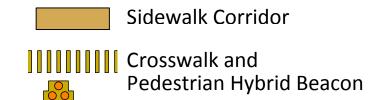


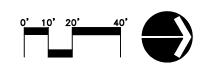


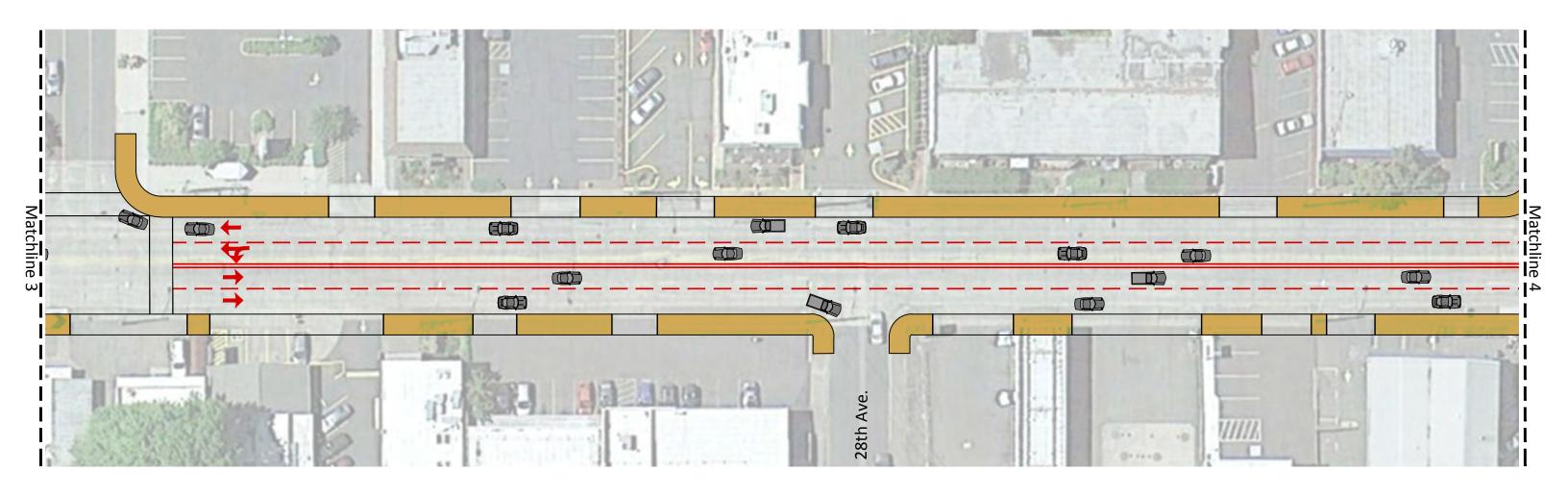


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Travel & Turn Lanes

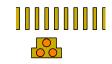
Pedestrian Crossing at Intersection

Bicycle Lane

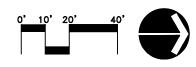


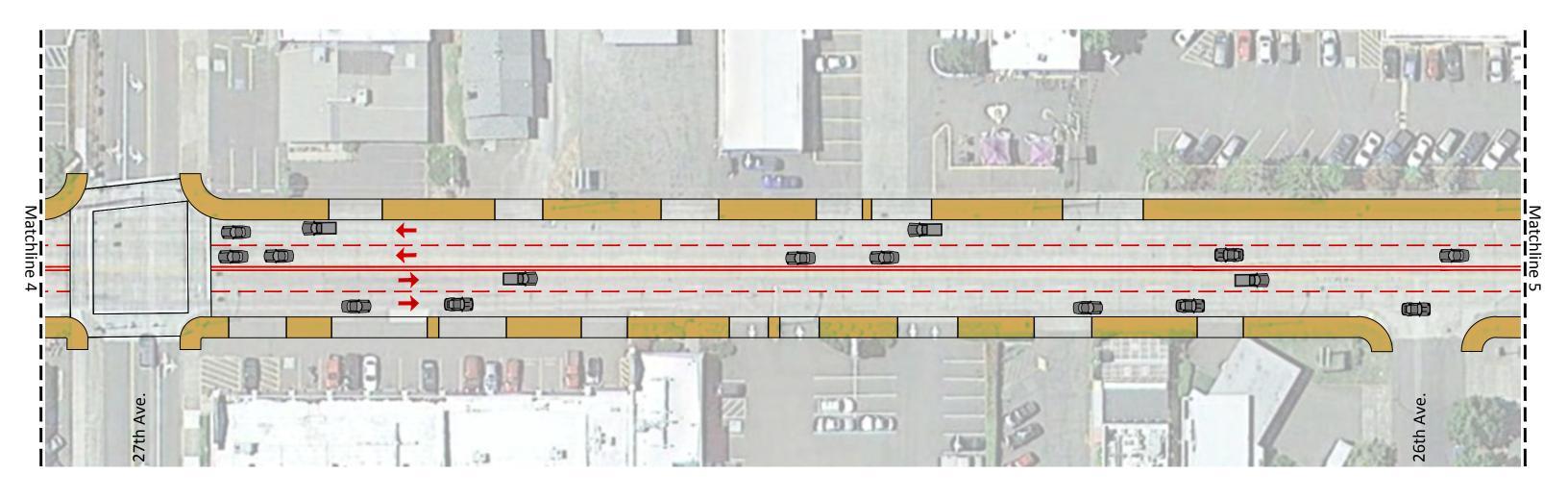


Sidewalk Corridor



Crosswalk and Pedestrian Hybrid Beacon



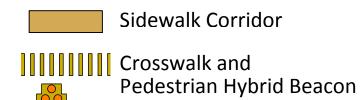


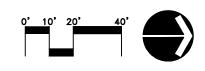
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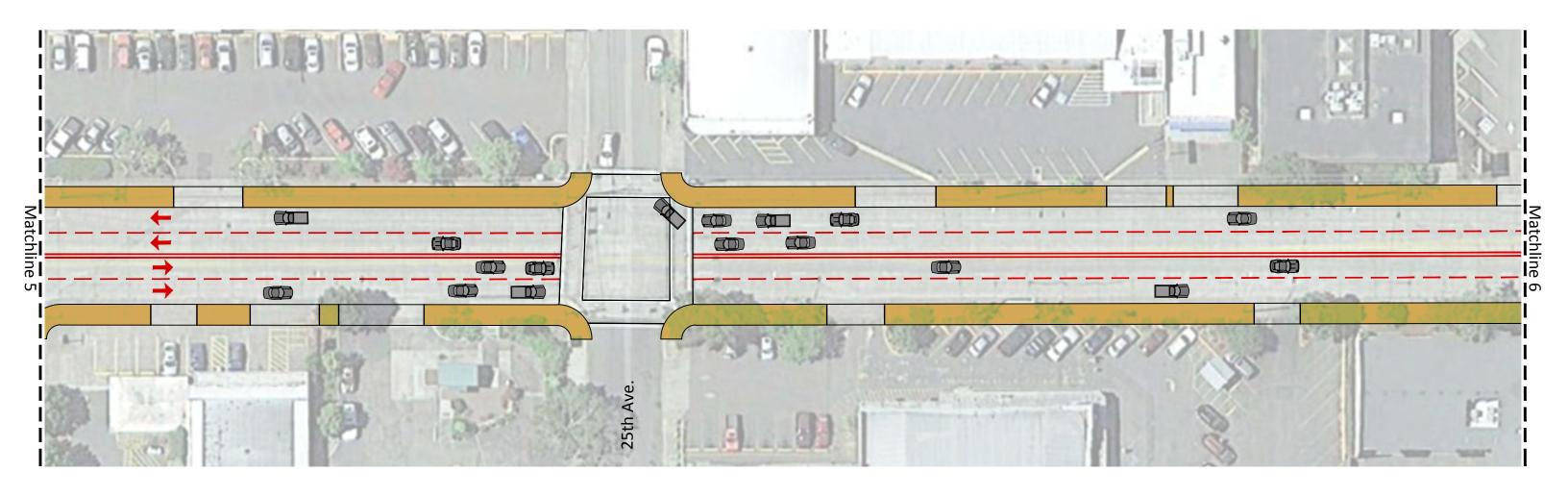
Travel & Turn Lanes

Driveway

Pedestrian Crossing at Intersection

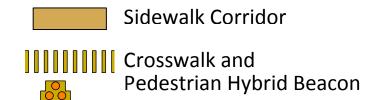


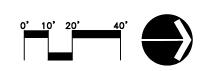


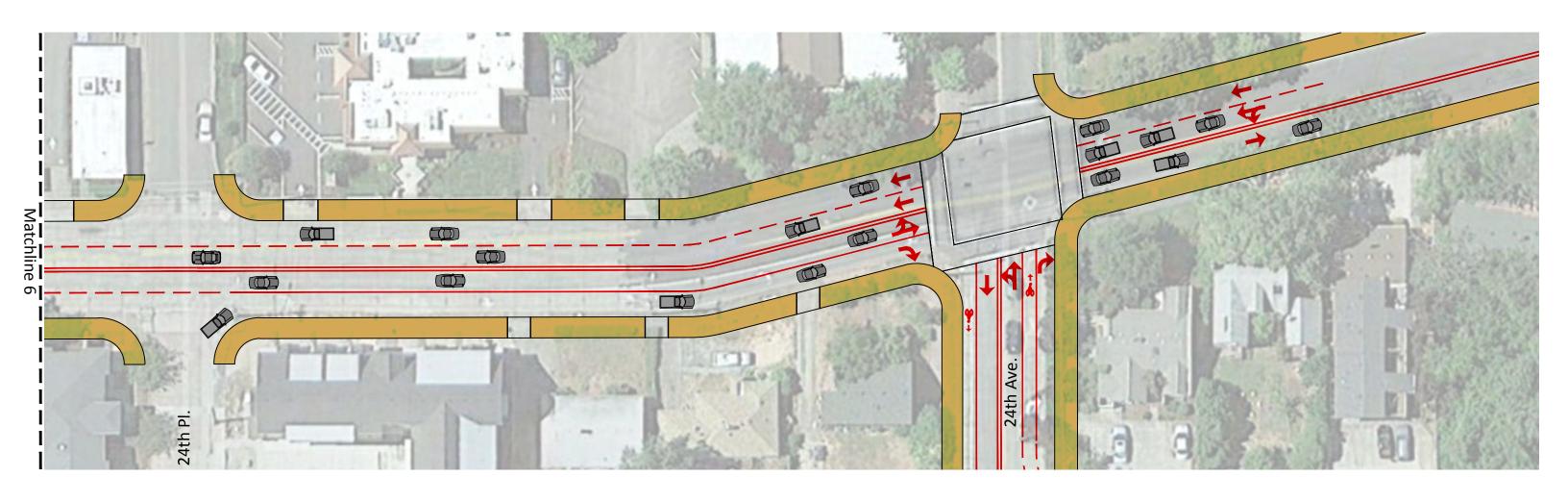


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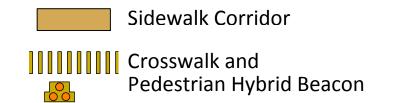
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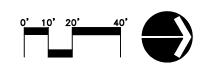
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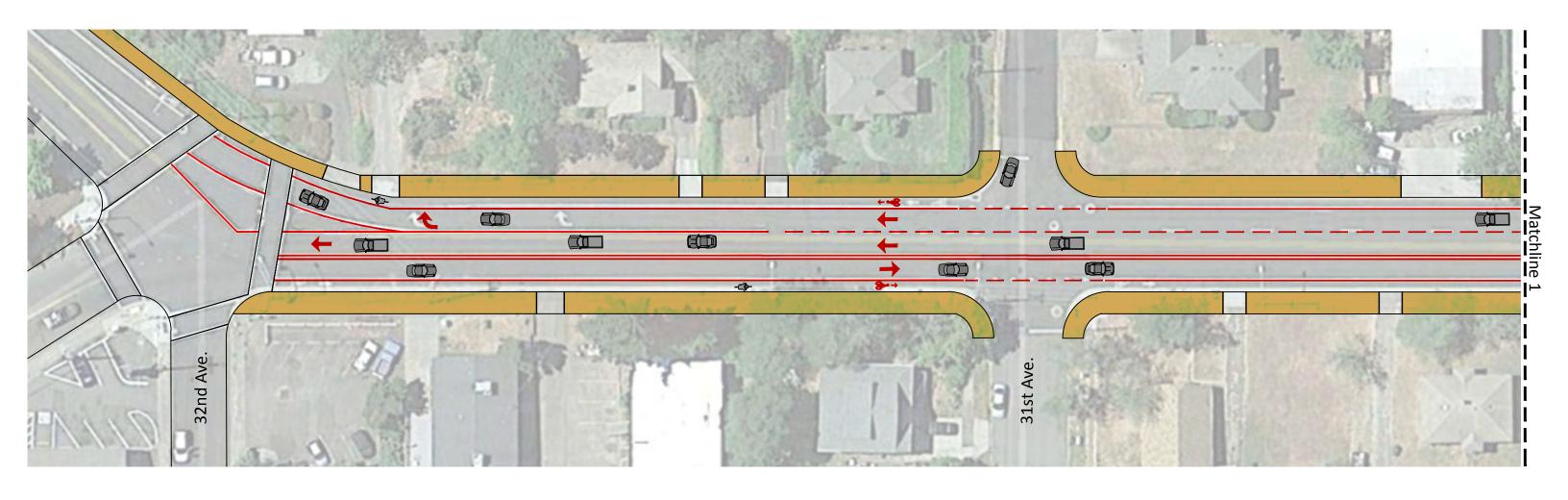
Bicycle Lane

Pedestrian Crossing at Intersection

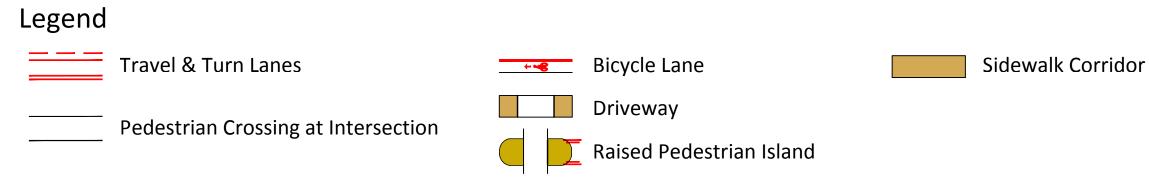
Driveway

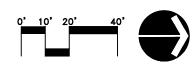






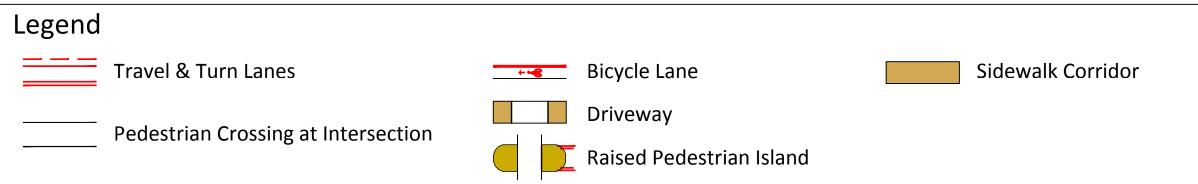
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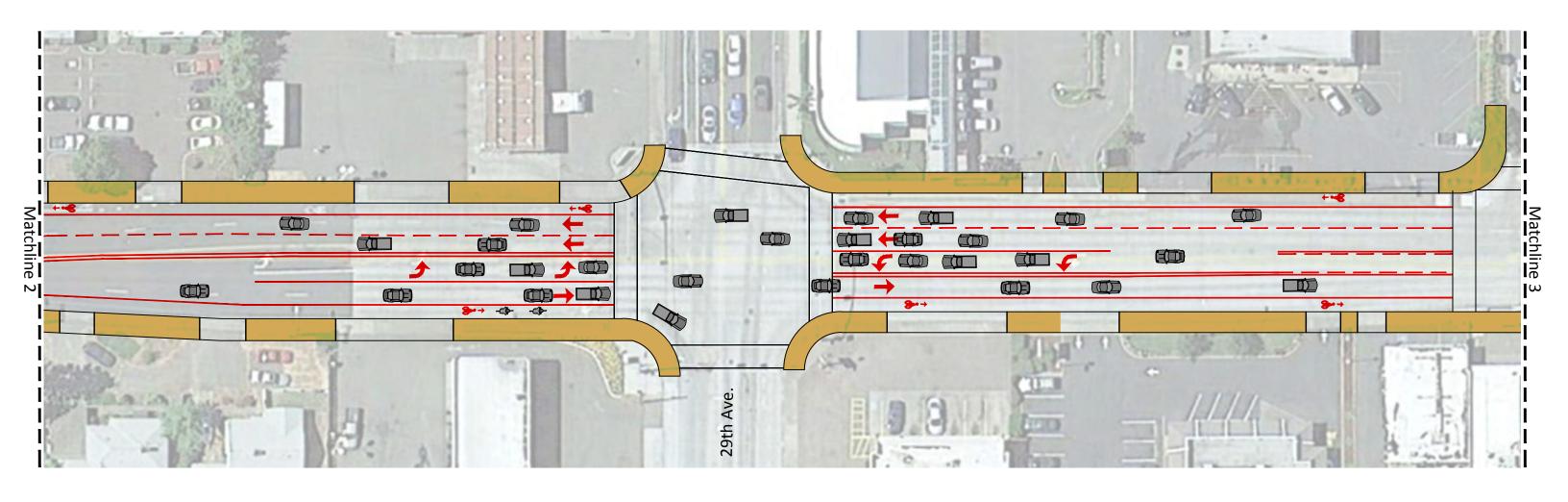




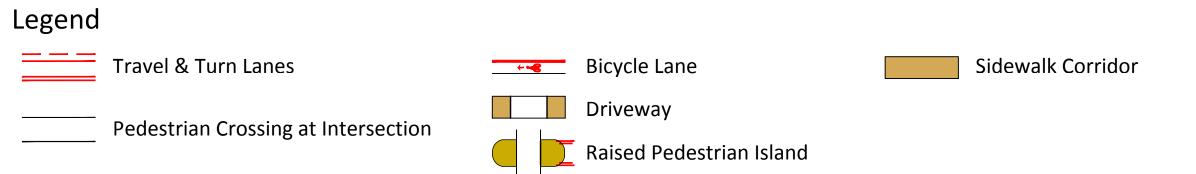


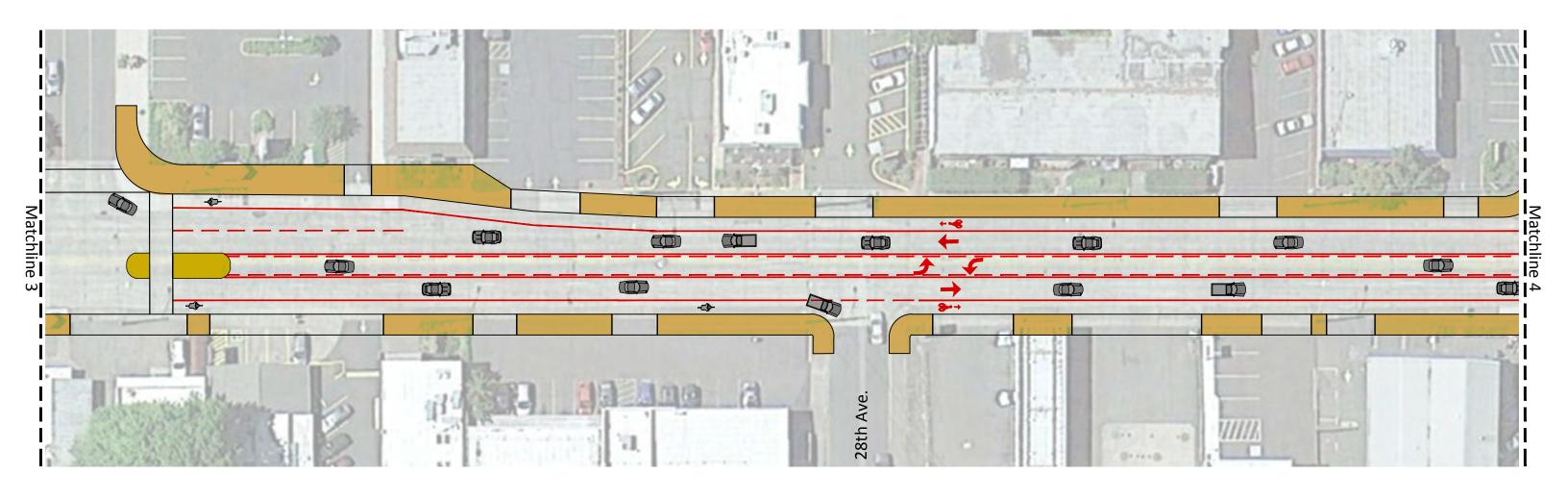
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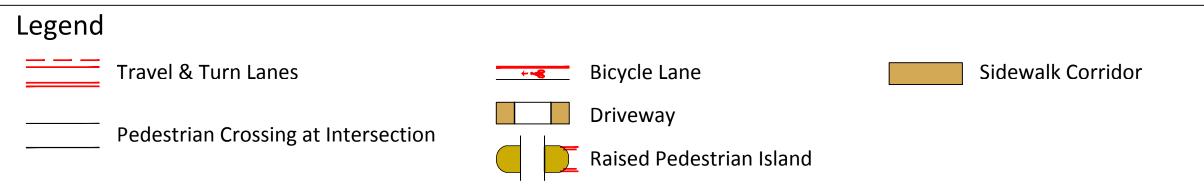




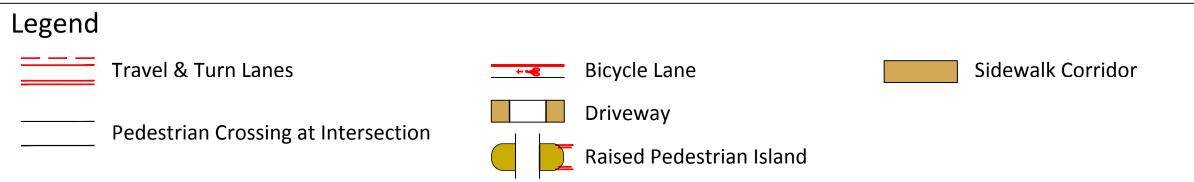
Alternative 3: 3-Lane with Bike Lanes

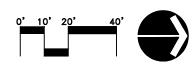


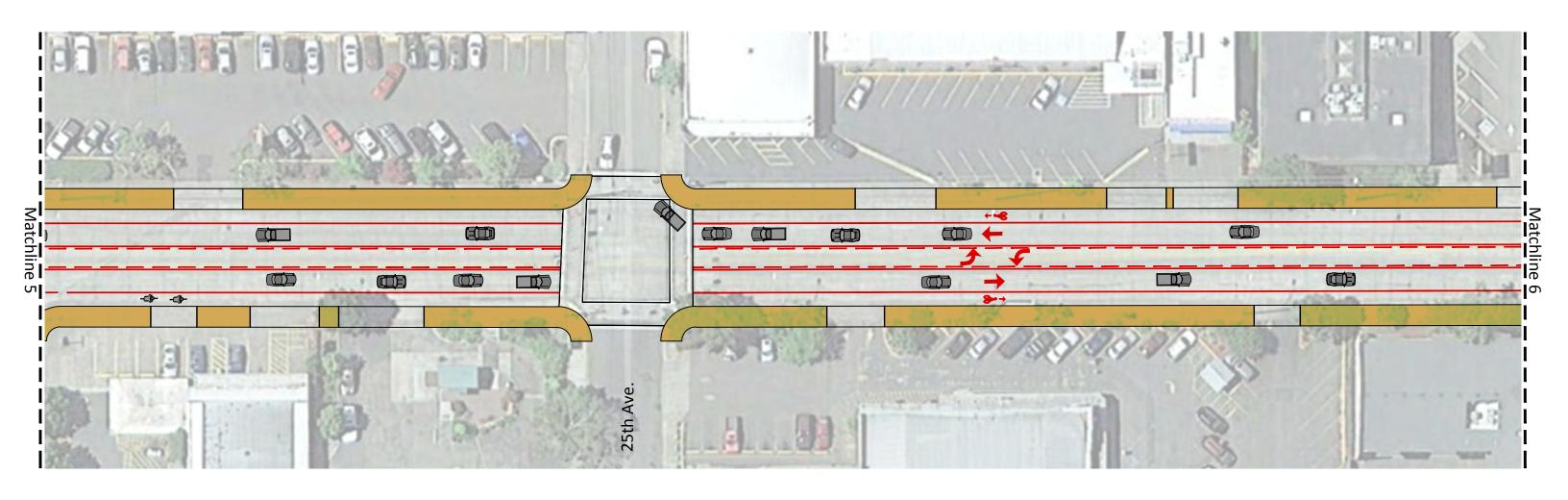




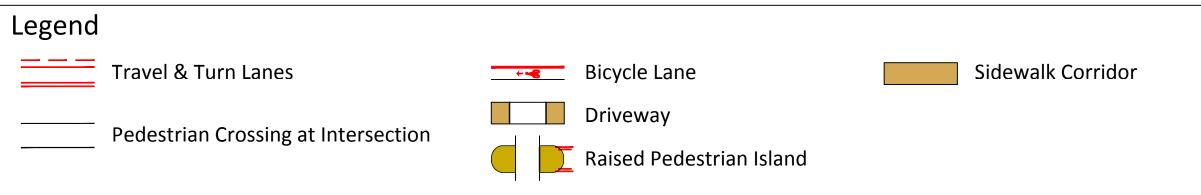


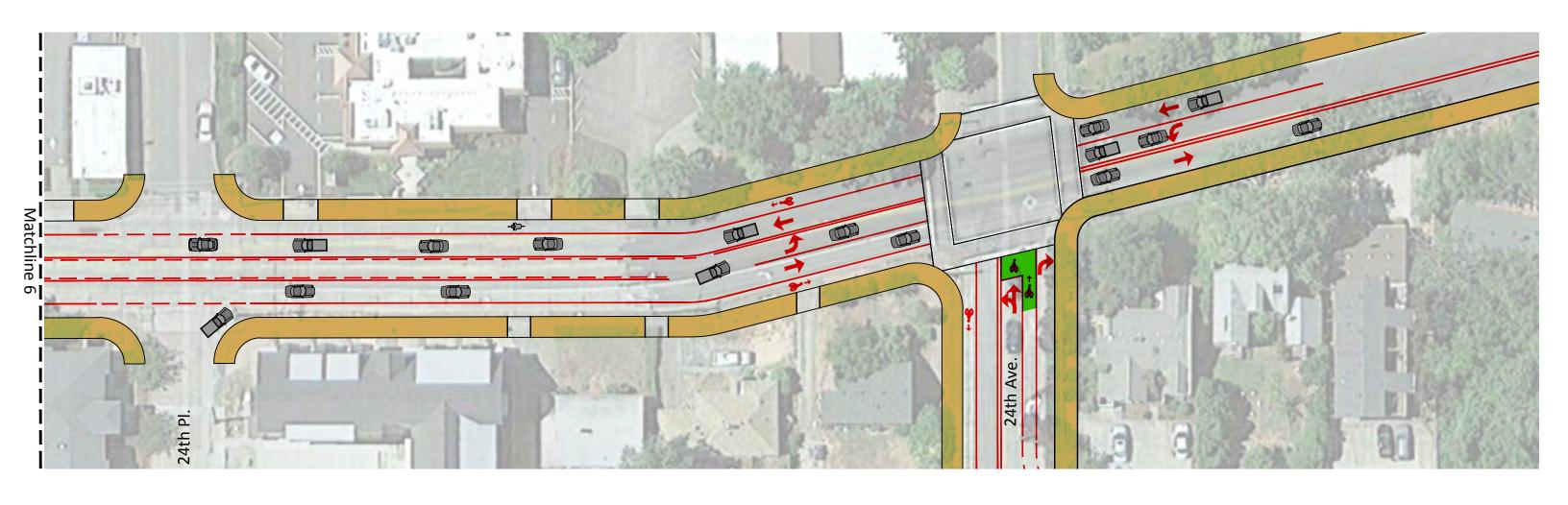


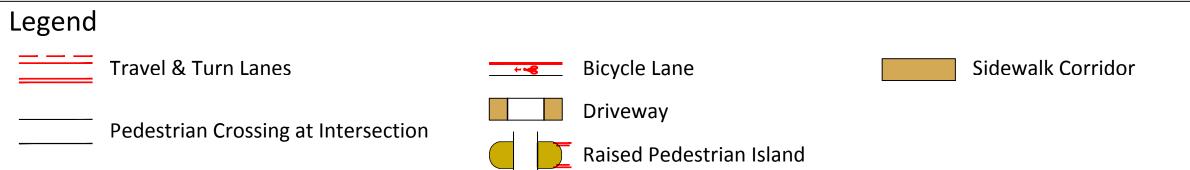


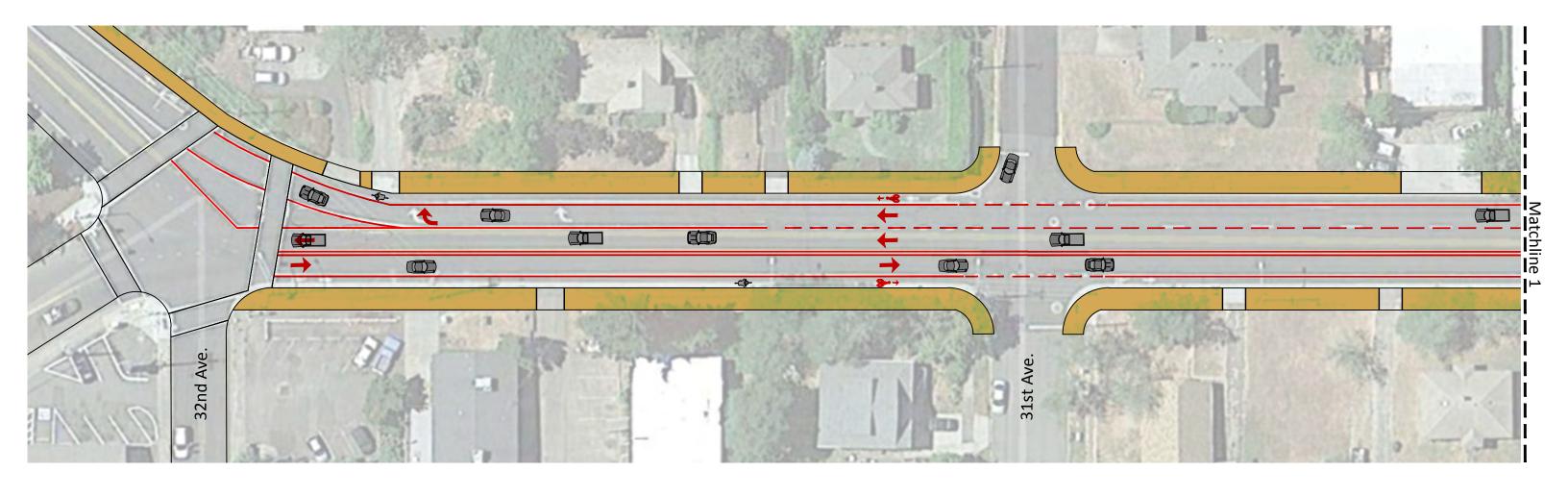


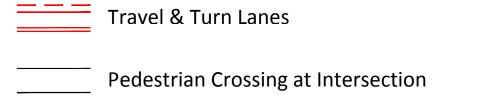
Sheet 6 of 7

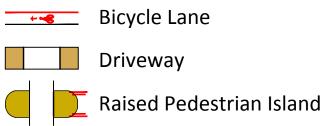


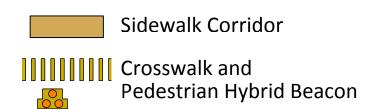


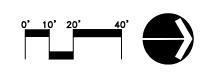




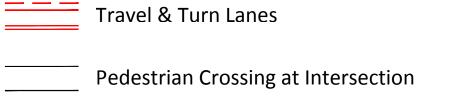


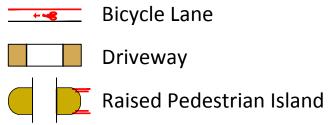


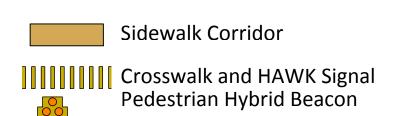


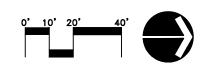


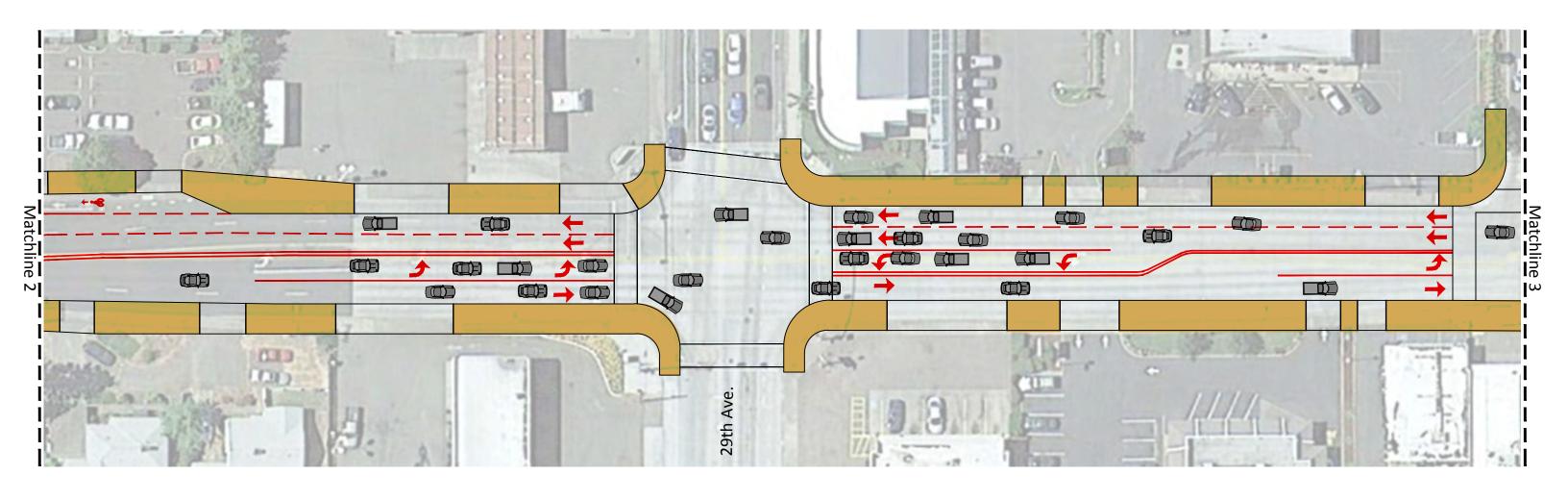


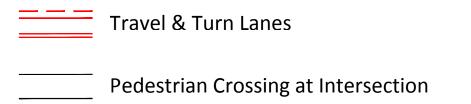




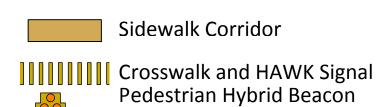


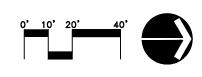


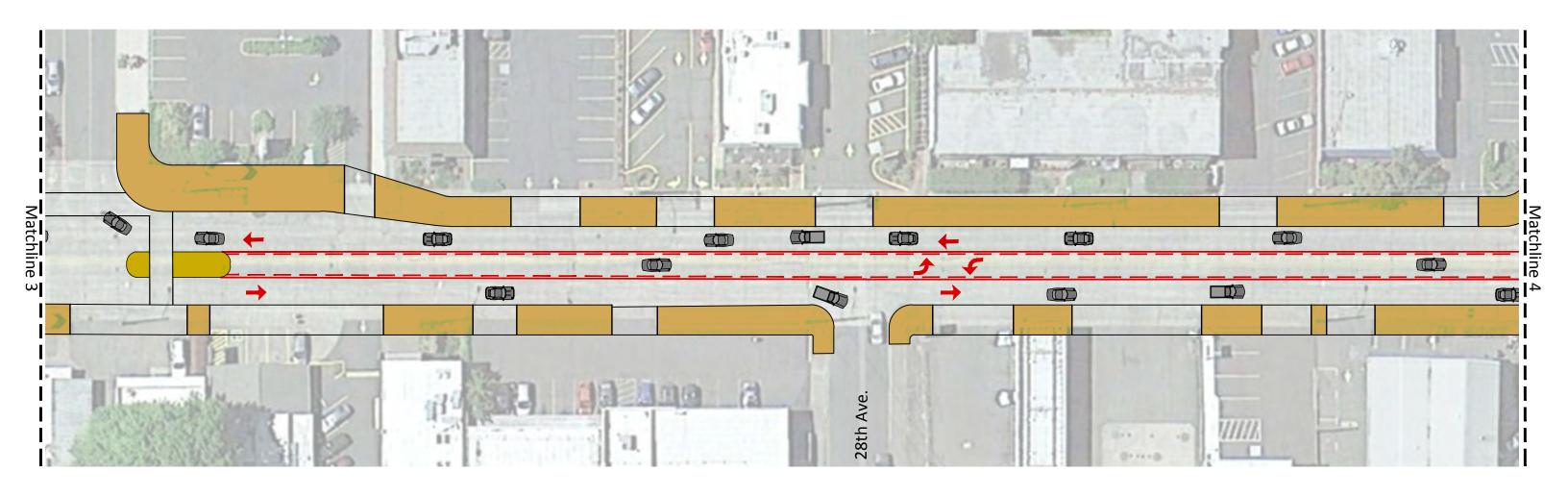


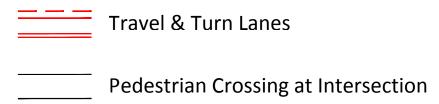




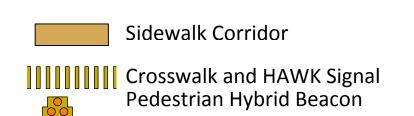


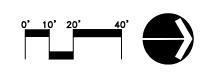






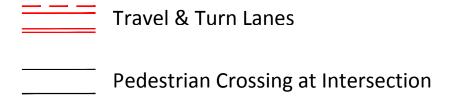








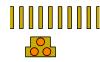
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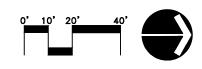


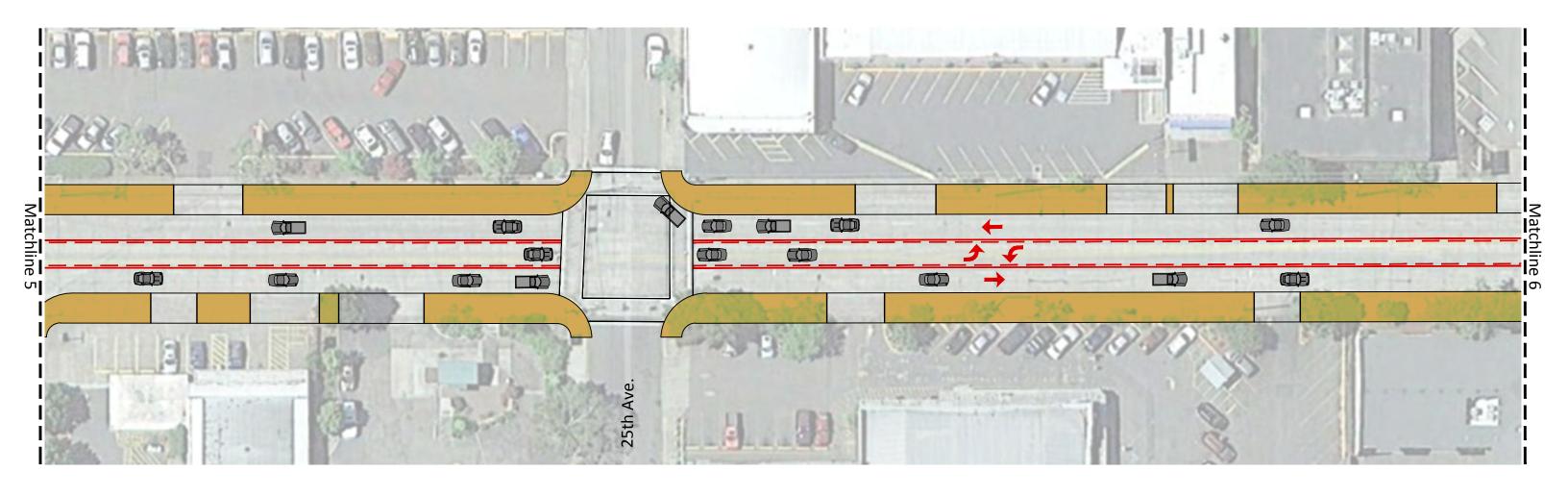


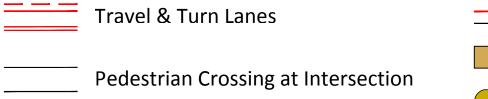
Sidewalk Corridor



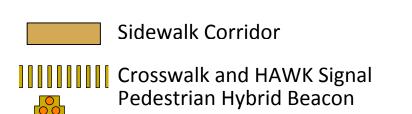
Crosswalk and HAWK Signal Pedestrian Hybrid Beacon

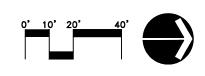


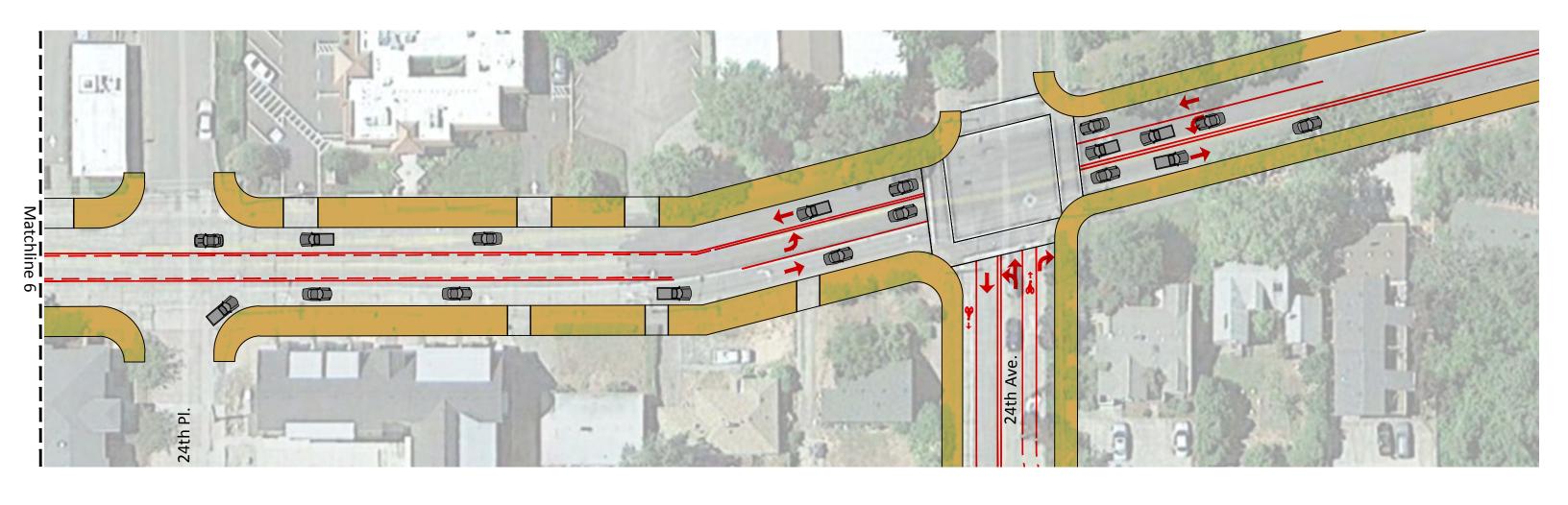


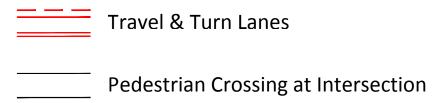




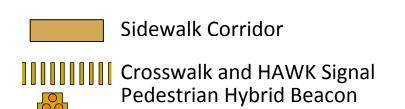


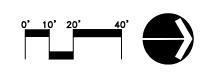












Streetscape Design Basics for Willamette Street

Unifying Elements

Unifying a palette of colors, textures and forms helps assemble a family of streetscape furnishings for Wilamette Street.







































Color

Color is a visually unifying element linked to textural materials. Colors can tie together spaces separated by distance and functions.

Texture

Texture can be a unifying element through a simple and consistent palette of materials. The material may be used in varying combinations as part of streetscape designs.

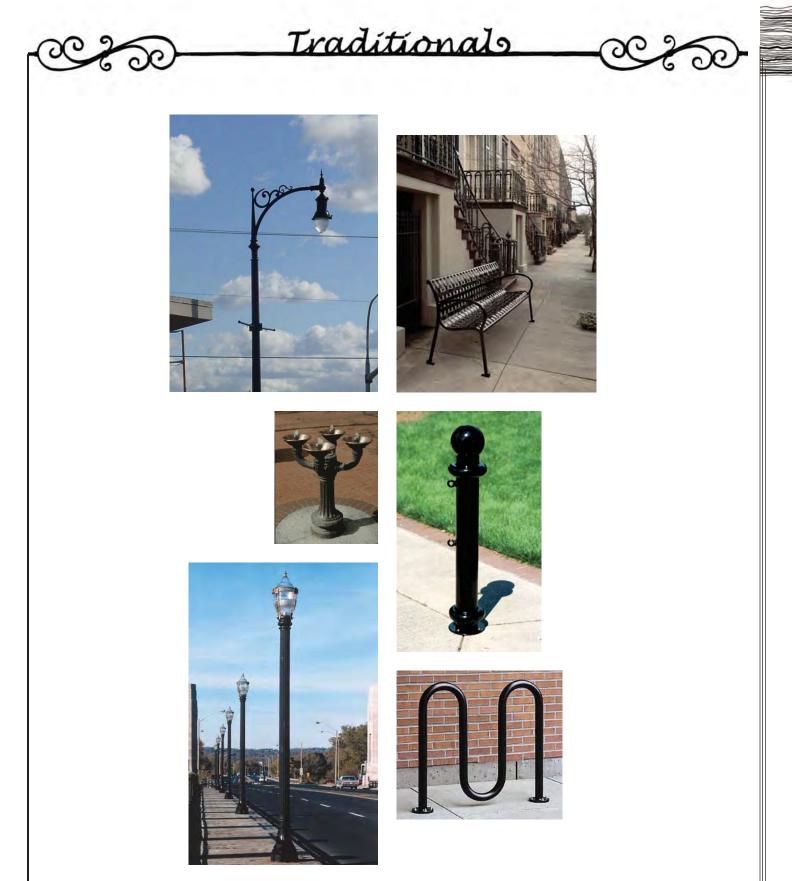
Form

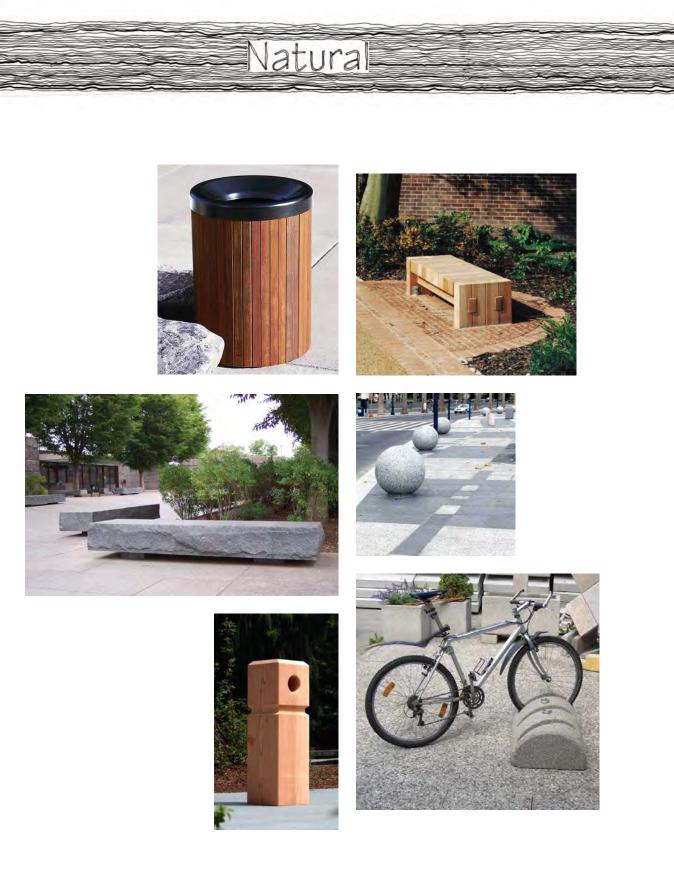
Form can provide both visual unity and interesting visual distinction.

Contemporary

Streetscape Furnishing Themes

Streetscape furnishings are the finishing touches to give Wilamette Street a cohesive sense of place. The emphasis on selecting one unified theme provides a sense of arrival and orientation and provides a human scale connection to the streetscape. Selecting a complementary family of furnishings will provide a thematic consistency that can be applied to the entire corridor.





Streetscape Design Basics for Willamette Street

Public Art

Public art is an opportunity to link us to both the site and our community. Lively public spaces can impact the entire corridor and public art can play an important role in improving the quality of civic life.

















County Courthouse > Singer Falls > Williamette River Trail

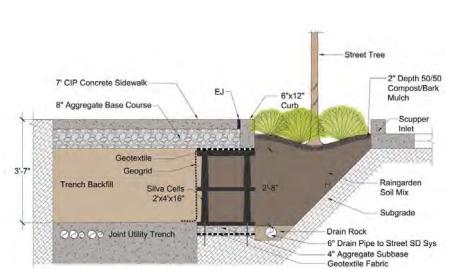
Transit Center Municipal Elevator

Green Street Opportunities

A Green Street is an opportunity to build a relationship between natural systems and the urban environment. Green Street elements are not only limited to techniques for the management of stormwater within the street rightof-way, they can also maximize opportunities for trees and landscaping.



Stormwater Treatment -Filterra





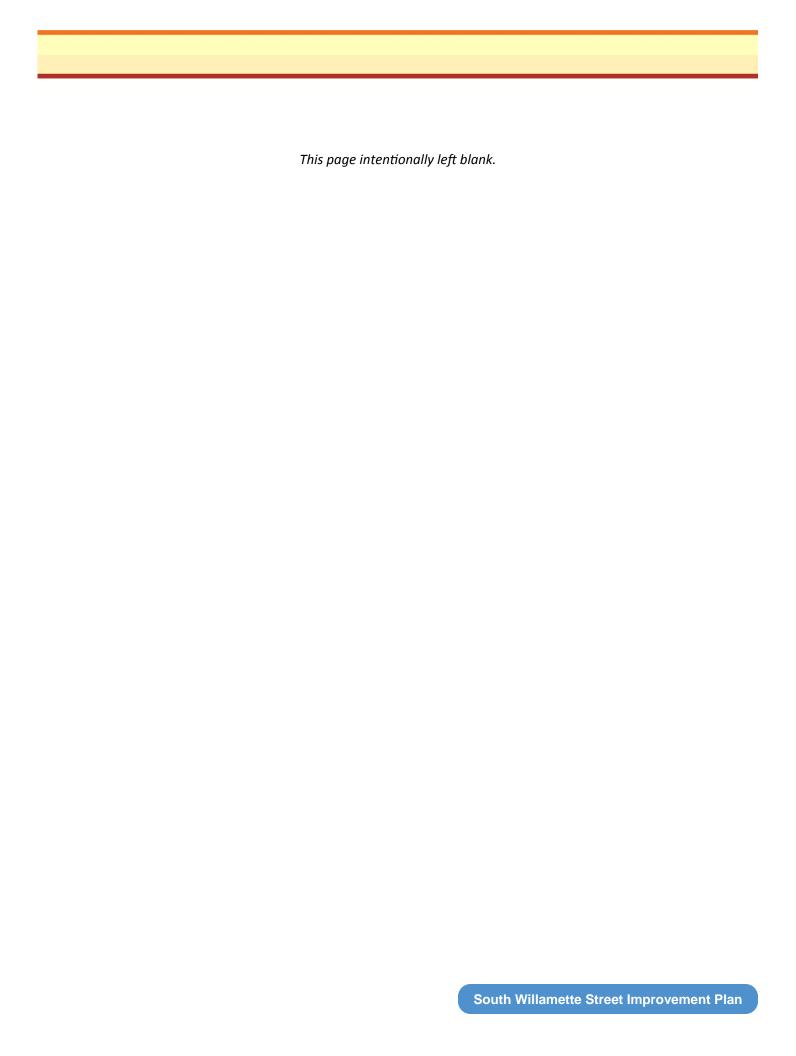
Stormwater Treatment -Silva Cells



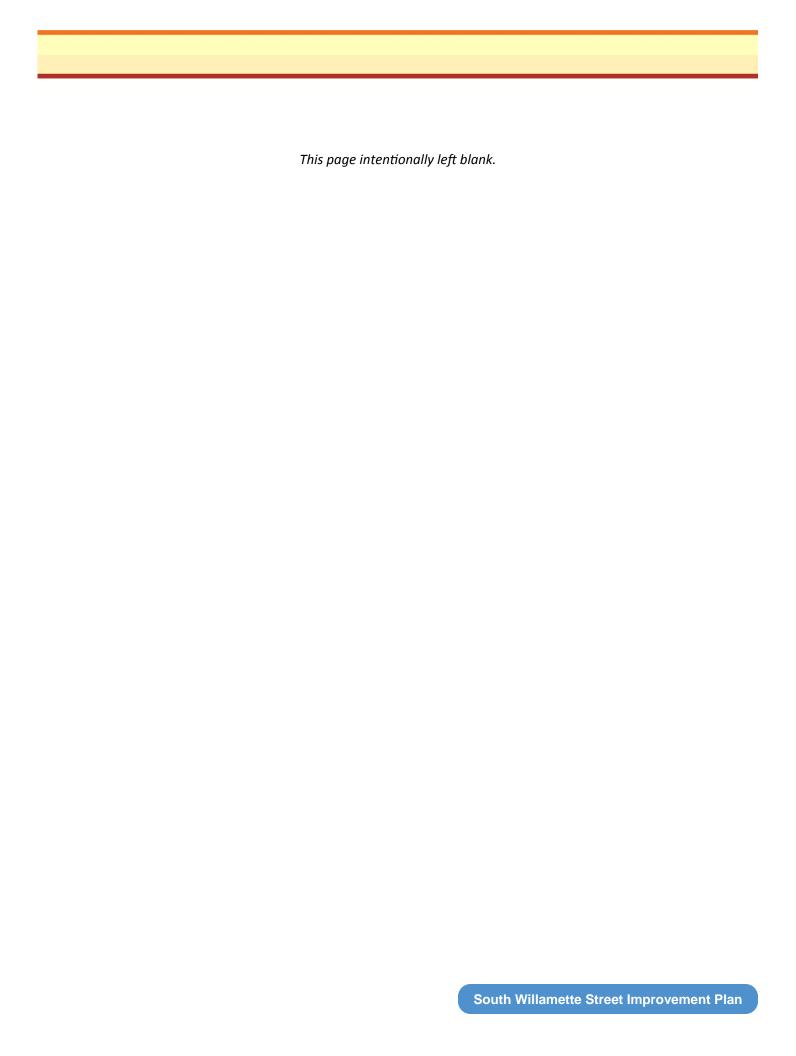
Stormwater Tretment -Flow Through Planter



Energy Conservation -Solar Panels on Bus Stop



TRAFFIC ANALYSIS FOR ROADWAY ALTERNATIVES





117 Commercial Street NE Suite 310 Salem, OR 97301 503.391.8773 www.dksassociates.com

MEMORANDUM #8

DATE: June 12, 2013

TO: Project Management Team

FROM: Scott Mansur, P.E., PTOE

Mat Dolata, P.E., PTP Peter Coffey, P.E.

SUBJECT: South Willamette Street Improvement Plan

Traffic Analysis for Roadway Alternatives

P10086-012

This memorandum summarizes the traffic analysis comparisons of the three alternatives advanced for the South Willamette Street Improvement Plan. The traffic analysis results include estimates of intersection operations, delay, vehicle queuing, travel time, neighborhood traffic shift and multi-modal system performance for bicycles, pedestrians and transit. The configurations of travel lanes, bike lanes, sidewalks, and other roadway elements for each alternative are detailed in Technical Memorandum #7 (Facility Design Alternatives).

Overview

The South Willamette Street Improvement Plan will explore options for people to easily and safely walk, bike, take transit, or drive in an eight-block study area from 24th Avenue to 32nd Avenue. The goal of the study is to help South Willamette Street become a vibrant urban corridor accessible by bicycle, foot, car, and bus. Seven conceptual alternatives have been refined to three based on direction from the City of Eugene staff after receiving community input and reviewing the results of the Tier 1 Screening.

The three alternative configurations advanced to the next screening phase are a 4-lane (Alternative 1), 3-lane with bike lanes (Alternative 3) and 3-lane with wide sidewalks (Alternative 5.) This memorandum, together with Technical Memorandum #7 (Facility Design Alternatives), provides the more detailed description and rigorous analysis of the facility design needed to perform the next level (Tier 2) screening and progress toward a selected design for the corridor.

Traffic Volume Forecasts

Future year travel volume forecasts were developed using the regional travel demand model developed by the Lane Council of Governments (LCOG). The LCOG model provides land use and transportation estimates for base year 2011 and future year 2035. Traffic volumes for 2018 were developed by scaling between traffic counts taken in 2012 and future year 2035 forecasts. Existing conditions and 2035 forecasts are documented in Technical Memorandum #2 (Existing Conditions, Forecast Conditions, and Planned Improvements.) The future year 2018 p.m. peak hour motor vehicle volumes at each study intersection are illustrated in Figure 1 and were used to represent the expected short-term build conditions.

The LCOG travel demand model was also used to evaluate the potential for traffic shifts resulting from modifying portions of Willamette Street from four motor vehicle travel lanes (in Alternative 1) to three (in Alternatives 3



and 5). Due to the anticipated traffic shift of 25 to 100 vehicles in the 2018 p.m. peak hour, the traffic volume forecasts for Alternative 1 differ slightly from the forecasts for Alternatives 3 and 5¹, as shown in Figure 1. The estimates and location of traffic shifts are discussed further in a later section.

Traffic Operations

This section provides a summary of future year (2018) motor vehicle traffic operations based on the traffic volume forecasts developed for 2018.

Peak Hour Intersection Operations

Traffic operations analysis is based on applying 2000 Highway Capacity Manual methodology² for isolated intersections. The estimated average delay, level of service (LOS), and volume to capacity (v/c) ratio of each study intersection is included.

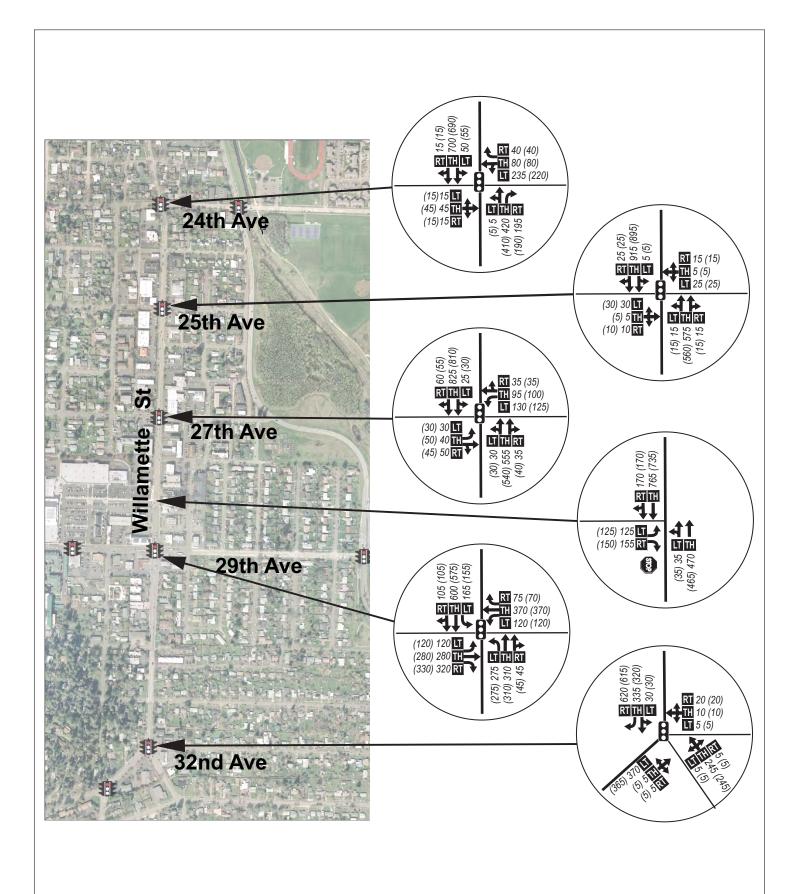
Table 1 compares traffic operations for existing conditions (2012) and future year (2018) conditions for the existing configuration of Willamette Street. As shown, all of the study intersections are anticipated to meet the minimum performance standard of LOS "D" operations. However, more delay is anticipated in 2018 as a result of expected growth in motor vehicle traffic volumes.

Table 1: Intersection Operations – Existing (2012) and Future No-Build (2018)

lutana asti an	Operating	Existin	g P.M. P	eak Hour	201	8 P.M. Pe	eak Hour
Intersection	Standard	Delay	LOS	V/C	Delay	LOS	V/C
Signalized							
Willamette Street/24 th Avenue	LOS D	12.4	В	0.61 (0.74)	12.5	В	0.62 (0.72)
Willamette Street/25 th Avenue	LOS D	10.9	В	0.39 (0.50)	11.7	В	0.40 (0.51)
Willamette Street/27 th Avenue	LOS D	8.6	Α	0.47 (0.50)	9.5	Α	0.51 (0.53)
Willamette Street/29 th Avenue	LOS D	40.7	D	0.83 (0.85)	46.8	D	0.88 (0.90)
Willamette Street/32 nd Avenue	LOS D	6.1	Α	0.63 (0.63)	6.6	Α	0.64 (0.64)
Unsignalized							
Willamette Street/Woodfield Station Driveway	N/A	4.7	A/D	0.58	4.7	A/D	0.59
Signalized Intersections: LOS = Level of Service of Inters V/C = Volume-to-Capacity Ratio Movement)		Critical	LOS	alized Intersection = Level of Service = Volume-to-Cap	ce of Major S		

¹ Although Alternatives 3 and 5 present different corridor configurations, the differences in motor vehicle traffic forecasts are considered to be negligible due to the similarity in motor vehicle capacity through the corridor.

² 2000 Highway Capacity Manual, Transportation Research Board, Washington DC, 2000.







STOP - Stop Sign





000 - Alternative 1 - PM Peak Hour Traffic Volumes

(000) - Alternatives 3 & 5 - PM Peak Hour Traffic Volumes

Left•Thru•Right - Volume Turn Movement





Figure

2018 PM PEAK HOUR TRAFFIC VOLUMES



Table 2 compares 2018 p.m. peak hour traffic operations for Alternatives 1, 3, and 5³. Alternatives 3 and 5 are considered to be the same for motor vehicle traffic operations. The design and configuration of each alternative is further detailed in Technical Memorandum #7 (Facility Design Alternatives). Key facility design assumptions affecting traffic operations are listed below:

- Applying the proposed 3-lane facility design (for Alternatives 3 and 5) on Willamette Street at the 29th Avenue would result in failing operations (LOS F) with traffic demand reaching capacity (v/c of 1.0). Therefore, a modified design including both of the existing southbound through travel lanes (and a left turn pocket) at 29th Avenue is proposed for Alternatives 3 and 5. In this configuration there would be one southbound travel lane at 24th Avenue through 27th Avenue, with the second southbound lane added approximately 100 feet north of the Woodfield Station Driveway and continuing southward to the 32nd Avenue intersection. Through iterative testing of traffic operations at the 29th Avenue intersection, it was determined that adding a second lane at this distance (approximately 450 feet from 29th Avenue) would approximately match the southbound capacity of the intersection in Alternative 1.
- For northbound travel through the 29th Avenue intersection, there are two travel lanes on Willamette Street included in Alternative 1 and one in Alternatives 3 and 5. The existing second northbound travel lane would be replaced by bike lanes (Alternative 3) or wider sidewalks (Alternative 5).
- A traffic signal at the Woodfield Station Driveway intersection is assumed to be constructed in each alternative. The signal provides a pedestrian crossing and improved turning opportunities for motor vehicle traffic.
- The Willamette Street approaches at 24th Avenue, 25th Avenue, and 27th Avenue intersections each have one through lane and a center left turn lane (with permissive left turn signal phasing assumed) in Alternatives 3 and 5.

Table 2: Intersection Operations for Alternatives - Future Year 2018 P.M. Peak Hour

Interpostion	Operating	A	Alternativ	re 1	Alternative 3 and 5			
Intersection	Standard	Delay	LOS	V/C	Delay	LOS	V/C	
Willamette Street/24 th Avenue	LOS D	13.2	В	0.63 (0.75)	22.4	С	0.80 (0.81)	
Willamette Street/25 th Avenue	LOS D	11.8	В	0.40 (0.51)	17.4	В	0.69 (0.91)	
Willamette Street/27 th Avenue	LOS D	10.7	В	0.51 (0.53)	13.9	В	0.82 (0.94)	
Willamette Street/Woodfield Station Driveway	LOS D	12.0	В	0.41 (0.46)	16.2	В	0.45 (0.50)	
Willamette Street/29 th Avenue ⁴	LOS D	48.5	D	0.87 (0.90)	56.3	Е	0.90 (0.94)	
Willamette Street/32 nd Avenue	LOS D	6.6	Α	0.64 (0.64)	6.4	Α	0.63 (0.63)	

Signalized Intersections:

LOS = Level of Service of Intersection

V/C = Volume-to-Capacity Ratio of Intersection (Critical Movement)

³ The 2018 traffic analysis of alternatives assumes bus service frequency is doubled compared to existing service. Pedestrian crossing volumes at study intersections are also assumed to approximately double.

⁴ The saturation flow rate for the northbound approach was reduced by approximately 15% to reflect simulation results showing lanes being blocked in Alternatives 3 and 5.

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June 12, 2013
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For most study intersections, more delay is anticipated in Alternatives 3 and 5 due to the reduction of travel lanes for motor vehicles. However, all of the study intersections are anticipated to meet the minimum performance standard of LOS "D" operations in all alternatives, with the exception of Willamette Street at 29th Avenue in Alternative 3 or 5.

At the intersection of Willamette Street and 29th Avenue, the southbound capacity is maintained (two southbound travel lanes and a left turn pocket) to serve the peak direction of travel (critical movement) resulting in no significant change in traffic delay in the southbound direction. However, the northbound approach has one fewer travel lanes and motor vehicle delay would increase for northbound travel. Furthermore, the northbound left turn lane may regularly exceed the available storage length of 155 feet. In the existing configuration (and Alternative 1), through traveling vehicles may use the right lane to get around when the left lane is blocked by the full left turn lane. With one through travel lane (Alternatives 3 and 5), the second lane will not be available and therefore through traveling vehicles will be blocked. This situation may be mitigated by modifying signal timing to provide more green time to the northbound left turn (which requires increasing delay for other movements) or widening to extend the storage length of the northbound left turn pocket.

Off-Peak Intersection Operations

The following section identifies intersection operations in 2018 during three periods outside of the p.m. peak hour: the a.m. peak hour (8-9 a.m.), the mid-day peak hour (12-1 p.m.), and the p.m. peak shoulder (4-5 p.m.). Traffic volume forecasts for each period were based on the traffic counts and the growth rate identified for the p.m. peak hour⁵. No differences in traffic volumes (shifts in traffic patterns) are assumed to occur between alternatives during the off-peak periods due to lower overall congestion and delay. The results of the off-peak intersection operations analysis, comparing Alternative 1 to Alternatives 3 and 5, are shown in Table 3.

A.M. Peak Hour

The a.m. peak hour has heavier traffic volumes in the northbound direction, mirroring the higher southbound traffic volumes observed during the p.m. peak hour. Overall traffic volumes are lower in the a.m. peak hour than the p.m. peak hour. The existing traffic patterns on Willamette Street are further detailed in Technical Memorandum #2 (Existing Conditions, Forecast Conditions, and Planned Improvements).

As shown in Table 3, for most study intersections, more delay is anticipated in Alternatives 3 and 5 due to the reduction of travel lanes for motor vehicles. However, all of the study intersections are anticipated to meet the minimum performance standard of LOS "D" operations in all alternatives for the a.m. peak hour, with the exception of Willamette Street at 29th Avenue in Alternative 3 or 5.

Compared to the p.m. peak hour, there is generally less average delay during the a.m. peak at Willamette Street intersections between 24th Avenue and the Woodfield Station Driveway. However, due to the directional characteristics of the a.m. traffic volume, delay on northbound approaches is higher in the a.m. peak.

⁵ The 2018 p.m. peak hour growth rate for each intersection was applied to the traffic counts taken for the a.m. peak hour and p.m. peak shoulder to estimate the 2018 turn movement volumes. Although intersection traffic counts were not available for the mid-day peak hour, 24-hour bidirectional counts taken on Willamette Street (south of 27th Avenue) were used together with the p.m. peak hour intersection traffic counts to estimate the intersection turn movements from 12-1 p.m.



Table 3: Intersection Operations for Alternatives – Future Year 2018 Off-Peak Hours

Interception	Operating	-	Alternativ	re 1	Alternative 3 and 5			
Intersection	Standard	Delay	LOS	V/C	Delay	LOS	V/C	
A.M. Peak Hour								
Willamette Street/24 th Avenue	LOS D	8.9	Α	0.55 (0.56)	12.5	В	0.73 (0.86)	
Willamette Street/25 th Avenue	LOS D	6.6	Α	0.36 (0.48)	12.0	В	0.62 (0.85)	
Willamette Street/27 th Avenue	LOS D	8.1	Α	0.39 (0.45)	15.8	В	0.69 (0.85)	
Willamette Street/Woodfield Station Driveway	LOS D	5.4	Α	0.32 (0.37)	6.9	Α	0.55 (0.50)	
Willamette Street/29 th Avenue	LOS D	44.6	D	0.80 (0.90)	58.9	Е	0.97 (0.97)	
Willamette Street/32 nd Avenue	LOS D	14.3	В	0.81 (0.83)	14.3	В	0.81(0.83)	
Mid-day Peak Hour	•							
Willamette Street/24 th Avenue	LOS D	10.1	В	0.58 (0.65)	17.4	В	0.72 (0.72)	
Willamette Street/25 th Avenue	LOS D	10.0	Α	0.31 (0.39)	14.0	В	0.54 (0.70)	
Willamette Street/27 th Avenue	LOS D	8.4	Α	0.40 (0.42)	12.1	В	0.67 (0.76)	
Willamette Street/Woodfield Station Driveway	LOS D	10.2	В	0.32 (0.36)	11.9	В	0.51 (0.53)	
Willamette Street/29 th Avenue	LOS D	42.6	D	0.68 (0.86)	48.3	D	0.80 (0.88)	
Willamette Street/32 nd Avenue	LOS D	7.0	Α	0.67 (0.67)	7.0	Α	0.67 (0.67)	
P.M. Peak Shoulder Hour		_						
Willamette Street/24 th Avenue	LOS D	10.6	В	0.56 (0.64)	19.5	В	0.69 (0.72)	
Willamette Street/25 th Avenue	LOS D	9.1	Α	0.38 (0.48)	16.3	В	0.66 (0.85)	
Willamette Street/27 th Avenue	LOS D	10.6	В	0.52 (0.55)	15.7	В	0.84 (0.95)	
Willamette Street/Woodfield Station Driveway	LOS D	11.1	В	0.38 (0.43)	15.2	В	0.43 (0.46)	
Willamette Street/29 th Avenue	LOS D	48.3	D	0.81 (0.91)	53.9	D	0.87 (0.93)	
Willamette Street/32 nd Avenue	LOS D	6.8	Α	0.62 (0.62)	6.8	Α	0.62 (0.62)	

Signalized Intersections:

LOS = Level of Service of Intersection

V/C = Volume-to-Capacity Ratio of Intersection (Critical Movement)

The intersection at 29th Avenue would have higher overall average delay in Alternative 3 and 5 during the a.m. peak hour compared to the p.m. peak hour. Alternative 3 and 5 provide one northbound through lane (compared to two in Alternative 1). The northbound approach volumes would come close to the available capacity during the 2018 a.m. peak, resulting in slightly higher overall delay compared to the p.m. peak hour. As in the p.m. peak hour, the intersection operations for Alternative 3 and 5 would be LOS "E" at the 29th Avenue intersection, exceeding the existing minimum performance standard for operations.

Traffic Analysis for Roadway Alternatives
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Mid-day Peak Hour

Overall traffic volumes in the mid-day peak hour are expected to be lower than in either the a.m. or p.m. peak hours. The mid-day peak hour has relatively balanced traffic volumes in the southbound and northbound directions. As shown in Table 3, all of the study intersections are anticipated to meet the minimum performance standard of LOS "D" operations in all alternatives for the mid-day peak hour. For most study intersections, more delay is anticipated in Alternatives 3 and 5 due to the reduction of travel lanes for motor vehicles.

P.M. Peak Shoulder Hour

The p.m. peak "shoulder" hour is the period that precedes the p.m. peak hour, when overall traffic volumes are slightly lower. Overall traffic volumes during the p.m. peak shoulder are higher than the a.m. or mid-day peak hours. All of the study intersections are anticipated to meet the minimum performance standard of LOS "D" operations in all alternatives, with more delay anticipated in Alternatives 3 and 5 due to the reduction of travel lanes for motor vehicles.

Vehicle Queuing

The following section describes the differences in estimated p.m. peak hour vehicle queuing between alternatives. Traffic simulations were performed for the 2018 p.m. peak hour to estimate expected vehicle queuing. The simulation results including vehicle queuing for all lane movements are detailed in the appendix. The key changes between alternatives to northbound and southbound queues are shown Table 4. Vehicle queue lengths on side streets would be generally unaffected by the proposed changes on Willamette Street for Alternatives 3 and 5.

The results of the p.m. peak hour vehicle queuing comparison between Alternative 1 and Alternatives 3 and 5 indicate that vehicle queuing increases most significantly for southbound through travel between 24th Avenue and 27th Avenue and northbound through travel at 29th Avenue. Average southbound vehicle queues between 24th and 27th Avenue may increase by 50 to 150 feet (or approximately 2-6 car lengths) at these intersections. However, with dedicated left turn lanes present, vehicle queues for left turns would decrease.

At 29th Avenue, removing one of the two northbound through travel lanes would increase northbound vehicle queues by up to 200 feet (or approximately 8 car lengths). As a result, access to the northbound left turn lane may be blocked more frequently during peak hours.



Table 4: Estimated Vehicle Queuing Comparison for Alternatives - Future Year 2018 P.M. Peak Hour

Intersection	Movement/ Direction	Alterna	ative 1	250 180 2d through/turn lanes ght turn lane northboroinclude a shared ions. As a result, buthbound vehicle questions. As a result, buthbound vehicle questions are (and center left be approximately two chicle queues on the result lanes would begin thicles would decrea natives 3 and 4. Very unchanged. 220 310 220 310 220 ifficantly change in retained as describe to 1 for through transport for the retained as describe to 1 for through transpoles will real result, the 29th Avenue and the poles will real result, the 29th Avenue and the poles will real result, the 29th Avenue and the poles will real result, the 29th Avenue and the poles will real result, the 29th Avenue and the poles will real result, the 29th Avenue and the poles will real result, the 29th Avenue and the poles will real result, the 29th Avenue and the poles will real result, the 29th Avenue and the poles will real result, the 29th Avenue and the poles will real result, the 29th Avenue and the poles will real result, the 29th Avenue and the poles will real result, the 29th Avenue and the poles will real result, the 29th Avenue and the poles will real result, the 29th Avenue and the poles will real result.	ve 3 or 5					
intersection	Movement/ Direction	Average	95 th	Average	95 th					
	Southbound	180	290	250	430					
	Northbound	100	170	180	260					
Willamette Street/ 24 th Avenue	Comment: The existing lane of southbound and a shared thro This configuration would be miright/through lane with a dedic operational efficiency would do would increase by 70 to 140 fe	ugh/left lane with odified in Alterna ated left turn lan ecrease and nor	h a dedicated ratives 3 and 5 to the in both direction.	ight turn lane n to include a sha tions. As a resu	orthbound. ared ult,					
	Southbound	140	230	290	570					
Willamette Street/	Northbound	100	190	220	370					
Willamette Street/ 25 th Avenue Willamette Street/ 27 th Avenue	Comment: By converting from two through lanes to a single lane (and center left turn lane), the northbound and southbound vehicle queues would be approximately twice as long in Alternatives 3 and 5, increasing by 120 to 340 feet. Vehicle queues on the mind street would be relatively unchanged.									
	Southbound	100	190	230	560					
	Northbound	100	160	160	290					
	Comment: By converting from two through lanes to a single lane (and center left turn lane), the northbound and southbound vehicle queues would be approximately twice as long in Alternatives 3 and 5, increasing by 60 to 370 feet. Vehicle queues on the minor street would be relatively unchanged.									
	Southbound	180	290	230	320					
	Northbound	50	110	40	100					
Willamette Street/ Woodfield Station Driveway	Comment: Alternatives 3 and two shared through/turn lanes Woodfield Station Driveway. In phasing at the intersection. So feet north of the intersection, the longer queues may increase be queues on the Woodfield Stati	to one lane with The northbound buthbound, one nerefore storage y approximately on Driveway wo	n a dedicated le left would prov of the two trave e for queuing ve 50 feet in Alte uld be relative	eft turn lane into ide protected le el lanes would t ehicles would d rnatives 3 and 4 y unchanged.	o the left turn pegin 100 ecrease and 4. Vehicle					
	Southbound	220	300	220	300					
Willamette Street/ 25 th Avenue Willamette Street/ 27 th Avenue Willamette Street/ Noodfield Station Driveway Willamette Street/ 29 th Avenue	Northbound (Through/Right)	140	300	310	530					
NACH CO. III	Northbound (Left)	190	270	220	300					
29 th Avenue	Comment: Southbound vehicl Alternatives 3 and 5 (assuming previously). The proposed not would result in vehicle queues the 155 foot-long northbound I have a second lane available tintersection may operate less.	g the two southb rthbound lane re increasing by a eft turn lane fills to pass the left-ti	oound lanes are eduction (from 2 pproximately 2 up, through tra urn queue. As	e retained as de 2 to 1 for throug 00 feet. In add aveling vehicles	escribed gh travel) ition, when s will not					
Willamette Street/ 32 nd Avenue	Comment: There are no chan no significant changes to vehic			is intersection.	As a result					
	nulation queue length (feet) entile (highest five percent) simul	ation queue leng	gth (feet)							



Travel Time

The following section describes the estimated p.m. peak hour travel times for alternatives. Traffic simulations were performed for the 2018 p.m. peak hour to estimate travel time between 24th Avenue and 32nd Avenue in both directions. The base year simulations were calibrated to field-measured travel times. The simulation results including travel times are detailed in the appendix.

The estimated travel times for each alternative are summarized in Table 5. Results of the simulation indicate travel times would increase by approximately 30 seconds in both directions for Alternatives 3 and 5. In addition, the reliability of travel time may be better in Alternative 1, as simulation results for Alternatives 3 and 5 showed increased variance.

Table 5: Estimated Travel Time Comparison for Alternatives - Future Year 2018 P.M. Peak Hour

Direction	Alternative 1	Alternative 3 and 5
Northbound:	2 minutes 55 seconds –	3 minutes 15 seconds –
32 nd Avenue to 24 th Avenue	3 minutes 05 seconds	3 minutes 45 seconds
Southbound:	3 minutes 20 seconds –	3 minutes 30 seconds –
24 th Avenue to 32 nd Avenue	4 minutes 10 seconds	4 minutes 50 seconds

Roundabout Evaluation

Roundabouts can improve traffic flow and reduce overall delay at many roadway intersections. Roundabouts generally reduce the number of overall collisions and fatalities when they are installed and are less expensive to operate and maintain compared to traffic signals. However, emergency vehicle and truck operators may be opposed to roundabouts in sensitive areas. Furthermore, there may be significant property acquisition costs to provide the right-of-way needed to construct appropriately-sized roundabouts.

To evaluate the effectiveness of roundabouts on Willamette Street, each of the study intersections was analyzed with a potential roundabout configuration. The assumed size and layout of the roundabouts analyzed are typical for urban environments. The results of the traffic operations analysis for the 2018 p.m. peak hour are summarized in Table 6.

Table 6: Intersection Operations for Roundabouts - Future Year 2018 P.M. Peak Hour

Intersection	Delay	LOS	V/C	Roundabout Layout
Willamette Street/24th Avenue	18.9	B (C)	0.97	Single-lane
Willamette Street/25th Avenue	7.3	A (C)	0.74	Single-lane
Willamette Street/27th Avenue	18.9	B (C)	0.97	Single-lane
Willamette Street/Woodfield Station Driveway	9.2	A (C)	0.67	Single-lane (3-leg)
Willamette Street/29th Avenue	22.4	C (D)	0.89	Multi-lane Roundabout (2-lanes)
Willamette Street/32nd Avenue	10.8	B (B)	0.70	Single-lane

Delay = Average Delay (seconds) on all intersection movements

LOS = Level of service of intersection (and worst approach)

V/C = Volume-to-Capacity Ratio (measured for worst approach)

Note: Operations analysis based on NCHRP 572 capacity model (adopted by ODOT)

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The analysis indicates that several intersections would have approaches operating near capacity during the p.m. peak hour if constructed as single lane roundabouts. At the intersection of 24th Avenue, the southbound approach of Willamette Street would be near capacity during the p.m. peak hour, due in part to the volume of southbound vehicles conflicting with westbound left turns from 24th Avenue to Willamette Street. A similar situation would exist at 27th Avenue, where the southbound approach would also be near capacity. Although roundabout operations would adequately serve traffic demand at the 25th Avenue and Woodfield Station Driveway intersections, mixing traffic signals and roundabouts in close proximity along the corridor could present negative outcomes for traffic operations and safety due to driver expectations.

The intersection of Willamette Street and 29th Avenue would need to be constructed as a multi-lane roundabout to sufficiently serve the traffic demand. Constructing such a roundabout would result in significant property acquisition at the intersection, as detailed in Technical Memorandum #7 (Facility Design Alternatives). Due to the roadway slope and sight distance on Willamette Street south of the 32nd Avenue intersection, a potential roundabout at this location may not be appropriate and would require further analysis.

While it is possible for larger roundabouts (with additional lanes) to adequately serve the future peak hour traffic demand, the costs of right-of-way acquisition and impacts to business owners and properties adjacent to these intersections would be significant. Roundabouts are not explicitly included in the facility design of any alternative but may be considered further as potential design refinements.

Bicycle Lanes Effects on Traffic Operations

The bicycle lanes included in Alternative 3 would make Willamette Street a more attractive bike route to many types of riders. The bike lanes would also provide a buffer for pedestrians. Bike lanes make it easier for cars and trucks to maneuver in and out of driveways, compared to a three-lane section with no bike lanes. In addition, buses would stop in bike lanes during passenger boarding, which would provide additional space for motor vehicles to overtake the bus when it is safe to do so.

However, to construct bike lanes either the roadway must be widened or existing travel lanes must be removed. Previous sections of this memorandum have covered the increased motor vehicle delay that results from removing travel lanes (i.e., traffic operations in Alternative 1 compared to Alternatives 3 and 5). This section discusses the differences in traffic operations between Alternative 3 and Alternative 5 (i.e., the effect of bike lines to otherwise identical roadway configurations).

Although bicycle lanes would not have a significant direct effect on motor vehicle operations, higher volumes of bicycles on the roadway may increase delays for turning motor vehicles. The magnitude of potential increase in bicycle traffic is not precisely known. However, to demonstrate potential sensitivity of motor vehicle operation to bike lanes, the intersection operations analysis was repeated with existing bicycle volumes doubled. Traffic operations analysis outputs, with bicycle volumes doubled for Alternative 3 are included in the appendix.

The results of this analysis indicate that doubling bike volumes would increase average delay per motor vehicle by less than half a second at all study intersections. No changes to level of service results were found to result from this sensitivity test. As a result of this analysis, motor vehicle traffic operations for Alternatives 3 and 5 are considered to be the same.

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Bus Pullout Effects on Traffic Operations

Bus pullouts provide a dedicated space outside of the primary travel lane for passenger loading and offloading. Where bus pullouts are constructed, buses exit the travel lane for passenger loading and reenter (merge) after loading is complete. The primary benefit of bus pullouts is that motor vehicles avoid delays when the travel lane is blocked by stopped buses. However, bus service would likely incur increased delay and potential conflicts when attempting to merge back into the travel lane. Therefore, transit operators often prefer to locate bus stops within the travel lane. Lane Transit District (LTD) has no official policy on bus pullouts, but would generally prefer to keep curbside transit stops along Willamette Street.⁶

To attempt to quantify the effect of including bus pullouts, p.m. peak hour intersection traffic operations were evaluated with and without bus blockages for Alternatives 3 and 5. The analysis assumed the existing service frequency was doubled (i.e., twice the number of buses on the corridor relative to the existing service which provides two per hour north of 29th Avenue and the five per hour south of 29th Avenue.) Details for intersection operations with bus pullouts are included in the appendix. Bus pullouts are not considered for Alternative 1 due to the presence of two travel lanes for most of the corridor.

Although travel time would likely increase a few times an hour for vehicles delayed behind slower-moving buses, the average effect for the overall p.m. peak hour is negligible. The results of the analysis indicate that bus pullouts would reduce average delay per vehicle by less than one second at all study intersections. No changes to level of service results were found.

Due to the relatively minor differences in travel delay, the right of way impacts if constructed, increased difficulty for bus operations and lack of support from LTD, bus pullouts are not included in any of the alternatives. Constructing bus pullouts may be revaluated with future redevelopment of the corridor or if additional transit services are provided (e.g., increased frequency, routing changes.)

Traffic Shift

Potential changes in traffic patterns could result from modifying portions of Willamette Street from four motor vehicle travel lanes (in Alternative 1) to three (in Alternatives 3 and 5). With increased travel times on Willamette Street estimated for Alternative 3 and 5, some traffic may shift away from Willamette Street to other roadways. Table 9 identifies estimated traffic volumes on Willamette Street for each alternative.

The LCOG travel demand model was used to evaluate the potential traffic shift away from Willamette Street and the relative effects to other roadways. The expected traffic shift was estimated by comparing differences in alternative model⁷ traffic volumes for the 2035 p.m. peak hour⁸. The traffic shift is expected to be smaller during off-peak periods, when there is less congestion compared to the p.m. peak hour.

⁶ South Willamette Street Improvement Plan Memorandum from Will Mueller, Lane Transit District, March 12, 2013.

⁷ Motor vehicle capacity on Willamette Street between 24th Avenue and 29th Avenue was reduced by approximately 33 percent in the model, to reflect the estimated change for through-traveling vehicles along a three-lane arterial compared to a four-lane arterial. This capacity reduction is based on typical travel demand model assumptions for capacities of urban roadways. For the modified design proposed in Alternatives 3 and 5, the addition of a second southbound travel lane near 29th Avenue would provide more capacity than is represented in the travel demand model. As a result of this simplified assumption, the change in capacity in the models is likely to be overestimated relative to the modified facility design. Therefore, the model traffic shift may be considered a high-end estimate.

⁸ The LCOG travel demand models for the 2035 p.m. peak hour indicate that approximately 350 vehicles may shift from traveling via

The LCOG travel demand models for the 2035 p.m. peak hour indicate that approximately 350 vehicles may shift from traveling via Williamette Street. This represents approximately 20 percent of total estimated traffic volumes for the 2035 p.m. peak hour. Because



Table 9: Willamette Street Traffic Volume Comparison for Alternatives -Future Year 2018

Scenario/Measure	Average Daily	P.M. Peak Hour
Current Year (2012)	16,360	1,550
Alternative 1	17,200	1,625
Alternative 3 & 5	16,700 to 17,100	1,525 to 1,600
Change (compared to Alternative 1)	100 to 500	25 to 100
Percent Change (compared to Alternative 1)	1 to 3%	2 to 6%
Traffic volume estimates are for V	Villamette Street south of 27 th Ave	nue

The distribution of traffic shifts rerouting away from Willamette Street was based on analysis of the LCOG regional travel demand model results. Traffic shifting away from Willamette Street would primarily reroute to streets east of Willamette Street. Approximately two thirds of the shift would go to Amazon Parkway and Hilyard Street. Approximately one third of the shift would redistribute to streets west of Willamette Street including Lincoln Street, Jefferson Street, Adams Street and Polk Street. The traffic shift west of Willamette Street would be fairly evenly distributed between those roadways.

Multimodal Level of Service

Auto, pedestrian, bicycle and transit operations along Willamette Street were evaluated using the multi-modal level of service (MMLOS) methodologies outlined in the *Highway Capacity Manual 2010 (HCM2010)*⁹. The MMLOS evaluation assesses how well a facility meets the needs of the traveling community by reporting a LOS grade (A-F) for each mode of transportation. This evaluation is performed for roadway segments and focuses on the users' perceived comfort level as they travel along the corridor.

Using signalized intersections as break points, Willamette Street was divided into four segments for analysis. Analysis was performed based on 2018 p.m. peak hour conditions when the higher traffic volumes would result in the worst case level of service for each mode of transportation. The methodology does not account for intersection operations, which were addressed previously.

Pedestrian LOS is influenced by traffic volumes, vehicle speeds, sidewalk width, and presence of a buffer. Bicycle LOS is influenced by bike lane width, pavement quality, on-street parking, and heavy vehicle percentage. Transit LOS is influenced by service frequency, bus reliability, average passenger load, and transit stop amenities.

traffic shifts are assumed to occur in proportion to increases in congestion that will occur with anticipated traffic growth, the estimated traffic shift for 2018 is significantly smaller than in 2035. Traffic volume shift estimates for 2018 were developed by proportionally scaling the 2035 traffic shift to forecasted traffic growth in 2018.

⁹ This analysis was performed using the LOS+ software that is a hybrid tool that utilizes two different MMLOS methodologies. The auto LOS component of the analysis is based on NCHRP Project 3-70, while the pedestrian, bicycle, and transit components are based on the HCM2010. While NCHRP 3-70 provided the basis for the MMLOS methodology described in the HCM2010, there were some significant differences. One of the main differences is that the LOS methodology for autos presented in the NCHRP 3-70 report requires less input data and is less intensive computationally. The LOS+ software was developed by Fehr and Peers.

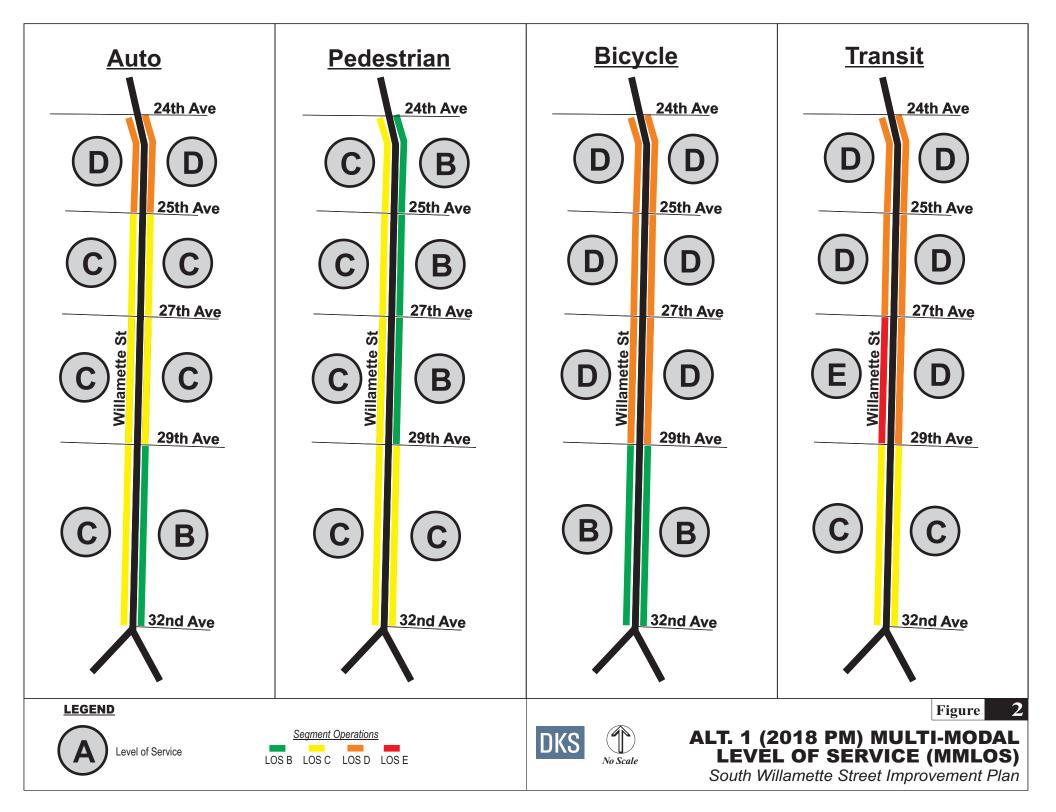
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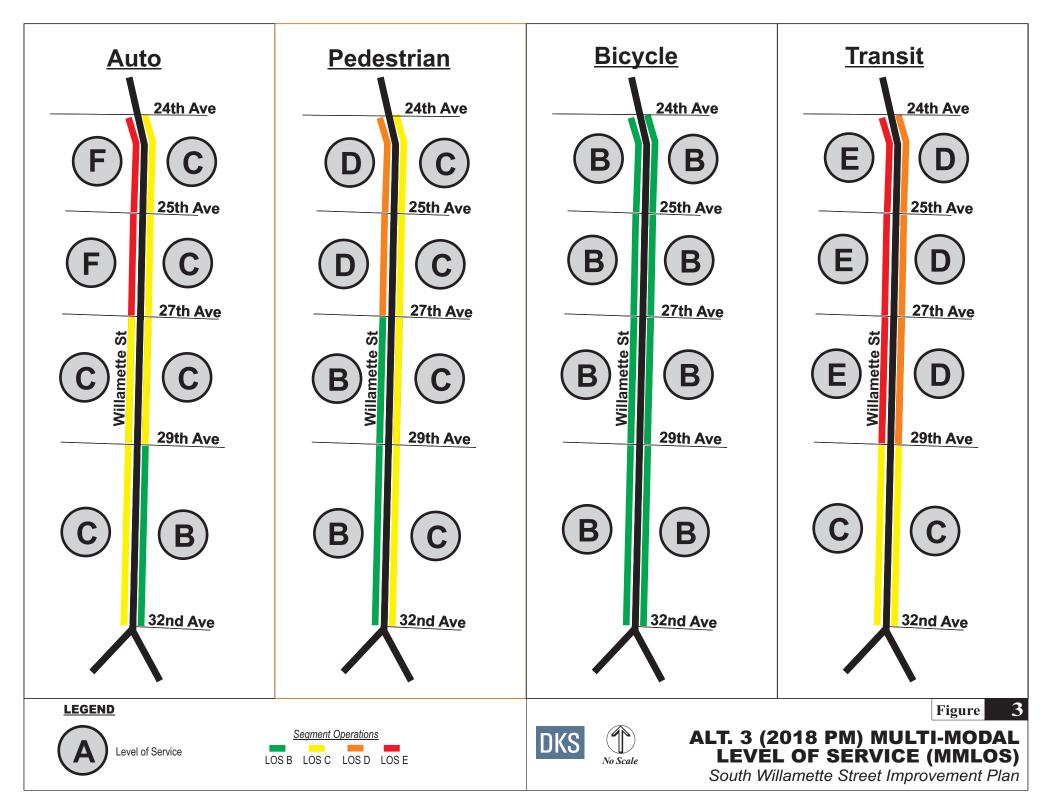


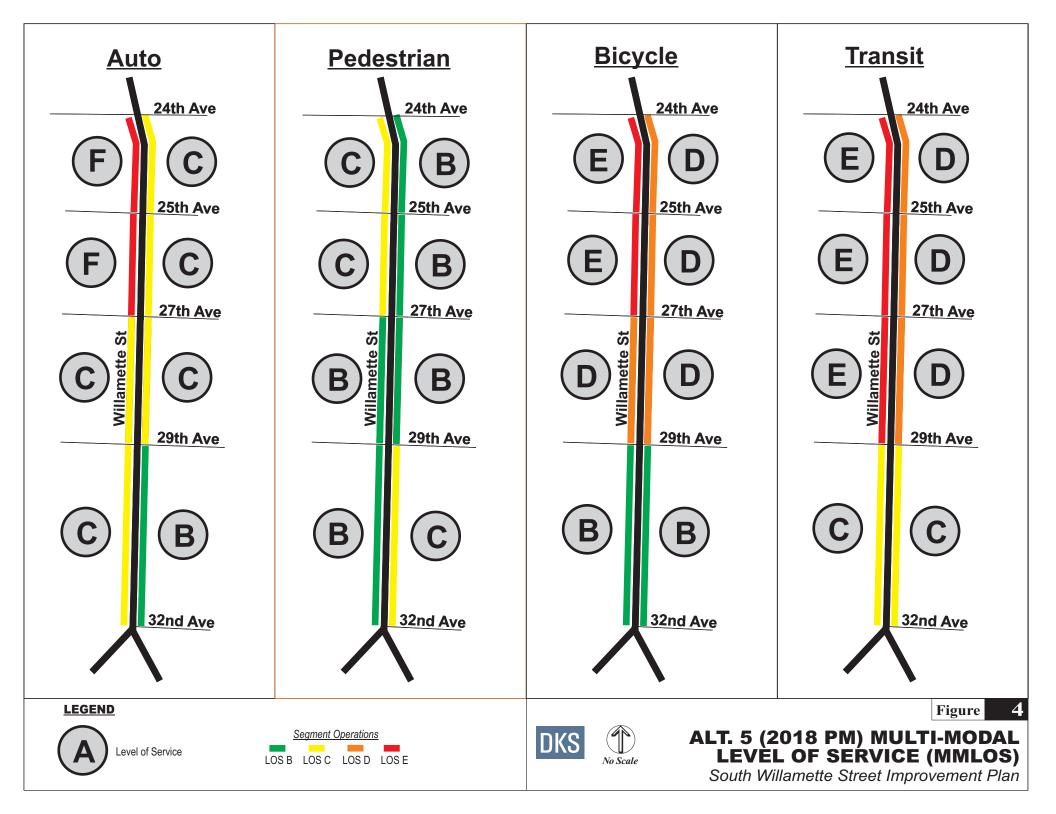
The limitations of the MMLOS analysis should be noted. For example, the existing facilities on Willamette Street were evaluated as LOS "D" MMLOS operations, a better than expected rating. Based on stakeholder interviews, most bicycle users are not comfortable biking on Willamette Street without bike lanes. Therefore, it is clear that the comfort level of motor vehicles driving on a roadway with LOS "D" conditions is not a suitable comparison to cyclists travelling on a facility with LOS "D" conditions. Despite the limitations, the MMLOS evaluation provides value as an objective comparison between alternatives that consider multiple modes.

The expected MMLOS operations for Willamette Street in the 2018 p.m. peak hour are shown for each Alternative in Figure 4. Results are summarized for each mode below:

- The auto mode results indicate the best performance in Alternative 1, with southbound segments from 24th Avenue to 27th Avenue degrading from LOS C or D to LOS F in Alternatives 3 and 5.
- The pedestrian mode results are best for Alternative 5, with several segments improving due to wider sidewalks than Alternative 1 or 3. Alternative 3 results in the lowest pedestrian operations; LOS D southbound between 24th Avenue and 27th Avenue, due to the higher volume of vehicles in the near travel lane. The MMLOS methodology rates pedestrian comfort higher in Alternative 1 than Alternative 3 despite the presence of a bike lane serving as a buffer between cars and pedestrians.
- Bicycle operations would improve from LOS D to LOS B by replacing a motor vehicle lane with continuous bike lanes (Alternative 3). However, bicycle operations would degrade from LOS D to LOS E on some segments if travel lanes are reduced without adding bike lanes (Alternative 5).
- Transit operations are rated slightly higher in Alternative 1 than in Alternatives 3 and 5 due to providing the highest level of mobility (i.e., travel time) for all motor vehicles, including buses.







Appendix

- Intersection Operations Analysis, 2018

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4	7		4	7		414	
Volume (vph)	15	45	15	220	80	40	5	420	195	50	700	15
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Lane Width	12	14	12	12	12	12	12	12	12	12	12	12
Total Lost time (s)		4.5			4.5	4.5		4.5	4.5		4.5	
Lane Util. Factor		1.00			1.00	1.00		1.00	1.00		0.95	
Frpb, ped/bikes		0.99			1.00	0.96		1.00	0.96		1.00	
Flpb, ped/bikes		1.00			1.00	1.00		1.00	1.00		1.00	
Frt		0.97			1.00	0.85		1.00	0.85		1.00	
Flt Protected		0.99			0.96	1.00		1.00	1.00		1.00	
Satd. Flow (prot)		1784			1675	1427		1732	1396		3270	
Flt Permitted		0.92			0.74	1.00		0.99	1.00		0.90	
Satd. Flow (perm)		1656			1287	1427		1720	1396		2937	
Peak-hour factor, PHF	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Adj. Flow (vph)	16	49	16	242	88	44	5	462	214	55	769	16
RTOR Reduction (vph)	0	10	0	0	0	28	0	0	102	0	1	0
Lane Group Flow (vph)	0	71	0	0	330	16	0	467	112	0	839	0
Confl. Peds. (#/hr)	7		6	6		7	11		6	6		11
Confl. Bikes (#/hr)			26			6			17			11
Heavy Vehicles (%)	0%	0%	0%	0%	1%	0%	0%	1%	1%	0%	1%	0%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	2	0	0	0
Turn Type	Perm	NA		Perm	NA	Perm	Perm	NA	Perm	Perm	NA	
Protected Phases		2			6			8			4	
Permitted Phases	2			6		6	8		8	4		
Actuated Green, G (s)		26.9			26.9	26.9		39.1	39.1		39.1	
Effective Green, g (s)		26.9			26.9	26.9		39.1	39.1		39.1	
Actuated g/C Ratio		0.36			0.36	0.36		0.52	0.52		0.52	
Clearance Time (s)		4.5			4.5	4.5		4.5	4.5		4.5	
Vehicle Extension (s)		3.0			3.0	3.0		2.0	2.0		2.0	
Lane Grp Cap (vph)		593			461	511		896	727		1531	
v/s Ratio Prot		0.04			0.07	0.04		0.07	0.00		0.00	
v/s Ratio Perm		0.04			c0.26	0.01		0.27	0.08		c0.29	
v/c Ratio		0.12			0.72	0.03		0.52	0.15		0.55	
Uniform Delay, d1		16.1			20.8	15.6		11.8	9.3		12.0	
Progression Factor		1.00			1.00	1.00		0.30	0.04		1.00	
Incremental Delay, d2		0.1			5.2	0.0		2.1	0.4		1.4	
Delay (s)		16.2			26.0 C	15.6		5.6	0.8 A		13.4 B	
Level of Service		B 16.2			24.8	В		A 4.1	А		13.4	
Approach LOS		10.2 B			24.8 C			4.1 A			13.4 B	
Approach LOS		ь			C			А			ь	
Intersection Summary												
HCM 2000 Control Delay			12.5	Н	CM 2000	Level of	Service		В			
HCM 2000 Volume to Capaci	ty ratio		0.62									
Actuated Cycle Length (s)			75.0		um of los	. ,			9.0			
Intersection Capacity Utilizati	on		83.1%	IC	U Level	of Service	;		Е			
Analysis Period (min)			15									
c Critical Lane Group												

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			€Î}•			4Th	
Volume (vph)	30	5	10	25	5	15	15	575	15	5	915	25
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Lane Width	12	14	12	12	14	12	12	12	12	12	12	12
Total Lost time (s)		4.5			4.5			4.5			4.5	
Lane Util. Factor		1.00			1.00			0.95			0.95	
Frpb, ped/bikes		0.99			0.99			1.00			1.00	
Flpb, ped/bikes		0.98			0.99			1.00			1.00	
Frt		0.97			0.95			1.00			1.00	
Flt Protected		0.97			0.97			1.00			1.00	
Satd. Flow (prot)		1679			1700			3261			3262	
Flt Permitted		0.83			0.86			0.92			0.95	
Satd. Flow (perm)		1441			1502			3018			3109	
Peak-hour factor, PHF	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Adj. Flow (vph)	32	5	11	27	5	16	16	618	16	5	984	27
RTOR Reduction (vph)	0	8	0	0	12	0	0	2	0	0	2	0
Lane Group Flow (vph)	0	40	0	0	36	0	0	648	0	0	1014	0
Confl. Peds. (#/hr)	20		8	8		20	7		5	5		7
Confl. Bikes (#/hr)			6			7			17			17
Heavy Vehicles (%)	3%	0%	0%	0%	0%	0%	0%	1%	0%	0%	1%	0%
Bus Blockages (#/hr)	0	0	2	0	0	2	0	2	0	0	2	0
Turn Type	Perm	NA		Perm	NA		Perm	NA		Perm	NA	
Protected Phases		2			6			4			8	
Permitted Phases	2			6			4			8		
Actuated Green, G (s)		18.0			18.0			48.0			48.0	
Effective Green, g (s)		18.0			18.0			48.0			48.0	
Actuated g/C Ratio		0.24			0.24			0.64			0.64	
Clearance Time (s)		4.5			4.5			4.5			4.5	
Vehicle Extension (s)		3.0			3.0			2.0			2.0	
Lane Grp Cap (vph)		345			360			1931			1989	
v/s Ratio Prot		0.00			0.00			0.04			0.00	
v/s Ratio Perm		c0.03			0.02			0.21			c0.33	
v/c Ratio		0.11			0.10			0.34			0.51	
Uniform Delay, d1		22.3			22.2			6.2			7.2	
Progression Factor		1.00			1.00			2.07			1.23	
Incremental Delay, d2		0.1			0.1 22.3			0.4			0.8	
Delay (s)		22.4 C			22.3 C			13.3			9.7	
Level of Service		22.4			22.3			B 13.3			A 9.7	
Approach Delay (s) Approach LOS		22.4 C			22.3 C			13.3 B			9.7 A	
•		C			C			ь			А	
Intersection Summary												
HCM 2000 Control Delay			11.7	H	CM 2000	Level of S	Service		В			
HCM 2000 Volume to Capacit	ty ratio		0.40									
Actuated Cycle Length (s)			75.0		um of lost				9.0			
Intersection Capacity Utilization	on		54.6%	IC	U Level o	of Service	;		Α			
Analysis Period (min)			15									
c Critical Lane Group												

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	f)		ň	1>			4îÞ			4TÞ	
Volume (vph)	30	40	50	130	95	35	30	555	35	25	825	60
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Total Lost time (s)	4.5	4.5		4.5	4.5			4.5			4.5	
Lane Util. Factor	1.00	1.00		1.00	1.00			0.95			0.95	
Frpb, ped/bikes	1.00	0.98		1.00	0.99			1.00			1.00	
Flpb, ped/bikes	0.97	1.00		0.98	1.00			1.00			1.00	
Frt	1.00	0.92		1.00	0.96			0.99			0.99	
Flt Protected	0.95	1.00		0.95	1.00			1.00			1.00	
Satd. Flow (prot)	1614	1551		1621	1621			3002			3028	
Flt Permitted	0.67	1.00		0.70	1.00			0.89			0.93	
Satd. Flow (perm)	1138	1551		1189	1621			2664			2816	
Peak-hour factor, PHF	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Adj. Flow (vph)	31	41	52	134	98	36	31	572	36	26	851	62
RTOR Reduction (vph)	0	39	0	0	23	0	0	4	0	0	5	0
Lane Group Flow (vph)	31	54	0	134	111	0	0	635	0	0	934	0
Confl. Peds. (#/hr)	28		14	14		28	9		14	14		9
Confl. Bikes (#/hr)			7			4			15			13
Heavy Vehicles (%)	0%	3%	0%	1%	3%	0%	0%	2%	3%	5%	1%	0%
Bus Blockages (#/hr)	0	0	2	0	0	2	0	2	0	0	2	0
Parking (#/hr)							5	5	5	5	5	5
Turn Type	Perm	NA		Perm	NA		Perm	NA		Perm	NA	
Protected Phases		2			6			4			8	
Permitted Phases	2			6			4			8		
Actuated Green, G (s)	18.6	18.6		18.6	18.6			47.4			47.4	
Effective Green, g (s)	18.6	18.6		18.6	18.6			47.4			47.4	
Actuated g/C Ratio	0.25	0.25		0.25	0.25			0.63			0.63	
Clearance Time (s)	4.5	4.5		4.5	4.5			4.5			4.5	
Vehicle Extension (s)	3.0	3.0		3.0	3.0			2.0			2.0	
Lane Grp Cap (vph)	282	384		294	402			1683			1779	
v/s Ratio Prot		0.03			0.07							
v/s Ratio Perm	0.03			c0.11				0.24			c0.33	
v/c Ratio	0.11	0.14		0.46	0.28			0.38			0.53	
Uniform Delay, d1	21.8	22.0		23.9	22.8			6.7			7.6	
Progression Factor	1.00	1.00		1.00	1.00			1.00			0.56	
Incremental Delay, d2	0.2	0.2		1.1	0.4			0.6			1.0	
Delay (s)	22.0	22.1		25.0	23.1			7.3			5.2	
Level of Service	С	С		С	С			Α			А	
Approach Delay (s)		22.1			24.1			7.3			5.2	
Approach LOS		С			С			Α			А	
Intersection Summary												
HCM 2000 Control Delay			9.5	H	CM 2000	Level of :	Service		А			
HCM 2000 Volume to Capa	city ratio		0.51									
Actuated Cycle Length (s)			75.0	Sı	um of lost	time (s)			9.0			
Intersection Capacity Utiliza	ition		81.2%		U Level o		:		D			
Analysis Period (min)			15									
c Critical Lane Group												

DKS Associates

6/2/2013

Synchro 8 - Report
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	٦	•	1	†	+	4
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	**	7		41∱	∱ ⊅	
Volume (veh/h)	125	155	35	470	765	170
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97
Hourly flow rate (vph)	129	160	36	485	789	175
Pedestrians	29			3	3	
Lane Width (ft)	12.0			12.0	12.0	
Walking Speed (ft/s)	4.0			4.0	4.0	
Percent Blockage	2			0	0	
Right turn flare (veh)		8				
Median type				None	None	
Median storage veh)						
Upstream signal (ft)				309	705	
pX, platoon unblocked	0.96	0.97	0.97			
vC, conflicting volume	1223	514	993			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	989	426	922			
tC, single (s)	6.8	6.9	4.2			
tC, 2 stage (s)						
tF (s)	3.5	3.3	2.2			
p0 queue free %	41	71	95			
cM capacity (veh/h)	217	545	688			
Direction, Lane #	EB 1	NB 1	NB 2	SB 1	SB 2	
Volume Total	289	198	323	526	438	
Volume Left	129	36	0	0	0	
Volume Right	160	0	0	0	175	
cSH	487	688	1700	1700	1700	
Volume to Capacity	0.59	0.05	0.19	0.31	0.26	
Queue Length 95th (ft)	95	4	0	0	0	
Control Delay (s)	27.2	2.4	0.0	0.0	0.0	
Lane LOS	D	A	0.0	0.0	0.0	
Approach Delay (s)	27.2	0.9		0.0		
Approach LOS	D	0.7		0.0		
Intersection Summary						
Average Delay			4.7			
Intersection Capacity Utiliza	ation		57.7%	IC	CU Level o	of Sorvice
Analysis Period (min)	auOH		15	IC	O Level (J Service
Analysis Fenou (IIIII)			10			

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	۲	†	7	Ŋ	†	7	7	∱ ∱		ř	∱ }	
Volume (vph)	120	280	320	120	370	75	275	310	45	165	600	105
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Lane Width	12	10	10	10	10	11	11	10	11	12	12	12
Total Lost time (s)	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5		4.5	4.5	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95		1.00	0.95	
Frpb, ped/bikes	1.00	1.00	0.97	1.00	1.00	0.94	1.00	0.99		1.00	0.99	
Flpb, ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00		1.00	1.00	
Frt	1.00	1.00	0.85	1.00	1.00	0.85	1.00	0.98		1.00	0.98	
Flt Protected	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00		0.95	1.00	
Satd. Flow (prot)	1662	1601	1339	1492	1617	1324	1591	2927		1626	3180	
Flt Permitted	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00		0.95	1.00	
Satd. Flow (perm)	1662	1601	1339	1492	1617	1324	1591	2927		1626	3180	
Peak-hour factor, PHF	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Adj. Flow (vph)	126	295	337	126	389	79	289	326	47	174	632	111
RTOR Reduction (vph)	0	0	45	0	0	58	0	9	0	0	12	0
Lane Group Flow (vph)	126	295	292	126	389	21	289	364	0	174	731	0
Confl. Peds. (#/hr)	23		15	15		23	20		13	13		20
Confl. Bikes (#/hr)			25			17			5			14
Heavy Vehicles (%)	0%	2%	1%	4%	1%	2%	1%	3%	5%	1%	1%	0%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	5	3	0	0
Turn Type	Prot	NA	pm+ov	Prot	NA	Perm	Prot	NA		Prot	NA	
Protected Phases	5	2	3	1	6		3	8		7	4	
Permitted Phases			2			6						
Actuated Green, G (s)	10.8	28.5	51.6	11.7	29.4	29.4	23.1	36.3		15.5	28.7	
Effective Green, g (s)	10.8	28.5	51.6	11.7	29.4	29.4	23.1	36.3		15.5	28.7	
Actuated g/C Ratio	0.10	0.26	0.47	0.11	0.27	0.27	0.21	0.33		0.14	0.26	
Clearance Time (s)	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5		4.5	4.5	
Vehicle Extension (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0		2.0	2.0	
Lane Grp Cap (vph)	163	414	682	158	432	353	334	965		229	829	
v/s Ratio Prot	0.08	0.18	0.09	c0.08	c0.24		c0.18	0.12		0.11	c0.23	
v/s Ratio Perm			0.13			0.02						
v/c Ratio	0.77	0.71	0.43	0.80	0.90	0.06	0.87	0.38		0.76	0.88	
Uniform Delay, d1	48.4	37.0	19.4	48.0	38.9	30.0	41.9	28.2		45.5	39.0	
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00		1.00	1.00	
Incremental Delay, d2	18.5	4.8	0.2	22.3	21.0	0.0	19.6	0.1		12.1	10.6	
Delay (s)	66.9	41.8	19.6	70.3	59.9	30.0	61.5	28.3		57.5	49.6	
Level of Service	E	D	В	Е	E	С	E	C		E	D	
Approach Delay (s)		36.1			58.2			42.8			51.1	
Approach LOS		D			E			D			D	
Intersection Summary												
HCM 2000 Control Delay			46.8	Н	CM 2000	Level of	Service		D			
HCM 2000 Volume to Capac	city ratio		0.88									
Actuated Cycle Length (s)			110.0		um of los				18.0			
Intersection Capacity Utilizat	tion		81.8%	IC	CU Level	of Service	!		D			
Analysis Period (min)			15									
c Critical Lane Group												

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Movement	WBL2	WBL	WBR	NBL	NBT	NBR	SBL	SBT	SBR	NEL	NER	NER2
Lane Configurations		M			4			4	7	M		
Volume (vph)	5	10	20	5	245	5	30	335	620	370	5	5
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Lane Width	12	12	12	12	14	12	12	12	12	12	12	12
Total Lost time (s)		4.0			4.0			4.0	5.0	5.0		
Lane Util. Factor		1.00			1.00			1.00	1.00	1.00		
Frpb, ped/bikes		0.99			1.00			1.00	0.99	1.00		
Flpb, ped/bikes		1.00			1.00			1.00	1.00	1.00		
Frt		0.92			1.00			1.00	0.85	1.00		
Flt Protected		0.98			1.00			1.00	1.00	0.95		
Satd. Flow (prot)		1508			1824			1726	1452	1645		
Flt Permitted		0.98			0.99			0.96	1.00	1.00		
Satd. Flow (perm)		1508			1812			1666	1452	1725		
Peak-hour factor, PHF	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Adj. Flow (vph)	5	11	21	5	258	5	32	353	653	389	5	5
RTOR Reduction (vph)	0	28	0	0	1	0	0	0	0	11	0	0
Lane Group Flow (vph)	0	9	0	0	267	0	0	385	653	388	0	0
Confl. Peds. (#/hr)	Ū	,	1	2	201	8	8	000	2	1		
Confl. Bikes (#/hr)			2			2	U		4	•		4
Heavy Vehicles (%)	0%	0%	6%	0%	2%	0%	0%	1%	1%	1%	0%	0%
Turn Type	Perm	NA	070	Perm	NA	070	Perm	NA	custom	NA	070	070
Protected Phases	I CIIII	8		I CIIII	2		I CIIII	6	cusioni	INA		
Permitted Phases	8	U		2	Z		6	U	3 6	3		
Actuated Green, G (s)	U	10.0			16.3		U	16.3	39.8	14.5		
Effective Green, g (s)		10.0			16.3			16.3	35.8	14.5		
Actuated g/C Ratio		0.25			0.41			0.41	0.90	0.36		
Clearance Time (s)		4.0			4.0			4.0	0.70	5.0		
Vehicle Extension (s)		2.5			2.5			2.5		2.0		
		378			742			682	1306	628		
Lane Grp Cap (vph) v/s Ratio Prot		3/8			742			082	1300	028		
v/s Ratio Prot v/s Ratio Perm		0.01			0.15			0.23	c0.45	c0.22		
v/c Ratio		0.01			0.15			0.23	0.50	0.62		
		11.2			8.1			9.0				
Uniform Delay, d1									0.4	10.4		
Progression Factor		1.00			1.00			1.00	1.00	1.00		
Incremental Delay, d2		0.0			0.2			0.9	0.1	1.3		
Delay (s)		11.2			8.4			9.9	0.5	11.7		
Level of Service		B			Α			A	Α	B		
Approach LOS		11.2			8.4			4.0		11.7		
Approach LOS		В			А			А		В		
Intersection Summary												
HCM 2000 Control Delay			6.6	Н	CM 2000	Level of S	Service		Α			
HCM 2000 Volume to Capac	city ratio		0.64									
Actuated Cycle Length (s)			39.8		um of lost				11.0			
Intersection Capacity Utilizat	ion		73.7%	IC	CU Level of	of Service	!		D			
Analysis Period (min)			15									
c Critical Lane Group												

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4	7		ર્ન	7		4T)	
Volume (vph)	15	45	15	235	80	40	5	420	195	50	700	15
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Lane Width	12	14	12	12	12	12	12	12	12	12	12	12
Total Lost time (s)		4.5			4.5	4.5		4.5	4.5		4.5	
Lane Util. Factor		1.00			1.00	1.00		1.00	1.00		0.95	
Frpb, ped/bikes		0.99			1.00	0.94		1.00	0.95		1.00	
Flpb, ped/bikes		1.00			0.99	1.00		1.00	1.00		1.00	
Frt		0.97			1.00	0.85		1.00	0.85		1.00	
Flt Protected		0.99			0.96	1.00		1.00	1.00		1.00	
Satd. Flow (prot)		1779			1669	1403		1732	1373		3269	
Flt Permitted		0.92			0.73	1.00		0.99	1.00		0.89	
Satd. Flow (perm)		1648			1268	1403		1720	1373		2935	
Peak-hour factor, PHF	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Adj. Flow (vph)	16	49	16	258	88	44	5	462	214	55	769	16
RTOR Reduction (vph)	0	10	0	0	0	28	0	0	104	0	1	0
Lane Group Flow (vph)	0	71	0	0	346	16	0	467	110	0	839	0
Confl. Peds. (#/hr)	15		10	10		15	20		10	10		20
Confl. Bikes (#/hr)			26			6			17			11
Heavy Vehicles (%)	0%	0%	0%	0%	1%	0%	0%	1%	1%	0%	1%	0%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	4	0	0	0
Turn Type	Perm	NA		Perm	NA	Perm	Perm	NA	Perm	Perm	NA	
Protected Phases		2			6			8			4	
Permitted Phases	2			6		6	8		8	4		
Actuated Green, G (s)		27.5			27.5	27.5		38.5	38.5		38.5	
Effective Green, g (s)		27.5			27.5	27.5		38.5	38.5		38.5	
Actuated g/C Ratio		0.37			0.37	0.37		0.51	0.51		0.51	
Clearance Time (s)		4.5			4.5	4.5		4.5	4.5		4.5	
Vehicle Extension (s)		3.0			3.0	3.0		2.0	2.0		2.0	
Lane Grp Cap (vph)		604			464	514		882	704		1506	
v/s Ratio Prot		0.04			0.07	0.04		0.07	0.00		0.00	
v/s Ratio Perm		0.04			c0.27	0.01		0.27	0.08		c0.29	
v/c Ratio		0.12			0.75	0.03		0.53	0.16		0.56	
Uniform Delay, d1		15.7			20.7	15.2		12.2	9.7		12.4	
Progression Factor		1.00			1.00	1.00		0.37	0.04		1.00	
Incremental Delay, d2		0.1			6.4	0.0		2.2	0.5		1.5	
Delay (s)		15.8			27.1	15.2		6.7	8.0		13.9	
Level of Service		B 15.8			C 25.8	В		A 4.8	А		B 13.9	
Approach LOS					25.8 C						13.9 B	
Approach LOS		В			C			А			D	
Intersection Summary												
HCM 2000 Control Delay			13.2	Н	CM 2000	Level of	Service		В			
HCM 2000 Volume to Capac	ity ratio		0.63									
Actuated Cycle Length (s)			75.0		um of los				9.0			
Intersection Capacity Utilizati	on		84.1%	IC	U Level	of Service)		Е			
Analysis Period (min)			15									
c Critical Lane Group												

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			€Î}•			414	
Volume (vph)	30	5	10	25	5	15	15	575	15	5	915	25
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Lane Width	12	14	12	12	14	12	12	12	12	12	12	12
Total Lost time (s)		4.5			4.5			4.5			4.5	
Lane Util. Factor		1.00			1.00			0.95			0.95	
Frpb, ped/bikes		0.99			0.98			1.00			1.00	
Flpb, ped/bikes		0.97			0.99			1.00			1.00	
Frt		0.97			0.95			1.00			1.00	
Flt Protected		0.97			0.97			1.00			1.00	
Satd. Flow (prot)		1649			1678			3247			3248	
Flt Permitted		0.83			0.86			0.92			0.95	
Satd. Flow (perm)		1416			1482			3005			3095	
Peak-hour factor, PHF	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Adj. Flow (vph)	32	5	11	27	5	16	16	618	16	5	984	27
RTOR Reduction (vph)	0	8	0	0	12	0	0	2	0	0	2	0
Lane Group Flow (vph)	0	40	0	0	36	0	0	648	0	0	1014	0
Confl. Peds. (#/hr)	40		15	15		40	15		10	10		15
Confl. Bikes (#/hr)			6			7			17			17
Heavy Vehicles (%)	3%	0%	0%	0%	0%	0%	0%	1%	0%	0%	1%	0%
Bus Blockages (#/hr)	0	0	4	0	0	4	0	4	0	0	4	0
Turn Type	Perm	NA		Perm	NA		Perm	NA		Perm	NA	
Protected Phases		2			6			4			8	
Permitted Phases	2			6			4			8		
Actuated Green, G (s)		18.0			18.0			48.0			48.0	
Effective Green, g (s)		18.0			18.0			48.0			48.0	
Actuated g/C Ratio		0.24			0.24			0.64			0.64	
Clearance Time (s)		4.5			4.5			4.5			4.5	
Vehicle Extension (s)		3.0			3.0			2.0			2.0	
Lane Grp Cap (vph)		339			355			1923			1980	
v/s Ratio Prot												
v/s Ratio Perm		c0.03			0.02			0.22			c0.33	
v/c Ratio		0.12			0.10			0.34			0.51	
Uniform Delay, d1		22.3			22.2			6.2			7.2	
Progression Factor		1.00			1.00			1.72			1.45	
Incremental Delay, d2		0.2			0.1			0.5			0.8	
Delay (s)		22.4			22.3			11.1			11.3	
Level of Service		C			С			В			В	
Approach Delay (s)		22.4			22.3			11.1			11.3	
Approach LOS		С			С			В			В	
Intersection Summary												
HCM 2000 Control Delay			11.8	H	CM 2000	Level of S	Service		В			
HCM 2000 Volume to Capacit	y ratio		0.40									
Actuated Cycle Length (s)			75.0	Sı	um of lost	time (s)			9.0			
Intersection Capacity Utilizatio	n		54.7%		:U Level o	• •	:		А			
Analysis Period (min)			15									
c Critical Lane Group												

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ň	f)		¥	1>			€ÎÞ			4Th	
Volume (vph)	30	40	50	130	95	35	30	555	35	25	825	60
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Total Lost time (s)	4.5	4.5		4.5	4.5			4.5			4.5	
Lane Util. Factor	1.00	1.00		1.00	1.00			0.95			0.95	
Frpb, ped/bikes	1.00	0.97		1.00	0.98			0.99			1.00	
Flpb, ped/bikes	0.95	1.00		0.97	1.00			1.00			1.00	
Frt	1.00	0.92		1.00	0.96			0.99			0.99	
Flt Protected	0.95	1.00		0.95	1.00			1.00			1.00	
Satd. Flow (prot)	1576	1534		1592	1610			2983			3010	
Flt Permitted	0.67	1.00		0.70	1.00			0.89			0.93	
Satd. Flow (perm)	1112	1534		1167	1610			2647			2799	
Peak-hour factor, PHF	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Adj. Flow (vph)	31	41	52	134	98	36	31	572	36	26	851	62
RTOR Reduction (vph)	0	39	0	0	23	0	0	4	0	0	5	0
Lane Group Flow (vph)	31	54	0	134	111	0	0	635	0	0	934	0
Confl. Peds. (#/hr)	50		30	30		50	20		30	30		20
Confl. Bikes (#/hr)			7			4			15			13
Heavy Vehicles (%)	0%	3%	0%	1%	3%	0%	0%	2%	3%	5%	1%	0%
Bus Blockages (#/hr)	0	0	4	0	0	4	0	4	0	0	4	0
Parking (#/hr)							5	5	5	5	5	5
Turn Type	Perm	NA		Perm	NA		Perm	NA		Perm	NA	
Protected Phases		2			6			4			8	
Permitted Phases	2			6			4			8		
Actuated Green, G (s)	18.7	18.7		18.7	18.7			47.3			47.3	
Effective Green, g (s)	18.7	18.7		18.7	18.7			47.3			47.3	
Actuated g/C Ratio	0.25	0.25		0.25	0.25			0.63			0.63	
Clearance Time (s)	4.5	4.5		4.5	4.5			4.5			4.5	
Vehicle Extension (s)	3.0	3.0		3.0	3.0			2.0			2.0	
Lane Grp Cap (vph)	277	382		290	401			1669			1765	
v/s Ratio Prot		0.04			0.07							
v/s Ratio Perm	0.03			c0.11				0.24			c0.33	
v/c Ratio	0.11	0.14		0.46	0.28			0.38			0.53	
Uniform Delay, d1	21.7	21.9		23.9	22.7			6.7			7.7	
Progression Factor	1.00	1.00		1.00	1.00			1.00			0.86	
Incremental Delay, d2	0.2	0.2		1.2	0.4			0.7			1.0	
Delay (s)	21.9	22.1		25.1	23.1			7.4			7.6	
Level of Service	С	С		С	С			Α			А	
Approach Delay (s)		22.0			24.1			7.4			7.6	
Approach LOS		С			С			Α			Α	
Intersection Summary												
HCM 2000 Control Delay			10.7	H	CM 2000	Level of	Service		В			
HCM 2000 Volume to Capa	city ratio		0.51									
Actuated Cycle Length (s)			75.0	Sı	um of lost	time (s)			9.0			
Intersection Capacity Utiliza	tion		81.3%	IC	U Level o	of Service)		D			
Analysis Period (min)			15									
c Critical Lane Group												

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Movement	EBL	EBR	NBL	NBT	SBT	SBR		
Lane Configurations	<u>ነ</u>	7		44	↑ 1>			
Volume (vph)	125	155	35	470	765	170		
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750		
Total Lost time (s)	4.0	4.0		4.0	4.0			
Lane Util. Factor	1.00	1.00		0.95	0.95			
Frpb, ped/bikes	1.00	0.95		1.00	0.96			
Flpb, ped/bikes	1.00	1.00		1.00	1.00			
Frt	1.00	0.85		1.00	0.97			
Flt Protected	0.95	1.00		1.00	1.00			
Satd. Flow (prot)	1646	1403		3213	3082			
Flt Permitted	0.95	1.00		0.83	1.00			
Satd. Flow (perm)	1646	1403		2692	3082			
Peak-hour factor, PHF	0.97	0.97	0.97	0.97	0.97	0.97		
Adj. Flow (vph)	129	160	36	485	789	175		
RTOR Reduction (vph)	0	117	0	0	16	0		
Lane Group Flow (vph)	129	43	0	521	948	0		
Confl. Peds. (#/hr)	15	15	50	JZI	740	50		
Confl. Bikes (#/hr)	10	3	50			9		
Heavy Vehicles (%)	1%	1%	3%	2%	0%	1%		
Bus Blockages (#/hr)	0	0	0	4	4	0		
						U		
Turn Type	NA	Perm	Perm	NA	NA			
Protected Phases	2	2	0	8	4			
Permitted Phases	22.0	2	8	00.0	00.0			
Actuated Green, G (s)	32.0	32.0		80.0	80.0			
Effective Green, g (s)	32.0	32.0		80.0	80.0			
Actuated g/C Ratio	0.27	0.27		0.67	0.67			
Clearance Time (s)	4.0	4.0		4.0	4.0			
Vehicle Extension (s)	3.0	3.0		3.0	3.0			
Lane Grp Cap (vph)	438	374		1794	2054			
v/s Ratio Prot	c0.08				c0.31			
v/s Ratio Perm		0.03		0.19				
v/c Ratio	0.29	0.11		0.29	0.46			
Uniform Delay, d1	35.0	33.3		8.3	9.6			
Progression Factor	1.00	1.00		0.19	1.00			
Incremental Delay, d2	1.7	0.6		0.4	0.7			
Delay (s)	36.7	33.9		2.0	10.4			
Level of Service	D	С		А	В			
Approach Delay (s)	35.2			2.0	10.4			
Approach LOS	D			Α	В			
Intersection Summary								
HCM 2000 Control Delay			12.0	Н	CM 2000	Level of Service	В	
HCM 2000 Volume to Cap	acity ratio		0.41					
Actuated Cycle Length (s)			120.0	Sı	um of lost	time (s)	8.0	
Intersection Capacity Utiliz	zation		63.0%		CU Level c		В	
Analysis Period (min)			15					
c Critical Lane Group								

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	7	†	7	ř	†	7	7	∱ Љ		ሻ	ተ ኈ	
Volume (vph)	120	280	320	120	370	75	275	310	45	165	600	105
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Lane Width	12	10	10	10	10	11	11	10	11	12	12	12
Total Lost time (s)	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5		4.5	4.5	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95		1.00	0.95	
Frpb, ped/bikes	1.00	1.00	0.96	1.00	1.00	0.90	1.00	0.99		1.00	0.98	
Flpb, ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00		1.00	1.00	
Frt	1.00	1.00	0.85	1.00	1.00	0.85	1.00	0.98		1.00	0.98	
Flt Protected	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00		0.95	1.00	
Satd. Flow (prot)	1662	1601	1318	1492	1617	1269	1591	2913		1607	3151	
Flt Permitted	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00		0.95	1.00	
Satd. Flow (perm)	1662	1601	1318	1492	1617	1269	1591	2913		1607	3151	
Peak-hour factor, PHF	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Adj. Flow (vph)	126	295	337	126	389	79	289	326	47	174	632	111
RTOR Reduction (vph)	0	0	89	0	0	58	0	8	0	0	11	0
Lane Group Flow (vph)	126	295	248	126	389	21	289	365	0	174	732	0
Confl. Peds. (#/hr)	45		30	30		45	40		25	25		40
Confl. Bikes (#/hr)			25			17			5			14
Heavy Vehicles (%)	0%	2%	1%	4%	1%	2%	1%	3%	5%	1%	1%	0%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	10	6	0	0
Turn Type	Prot	NA	pm+ov	Prot	NA	Perm	Prot	NA		Prot	NA	
Protected Phases	5	2	3	1	6		3	8		7	4	
Permitted Phases			2			6						
Actuated Green, G (s)	13.3	31.4	56.0	14.0	32.1	32.1	24.6	39.2		17.4	32.0	
Effective Green, g (s)	13.3	31.4	56.0	14.0	32.1	32.1	24.6	39.2		17.4	32.0	
Actuated g/C Ratio	0.11	0.26	0.47	0.12	0.27	0.27	0.21	0.33		0.14	0.27	
Clearance Time (s)	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5		4.5	4.5	
Vehicle Extension (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0		2.0	2.0	
Lane Grp Cap (vph)	184	418	664	174	432	339	326	951		233	840	
v/s Ratio Prot	0.08	0.18	0.08	c0.08	c0.24	0.00	c0.18	0.13		0.11	c0.23	
v/s Ratio Perm	0.70	0.71	0.11	0.70	0.00	0.02	0.00	0.20		0.75	0.07	
v/c Ratio	0.68	0.71	0.37	0.72	0.90	0.06	0.89	0.38		0.75	0.87	
Uniform Delay, d1	51.3	40.1	20.7	51.1	42.4	32.7	46.3	31.1		49.2	42.0	
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00		1.33	0.83	
Incremental Delay, d2	8.1	4.4	0.1	11.9	21.0	0.0	23.2	1.2		9.9	11.1	
Delay (s)	59.5	44.5	20.8 C	63.0	63.4	32.8 C	69.6 E	32.3 C		75.4	45.9	
Level of Service Approach Delay (s)	Е	D 24 E	C	E	E 59.3	C	E	48.6		E	D 51.5	
7		36.5 D			59.3 E						51.5 D	
Approach LOS		D			E			D			D	
Intersection Summary												
HCM 2000 Control Delay			48.5	Н	CM 2000	Level of	Service		D			
HCM 2000 Volume to Capac	ity ratio		0.87									
Actuated Cycle Length (s)			120.0		um of lost				18.0			
Intersection Capacity Utilizati	on		82.0%	IC	CU Level	of Service)		Ε			
Analysis Period (min)			15									
c Critical Lane Group												

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Movement	WBL2	WBL	WBR	NBL	NBT	NBR	SBL	SBT	SBR	NEL	NER	NER2
Lane Configurations		M			4			4	7	M		
Volume (vph)	5	10	20	5	245	5	30	335	620	370	5	5
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Lane Width	12	12	12	12	14	12	12	12	12	12	12	12
Total Lost time (s)		4.0			4.0			4.0	5.0	5.0		
Lane Util. Factor		1.00			1.00			1.00	1.00	1.00		
Frpb, ped/bikes		0.98			1.00			1.00	0.98	1.00		
Flpb, ped/bikes		1.00			1.00			1.00	1.00	1.00		
Frt		0.92			1.00			1.00	0.85	1.00		
Flt Protected		0.98			1.00			1.00	1.00	0.95		
Satd. Flow (prot)		1495			1824			1725	1449	1642		
Flt Permitted		0.98			0.99			0.96	1.00	1.00		
Satd. Flow (perm)		1495			1812			1665	1449	1722		
Peak-hour factor, PHF	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Adj. Flow (vph)	5	11	21	5	258	5	32	353	653	389	5	5
RTOR Reduction (vph)	0	28	0	0	1	0	0	0	0	11	0	0
Lane Group Flow (vph)	0	9	0	0	267	0	0	385	653	388	0	0
Confl. Peds. (#/hr)	5		5	5		15	15		5	5		5
Confl. Bikes (#/hr)			2			2			4			4
Heavy Vehicles (%)	0%	0%	6%	0%	2%	0%	0%	1%	1%	1%	0%	0%
Turn Type	Perm	NA	0.0	Perm	NA		Perm	NA	custom	NA		
Protected Phases	1 01111	8		1 01111	2		1 01111	6	ouston			
Permitted Phases	8	J		2	_		6	J	3 6	3		
Actuated Green, G (s)		10.2		_	16.5			16.5	40.2	14.7		
Effective Green, g (s)		10.2			16.5			16.5	36.2	14.7		
Actuated g/C Ratio		0.25			0.41			0.41	0.90	0.37		
Clearance Time (s)		4.0			4.0			4.0	0.70	5.0		
Vehicle Extension (s)		2.5			2.5			2.5		2.0		
Lane Grp Cap (vph)		379			743			683	1304	629		
v/s Ratio Prot		377			7 13			003	1304	027		
v/s Ratio Perm		0.01			0.15			0.23	c0.45	c0.23		
v/c Ratio		0.02			0.36			0.56	0.50	0.62		
Uniform Delay, d1		11.3			8.2			9.1	0.4	10.4		
Progression Factor		1.00			1.00			1.00	1.00	1.00		
Incremental Delay, d2		0.0			0.2			0.9	0.1	1.3		
Delay (s)		11.3			8.4			10.0	0.5	11.7		
Level of Service		В			A			Α	A	В		
Approach Delay (s)		11.3			8.4			4.0	, , , , , , , , , , , , , , , , , , ,	11.7		
Approach LOS		В			A			A		В		
Intersection Summary												
HCM 2000 Control Delay			6.6	Н	CM 2000	Level of	Service	-	Α			
HCM 2000 Volume to Capaci	ty ratio		0.64									
Actuated Cycle Length (s)			40.2	S	um of lost	time (s)			11.0			
Intersection Capacity Utilization	on		73.7%	IC	CU Level o	of Service)		D			
Analysis Period (min)			15									
c Critical Lane Group												

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ň	†	7	7	†	7	ሻ	f÷		ሻ	4î	
Volume (vph)	120	280	330	120	370	70	275	310	45	155	575	105
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Lane Width	12	10	10	10	10	11	11	10	11	12	12	12
Total Lost time (s)	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5		4.5	4.5	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00		1.00	1.00	
Frpb, ped/bikes	1.00	1.00	0.91	1.00	1.00	0.83	1.00	0.99		1.00	0.98	
Flpb, ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00		1.00	1.00	
Frt	1.00	1.00	0.85	1.00	1.00	0.85	1.00	0.98		1.00	0.98	
Flt Protected	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00		0.95	1.00	
Satd. Flow (prot)	1662	1601	1254	1492	1617	1170	1591	1534		1607	1657	
Flt Permitted	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00		0.95	1.00	
Satd. Flow (perm)	1662	1601	1254	1492	1617	1170	1591	1534		1607	1657	
Peak-hour factor, PHF	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Adj. Flow (vph)	126	295	347	126	389	74	289	326	47	163	605	111
RTOR Reduction (vph)	0	0	198	0	0	57	0	4	0	0	5	0
Lane Group Flow (vph)	126	295	149	126	389	17	289	369	0	163	711	0
Confl. Peds. (#/hr)	45		30	30		45	40		25	25		40
Confl. Bikes (#/hr)			25			17			5			14
Heavy Vehicles (%)	0%	2%	1%	4%	1%	2%	1%	3%	5%	1%	1%	0%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	10	6	0	0
Turn Type	Prot	NA	Perm	Prot	NA	Perm	Prot	NA		Prot	NA	
Protected Phases	5	2		1	6		3	8		7	4	
Permitted Phases			2			6						
Actuated Green, G (s)	7.5	24.3	24.3	10.5	27.3	27.3	20.5	51.2		16.0	46.7	
Effective Green, g (s)	7.5	24.3	24.3	10.5	27.3	27.3	20.5	51.2		16.0	46.7	
Actuated g/C Ratio	0.06	0.20	0.20	0.09	0.23	0.23	0.17	0.43		0.13	0.39	
Clearance Time (s)	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5		4.5	4.5	
Vehicle Extension (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0		2.0	2.0	
Lane Grp Cap (vph)	103	324	253	130	367	266	271	654		214	644	
v/s Ratio Prot	0.08	0.18		c0.08	c0.24		c0.18	c0.24		0.10	c0.43	
v/s Ratio Perm	4.00	0.01	0.12	0.07	101	0.01	4.07	0.57		0.77	4.40	
v/c Ratio	1.22	0.91	0.59	0.97	1.06	0.06	1.07	0.56		0.76	1.10	
Uniform Delay, d1	56.2	46.8	43.3	54.6	46.4	36.3	49.8	26.0		50.2	36.6	
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00		0.87	1.12	
Incremental Delay, d2	160.4	28.0	2.3	68.3	63.7	0.0	73.3	3.5		7.4	58.8	
Delay (s)	216.7	74.7	45.6	122.8	110.1	36.4	123.1	29.5		51.2	99.7	
Level of Service	F	E	D	F	F	D	F	C 70.2		D	F	
Approach LOS		84.9			103.5			70.3			90.7	
Approach LOS		F			F			E			F	
Intersection Summary												
HCM 2000 Control Delay			87.1	Н	CM 2000	Level of	Service		F			
HCM 2000 Volume to Capac	city ratio		1.08									
Actuated Cycle Length (s)			120.0	, ,					18.0			
Intersection Capacity Utilizat	tion		100.2%	IC	CU Level	of Service)		G			
Analysis Period (min)			15									
c Critical Lane Group												

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4	7	7	f)		ř	4î	
Volume (vph)	15	45	15	220	80	40	5	410	190	55	690	15
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Lane Width	12	14	12	12	12	12	12	12	12	12	12	12
Total Lost time (s)		4.5			4.5	4.5	4.5	4.5		4.5	4.5	
Lane Util. Factor		1.00			1.00	1.00	1.00	1.00		1.00	1.00	
Frpb, ped/bikes		0.99			1.00	0.94	1.00	0.98		1.00	1.00	
Flpb, ped/bikes		1.00			0.99	1.00	1.00	1.00		0.99	1.00	
Frt		0.97			1.00	0.85	1.00	0.95		1.00	1.00	
Flt Protected		0.99			0.96	1.00	0.95	1.00		0.95	1.00	
Satd. Flow (prot)		1768			1661	1402	1662	1624		1654	1725	
Flt Permitted		0.92			0.76	1.00	0.20	1.00		0.28	1.00	
Satd. Flow (perm)		1634			1310	1402	347	1624		480	1725	
Peak-hour factor, PHF	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Adj. Flow (vph)	16	49	16	242	88	44	5	451	209	60	758	16
RTOR Reduction (vph)	0	11	0	0	0	30	0	20	0	0	1	0
Lane Group Flow (vph)	0	70	0	0	330	14	5	640	0	60	773	0
Confl. Peds. (#/hr)	15		10	10		15	20		10	10		20
Confl. Bikes (#/hr)			26			6			17			11
Heavy Vehicles (%)	0%	0%	0%	0%	1%	0%	0%	1%	1%	0%	1%	0%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	4	0	0	0
Turn Type	Perm	NA		Perm	NA	Perm	Perm	NA		Perm	NA	
Protected Phases		2			6			8			4	
Permitted Phases	2			6		6	8			4		
Actuated Green, G (s)		23.4			23.4	23.4	42.6	42.6		42.6	42.6	
Effective Green, g (s)		23.4			23.4	23.4	42.6	42.6		42.6	42.6	
Actuated g/C Ratio		0.31			0.31	0.31	0.57	0.57		0.57	0.57	
Clearance Time (s)		4.5			4.5	4.5	4.5	4.5		4.5	4.5	
Vehicle Extension (s)		3.0			3.0	3.0	2.0	2.0		2.0	2.0	
Lane Grp Cap (vph)		509			408	437	197	922		272	979	
v/s Ratio Prot		0.04			0.05	0.04	0.01	0.39		0.40	c0.45	
v/s Ratio Perm		0.04			c0.25	0.01	0.01	0.70		0.13	0.70	
v/c Ratio		0.14			0.81	0.03	0.03	0.69		0.22	0.79	
Uniform Delay, d1		18.5			23.7	17.9	7.1	11.6		8.0	12.7	
Progression Factor		1.00			1.00	1.00	1.63	1.57		1.00	1.00	
Incremental Delay, d2		0.1			11.2	0.0	0.2	3.7		1.9	6.5	
Delay (s)		18.7			35.0	18.0	11.8	21.9		9.9	19.2	
Level of Service		B 18.7			C 33.0	В	В	C 21.8		А	B 18.5	
Approach LOS					33.0 C			21.8 C			16.5 B	
Approach LOS		В			C			C			D	
Intersection Summary												
HCM 2000 Control Delay			22.4	Н	CM 2000	Level of	Service		С			
HCM 2000 Volume to Capaci	ty ratio		0.80									
Actuated Cycle Length (s)			75.0		um of los	٠,			9.0			
Intersection Capacity Utilizati	on		81.6%	IC	U Level	of Service)		D			
Analysis Period (min)			15									
c Critical Lane Group												

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4		7	f)		ሻ	4	
Volume (vph)	30	5	10	25	5	15	15	560	15	5	895	25
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Lane Width	12	14	12	12	14	12	12	12	12	12	12	12
Total Lost time (s)		4.5			4.5		4.5	4.5		4.5	4.5	
Lane Util. Factor		1.00			1.00		1.00	1.00		1.00	1.00	
Frpb, ped/bikes		0.99			0.96		1.00	1.00		1.00	1.00	
Flpb, ped/bikes		0.95			0.98		1.00	1.00		0.99	1.00	
Frt		0.97			0.95		1.00	1.00		1.00	1.00	
Flt Protected		0.97			0.97		0.95	1.00		0.95	1.00	
Satd. Flow (prot)		1605			1642		1662	1697		1652	1696	
Flt Permitted		0.83			0.86		0.12	1.00		0.34	1.00	
Satd. Flow (perm)		1378			1450		217	1697		599	1696	
Peak-hour factor, PHF	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Adj. Flow (vph)	32	5	11	27	5	16	16	602	16	5	962	27
RTOR Reduction (vph)	0	8	0	0	12	0	0	1	0	0	1	0
Lane Group Flow (vph)	0	40	0	0	36	0	16	617	0	5	988	0
Confl. Peds. (#/hr)	40		15	15		40	15		10	10		15
Confl. Bikes (#/hr)			6			7			17			17
Heavy Vehicles (%)	3%	0%	0%	0%	0%	0%	0%	1%	0%	0%	1%	0%
Bus Blockages (#/hr)	0	0	4	0	0	4	0	4	0	0	4	0
Turn Type	Perm	NA		Perm	NA		Perm	NA		Perm	NA	
Protected Phases		2			6			4			8	
Permitted Phases	2			6			4			8		
Actuated Green, G (s)		18.0			18.0		48.0	48.0		48.0	48.0	
Effective Green, g (s)		18.0			18.0		48.0	48.0		48.0	48.0	
Actuated g/C Ratio		0.24			0.24		0.64	0.64		0.64	0.64	
Clearance Time (s)		4.5			4.5		4.5	4.5		4.5	4.5	
Vehicle Extension (s)		3.0			3.0		2.0	2.0		2.0	2.0	
Lane Grp Cap (vph)		330			348		138	1086		383	1085	
v/s Ratio Prot								0.36			c0.58	
v/s Ratio Perm		c0.03			0.02		0.07			0.01		
v/c Ratio		0.12			0.10		0.12	0.57		0.01	0.91	
Uniform Delay, d1		22.3			22.2		5.2	7.6		4.9	11.6	
Progression Factor		1.00			1.00		1.91	1.97		0.91	0.76	
Incremental Delay, d2		0.2			0.1		1.4	1.8		0.0	8.7	
Delay (s)		22.5			22.3		11.4	16.8		4.5	17.5	
Level of Service		C			C		В	В		Α	B	
Approach Delay (s)		22.5			22.3			16.7			17.5	
Approach LOS		С			С			В			В	
Intersection Summary												
HCM 2000 Control Delay			17.4	H	CM 2000	Level of	Service		В			
HCM 2000 Volume to Capaci	ty ratio		0.69									
Actuated Cycle Length (s)	75.0			Sı	um of lost	t time (s)			9.0			
Intersection Capacity Utilization	on				CU Level	of Service)		D			
Analysis Period (min)			15									
c Critical Lane Group												

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ň	f)		ň	1>		ř	f)		ř	f)	
Volume (vph)	30	50	45	125	100	35	30	540	40	30	810	55
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Total Lost time (s)	4.5	4.5		4.5	4.5		4.5	4.5		4.5	4.5	
Lane Util. Factor	1.00	1.00		1.00	1.00		1.00	1.00		1.00	1.00	
Frpb, ped/bikes	1.00	0.96		1.00	0.97		1.00	0.99		1.00	1.00	
Flpb, ped/bikes	0.91	1.00		0.95	1.00		1.00	1.00		0.98	1.00	
Frt	1.00	0.93		1.00	0.96		1.00	0.99		1.00	0.99	
Flt Protected	0.95	1.00		0.95	1.00		0.95	1.00		0.95	1.00	
Satd. Flow (prot)	1521	1533		1556	1591		1455	1452		1357	1472	
Flt Permitted	0.66	1.00		0.69	1.00		0.18	1.00		0.36	1.00	
Satd. Flow (perm)	1054	1533		1136	1591		276	1452		510	1472	
Peak-hour factor, PHF	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Adj. Flow (vph)	31	52	46	129	103	36	31	557	41	31	835	57
RTOR Reduction (vph)	0	35	0	0	17	0	0	4	0	0	3	0
Lane Group Flow (vph)	31	63	0	129	122	0	31	594	0	31	889	0
Confl. Peds. (#/hr)	50		30	30		50	20		30	30		20
Confl. Bikes (#/hr)			7			4			15			13
Heavy Vehicles (%)	0%	3%	0%	1%	3%	0%	0%	2%	3%	5%	1%	0%
Bus Blockages (#/hr)	0	0	4	0	0	4	0	4	0	0	4	0
Parking (#/hr)							5	5	5	5	5	5
Turn Type	Perm	NA		Perm	NA		Perm	NA		Perm	NA	
Protected Phases		2			6			4			8	
Permitted Phases	2			6			4			8		
Actuated Green, G (s)	18.0	18.0		18.0	18.0		48.0	48.0		48.0	48.0	
Effective Green, g (s)	18.0	18.0		18.0	18.0		48.0	48.0		48.0	48.0	
Actuated g/C Ratio	0.24	0.24		0.24	0.24		0.64	0.64		0.64	0.64	
Clearance Time (s)	4.5	4.5		4.5	4.5		4.5	4.5		4.5	4.5	
Vehicle Extension (s)	3.0	3.0		3.0	3.0		2.0	2.0		2.0	2.0	
Lane Grp Cap (vph)	252	367		272	381		176	929		326	942	
v/s Ratio Prot	0.00	0.04		0.11	0.08		0.44	0.41		0.07	c0.60	
v/s Ratio Perm	0.03	0.47		c0.11	0.00		0.11	0.44		0.06	0.04	
v/c Ratio	0.12	0.17		0.47	0.32		0.18	0.64		0.10	0.94	
Uniform Delay, d1	22.3	22.6		24.4	23.5		5.5	8.2		5.2	12.3	
Progression Factor	1.00	1.00		1.00	1.00		1.00	1.00		0.13	0.11	
Incremental Delay, d2	0.2	0.2		1.3	0.5		2.2	3.4		0.3	10.2	
Delay (s)	22.5	22.8		25.7	24.0		7.7	11.6		0.9	11.6	
Level of Service	С	C		С	C		Α	B		Α	В	
Approach Delay (s)		22.7			24.8			11.4			11.2	
Approach LOS		С			С			В			В	
Intersection Summary												
HCM 2000 Control Delay			13.9	H	CM 2000	Level of S	Service		В			
HCM 2000 Volume to Capac	city ratio		0.82									
Actuated Cycle Length (s)			75.0	Sı	um of lost	time (s)			9.0			
Intersection Capacity Utilizat	tion		84.6%	IC	U Level o	of Service			Е			
Analysis Period (min)			15									
c Critical Lane Group												

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Movement	EBL	EBR	NBL	NBT	SBT	SBR		
Lane Configurations	*	7	ሻ	†	† ‡	-		
Volume (vph)	125	150	35	465	735	170		
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750		
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	1700		
Lane Util. Factor	1.00	1.00	1.00	1.00	0.95			
Frpb, ped/bikes	1.00	0.95	1.00	1.00	0.96			
Flpb, ped/bikes	1.00	1.00	1.00	1.00	1.00			
Frt	1.00	0.85	1.00	1.00	0.97			
Flt Protected	0.95	1.00	0.95	1.00	1.00			
Satd. Flow (prot)	1646	1403	1614	1688	3075			
Flt Permitted	0.95	1.00	0.95	1.00	1.00			
Satd. Flow (perm)	1646	1403	1614	1688	3075			
Peak-hour factor, PHF	0.97	0.97	0.97	0.97	0.97	0.97		
Adj. Flow (vph)	129	155	36	479	758	175		
RTOR Reduction (vph)	0	116	0	4/9	15	0		
Lane Group Flow (vph)	129	39	36	479	918	0		
Confl. Peds. (#/hr)	129	3 9 15	50	4/9	910	50		
Confl. Bikes (#/hr)	10	3	30			9		
Heavy Vehicles (%)	1%	1%	3%	2%	0%	1%		
Bus Blockages (#/hr)	0	1%	3% 0	2% 4	0% 4	0		
						U		
Turn Type Protected Phases	NA 2	Perm	Prot 3	NA 8	NA 4			
	Z	2	3	Ö	4			
Permitted Phases	20.0	20.0	гэ	02.0	70.0			
Actuated Green, G (s)	30.0	30.0	5.7	82.0	72.3			
Effective Green, g (s)	30.0	30.0	5.7	82.0	72.3			
Actuated g/C Ratio	0.25	0.25	0.05	0.68	0.60			
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0			
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0			
Lane Grp Cap (vph)	411	350	76	1153	1852			
v/s Ratio Prot	c0.08	0.00	0.02	c0.28	c0.30			
v/s Ratio Perm	0.04	0.03	0.47	0.10	0.50			
v/c Ratio	0.31	0.11	0.47	0.42	0.50			
Uniform Delay, d1	36.6	34.7	55.7	8.4	13.5			
Progression Factor	1.00	1.00	1.47	0.14	1.00			
Incremental Delay, d2	2.0	0.6	3.4	0.8	1.0			
Delay (s)	38.6	35.3	85.3	2.0	14.5			
Level of Service	D	D	F	A	В			
Approach Delay (s)	36.8			7.8	14.5			
Approach LOS	D			А	В			
Intersection Summary								
HCM 2000 Control Delay			16.2	Н	CM 2000	Level of Service		В
HCM 2000 Volume to Capa	city ratio		0.45					
Actuated Cycle Length (s)			120.0	Si	um of lost	time (s)	13	2.0
Intersection Capacity Utiliza	ation		51.6%	• •			12	Α
Analysis Period (min)			15					
c Critical Lane Group			10					
o ontiour Larie Group								

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	↑	7	ሻ	†	7	ň	f)		7	ħβ	
Volume (vph)	120	280	330	120	370	70	275	310	45	155	575	105
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1500	1500	1500	1750	1750	1750
Lane Width	12	10	10	10	10	11	11	10	11	12	12	12
Total Lost time (s)	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5		4.5	4.5	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00		1.00	0.95	
Frpb, ped/bikes	1.00	1.00	0.92	1.00	1.00	0.83	1.00	0.99		1.00	0.98	
Flpb, ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00		1.00	1.00	
Frt	1.00	1.00	0.85	1.00	1.00	0.85	1.00	0.98		1.00	0.98	
Flt Protected	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00		0.95	1.00	
Satd. Flow (prot)	1662	1601	1262	1492	1617	1175	1364	1314		1607	3145	
Flt Permitted	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00		0.95	1.00	
Satd. Flow (perm)	1662	1601	1262	1492	1617	1175	1364	1314		1607	3145	
Peak-hour factor, PHF	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Adj. Flow (vph)	126	295	347	126	389	74	289	326	47	163	605	111
RTOR Reduction (vph)	0	0	196	0	0	54	0	3	0	0	12	0
Lane Group Flow (vph)	126	295	151	126	389	20	289	370	0	163	704	0
Confl. Peds. (#/hr)	45		30	30		45	40		25	25		40
Confl. Bikes (#/hr)			25			17			5			14
Heavy Vehicles (%)	0%	2%	1%	4%	1%	2%	1%	3%	5%	1%	1%	0%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	10	6	0	0
Turn Type	Prot	NA	Perm	Prot	NA	Perm	Prot	NA		Prot	NA	
Protected Phases	5	2		1	6		3	8		7	4	
Permitted Phases			2			6						
Actuated Green, G (s)	13.3	31.4	31.4	14.0	32.1	32.1	27.1	40.0		16.6	29.5	
Effective Green, g (s)	13.3	31.4	31.4	14.0	32.1	32.1	27.1	40.0		16.6	29.5	
Actuated g/C Ratio	0.11	0.26	0.26	0.12	0.27	0.27	0.23	0.33		0.14	0.25	
Clearance Time (s)	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5		4.5	4.5	
Vehicle Extension (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0		2.0	2.0	
Lane Grp Cap (vph)	184	418	330	174	432	314	308	438		222	773	
v/s Ratio Prot	0.08	0.18		c0.08	c0.24		c0.21	c0.28		0.10	0.22	
v/s Ratio Perm			0.12			0.02						
v/c Ratio	0.68	0.71	0.46	0.72	0.90	0.06	0.94	0.84		0.73	0.91	
Uniform Delay, d1	51.3	40.1	37.2	51.1	42.4	32.7	45.6	37.1		49.6	44.0	
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00		1.22	0.90	
Incremental Delay, d2	8.1	4.4	0.4	11.9	21.0	0.0	34.6	17.7		9.4	15.5	
Delay (s)	59.5	44.5	37.5	63.0	63.4	32.8	80.2	54.9		69.8	55.2	
Level of Service	Е	D	D	Е	Е	С	F	D		Е	Е	
Approach Delay (s)		43.8			59.5			65.9			57.9	
Approach LOS		D			Е			Е			Е	
Intersection Summary												
HCM 2000 Control Delay			56.3	Н	CM 2000	Level of	Service		Е			
HCM 2000 Volume to Capac	city ratio											
Actuated Cycle Length (s)		120.0			um of los				18.0			
Intersection Capacity Utiliza	tion		84.1%	IC	CU Level	of Service			Е			
Analysis Period (min)			15									
c Critical Lane Group												

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Movement	WBL2	WBL	WBR	NBL	NBT	NBR	SBL	SBT	SBR	NEL	NER	NER2
Lane Configurations		M			4			4	7	M		
Volume (vph)	5	10	20	5	245	5	30	320	615	365	5	5
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Lane Width	12	12	12	12	14	12	12	12	12	12	12	12
Total Lost time (s)		4.0			4.0			4.0	5.0	5.0		
Lane Util. Factor		1.00			1.00			1.00	1.00	1.00		
Frpb, ped/bikes		0.98			1.00			1.00	0.98	1.00		
Flpb, ped/bikes		1.00			1.00			1.00	1.00	1.00		
Frt		0.92			1.00			1.00	0.85	1.00		
Flt Protected		0.98			1.00			1.00	1.00	0.95		
Satd. Flow (prot)		1495			1824			1725	1449	1642		
Flt Permitted		0.98			0.99			0.96	1.00	1.00		
Satd. Flow (perm)		1495			1812			1662	1449	1722		
Peak-hour factor, PHF	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Adj. Flow (vph)	5	11	21	5	258	5	32	337	647	384	5	5
RTOR Reduction (vph)	0	28	0	0	1	0	0	0	0	11	0	0
Lane Group Flow (vph)	0	9	0	0	267	0	0	369	647	383	0	0
Confl. Peds. (#/hr)	5	,	5	5	201	15	15	007	5	5		5
Confl. Bikes (#/hr)	U		2	J		2	10		4	O .		4
Heavy Vehicles (%)	0%	0%	6%	0%	2%	0%	0%	1%	1%	1%	0%	0%
Turn Type	Perm	NA	070	Perm	NA	070	Perm	NA	custom	NA	070	070
Protected Phases	I CIIII	8		I CIIII	2		I CIIII	6	cusioni	INA		
Permitted Phases	8	U		2	Z		6	U	3 6	3		
Actuated Green, G (s)	U	10.0		2	15.9		U	15.9	39.4	14.5		
Effective Green, g (s)		10.0			15.7			15.7	35.4	14.5		
Actuated g/C Ratio		0.25			0.40			0.40	0.90	0.37		
Clearance Time (s)		4.0			4.0			4.0	0.70	5.0		
Vehicle Extension (s)		2.5			2.5			2.5		2.0		
Lane Grp Cap (vph)		379			731			670	1301	633		
v/s Ratio Prot		3/9			731			070	1301	033		
v/s Ratio Prot v/s Ratio Perm		0.01			0.15			0.22	c0.45	c0.22		
v/c Ratio		0.01			0.15			0.22	0.50	0.61		
Uniform Delay, d1		11.0			8.2			9.0		10.1		
J ·									0.4			
Progression Factor		1.00			1.00			1.00	1.00	1.00		
Incremental Delay, d2		0.0			0.2			0.8	0.1	1.1		
Delay (s) Level of Service		11.1			8.4			9.8	0.5	11.3		
		B			Α			A	Α	B		
Approach LOS		11.1			8.4			3.9		11.3		
Approach LOS		В			А			А		В		
Intersection Summary												
HCM 2000 Control Delay			6.4						Α			
HCM 2000 Volume to Capac	city ratio		0.63									
Actuated Cycle Length (s)			39.4		um of lost				11.0			
Intersection Capacity Utilizat	ion		72.6%	IC	CU Level of	of Service	;		С			
Analysis Period (min)			15									
c Critical Lane Group												

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4	7	7	f		ሻ	f)	
Volume (vph)	15	45	15	220	80	40	5	410	190	55	690	15
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Lane Width	12	14	12	12	12	12	12	12	12	12	12	12
Total Lost time (s)		4.5			4.5	4.5	4.5	4.5		4.5	4.5	
Lane Util. Factor		1.00			1.00	1.00	1.00	1.00		1.00	1.00	
Frpb, ped/bikes		0.98			1.00	0.93	1.00	0.98		1.00	1.00	
Flpb, ped/bikes		1.00			0.99	1.00	1.00	1.00		0.99	1.00	
Frt		0.97			1.00	0.85	1.00	0.95		1.00	1.00	
Flt Protected		0.99			0.96	1.00	0.95	1.00		0.95	1.00	
Satd. Flow (prot)		1757			1661	1387	1662	1618		1654	1725	
Flt Permitted		0.92			0.76	1.00	0.20	1.00		0.28	1.00	
Satd. Flow (perm)		1623			1310	1387	347	1618		480	1725	
Peak-hour factor, PHF	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Adj. Flow (vph)	16	49	16	242	88	44	5	451	209	60	758	16
RTOR Reduction (vph)	0	11	0	0	0	30	0	20	0	0	1	0
Lane Group Flow (vph)	0	70	0	0	330	14	5	640	0	60	773	0
Confl. Peds. (#/hr)	15		10	10		15	20		10	10		20
Confl. Bikes (#/hr)			55			15			35			20
Heavy Vehicles (%)	0%	0%	0%	0%	1%	0%	0%	1%	1%	0%	1%	0%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	4	0	0	0
Turn Type	Perm	NA		Perm	NA	Perm	Perm	NA		Perm	NA	
Protected Phases		2			6			8			4	
Permitted Phases	2			6		6	8			4		
Actuated Green, G (s)		23.4			23.4	23.4	42.6	42.6		42.6	42.6	
Effective Green, g (s)		23.4			23.4	23.4	42.6	42.6		42.6	42.6	
Actuated g/C Ratio		0.31			0.31	0.31	0.57	0.57		0.57	0.57	
Clearance Time (s)		4.5			4.5	4.5	4.5	4.5		4.5	4.5	
Vehicle Extension (s)		3.0			3.0	3.0	2.0	2.0		2.0	2.0	
Lane Grp Cap (vph)		506			408	432	197	919		272	979	
v/s Ratio Prot								0.40			c0.45	
v/s Ratio Perm		0.04			c0.25	0.01	0.01			0.13		
v/c Ratio		0.14			0.81	0.03	0.03	0.70		0.22	0.79	
Uniform Delay, d1		18.6			23.7	17.9	7.1	11.6		8.0	12.7	
Progression Factor		1.00			1.00	1.00	1.63	1.57		1.00	1.00	
Incremental Delay, d2		0.1			11.2	0.0	0.2	3.8		1.9	6.5	
Delay (s)		18.7			35.0	18.0	11.8	22.0		9.9	19.2	
Level of Service		В			С	В	В	С		А	В	
Approach Delay (s)		18.7			33.0			21.9			18.5	
Approach LOS		В			С			С			В	
Intersection Summary												
HCM 2000 Control Delay			22.4	Н	CM 2000	Level of	Service		С			
HCM 2000 Volume to Capaci	ity ratio	0.80										
Actuated Cycle Length (s)		75.0		S	um of los	t time (s)			9.0			
Intersection Capacity Utilizati	on		81.6%	IC	U Level	of Service)		D			
Analysis Period (min)			15									
c Critical Lane Group												

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4		ሻ	f)		ሻ	1	
Volume (vph)	30	5	10	25	5	15	15	560	15	5	895	25
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Lane Width	12	14	12	12	14	12	12	12	12	12	12	12
Total Lost time (s)		4.5			4.5		4.5	4.5		4.5	4.5	
Lane Util. Factor		1.00			1.00		1.00	1.00		1.00	1.00	
Frpb, ped/bikes		0.98			0.96		1.00	1.00		1.00	1.00	
Flpb, ped/bikes		0.95			0.98		1.00	1.00		0.99	1.00	
Frt		0.97			0.95		1.00	1.00		1.00	1.00	
Flt Protected		0.97			0.97		0.95	1.00		0.95	1.00	
Satd. Flow (prot)		1600			1636		1662	1696		1652	1695	
Flt Permitted		0.83			0.86		0.12	1.00		0.34	1.00	
Satd. Flow (perm)		1373			1445		217	1696		599	1695	
Peak-hour factor, PHF	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Adj. Flow (vph)	32	5	11	27	5	16	16	602	16	5	962	27
RTOR Reduction (vph)	0	8	0	0	12	0	0	1	0	0	1	0
Lane Group Flow (vph)	0	40	0	0	36	0	16	617	0	5	988	0
Confl. Peds. (#/hr)	40		15	15		40	15		10	10		15
Confl. Bikes (#/hr)			15			15			35			35
Heavy Vehicles (%)	3%	0%	0%	0%	0%	0%	0%	1%	0%	0%	1%	0%
Bus Blockages (#/hr)	0	0	4	0	0	4	0	4	0	0	4	0
Turn Type	Perm	NA		Perm	NA		Perm	NA		Perm	NA	
Protected Phases		2			6			4			8	
Permitted Phases	2			6			4			8		
Actuated Green, G (s)		18.0			18.0		48.0	48.0		48.0	48.0	
Effective Green, g (s)		18.0			18.0		48.0	48.0		48.0	48.0	
Actuated g/C Ratio		0.24			0.24		0.64	0.64		0.64	0.64	
Clearance Time (s)		4.5			4.5		4.5	4.5		4.5	4.5	
Vehicle Extension (s)		3.0			3.0		2.0	2.0		2.0	2.0	
Lane Grp Cap (vph)		329			346		138	1085		383	1084	
v/s Ratio Prot								0.36			c0.58	
v/s Ratio Perm		c0.03			0.02		0.07			0.01		
v/c Ratio		0.12			0.10		0.12	0.57		0.01	0.91	
Uniform Delay, d1		22.3			22.2		5.2	7.6		4.9	11.7	
Progression Factor		1.00			1.00		1.91	1.97		0.91	0.76	
Incremental Delay, d2		0.2			0.1		1.4	1.8		0.0	8.8	
Delay (s)		22.5			22.3		11.4	16.8		4.5	17.6	
Level of Service		С			С		В	В		Α	В	
Approach Delay (s)		22.5			22.3			16.7			17.5	
Approach LOS		С			С			В			В	
Intersection Summary												
HCM 2000 Control Delay		17.5			CM 2000	Level of	Service		В			
HCM 2000 Volume to Capaci	ty ratio 0.70											
Actuated Cycle Length (s)	75.0		S	um of lost	time (s)			9.0				
Intersection Capacity Utilization				U Level o		<u>,</u>		D				
Analysis Period (min)			15									
c Critical Lane Group												

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	*	1 >		ሻ	1>		ሻ	1>		*	4	
Volume (vph)	30	50	45	125	100	35	30	540	40	30	810	55
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Total Lost time (s)	4.5	4.5		4.5	4.5		4.5	4.5		4.5	4.5	
Lane Util. Factor	1.00	1.00		1.00	1.00		1.00	1.00		1.00	1.00	
Frpb, ped/bikes	1.00	0.95		1.00	0.96		1.00	0.99		1.00	1.00	
Flpb, ped/bikes	0.91	1.00		0.95	1.00		1.00	1.00		0.98	1.00	
Frt	1.00	0.93		1.00	0.96		1.00	0.99		1.00	0.99	
Flt Protected	0.95	1.00		0.95	1.00		0.95	1.00		0.95	1.00	
Satd. Flow (prot)	1521	1524		1556	1588		1455	1451		1357	1471	
Flt Permitted	0.66	1.00		0.69	1.00		0.18	1.00		0.36	1.00	
Satd. Flow (perm)	1054	1524		1136	1588		276	1451		510	1471	
Peak-hour factor, PHF	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Adj. Flow (vph)	31	52	46	129	103	36	31	557	41	31	835	57
RTOR Reduction (vph)	0	35	0	0	17	0	0	4	0	0	3	0
Lane Group Flow (vph)	31	63	0	129	122	0	31	594	0	31	889	0
Confl. Peds. (#/hr)	50		30	30		50	20		30	30		20
Confl. Bikes (#/hr)			15			10			30			25
Heavy Vehicles (%)	0%	3%	0%	1%	3%	0%	0%	2%	3%	5%	1%	0%
Bus Blockages (#/hr)	0	0	4	0	0	4	0	4	0	0	4	0
Parking (#/hr)							5	5	5	5	5	5
Turn Type	Perm	NA		Perm	NA		Perm	NA		Perm	NA	
Protected Phases		2			6			4			8	
Permitted Phases	2			6			4			8		
Actuated Green, G (s)	18.0	18.0		18.0	18.0		48.0	48.0		48.0	48.0	
Effective Green, g (s)	18.0	18.0		18.0	18.0		48.0	48.0		48.0	48.0	
Actuated g/C Ratio	0.24	0.24		0.24	0.24		0.64	0.64		0.64	0.64	
Clearance Time (s)	4.5	4.5		4.5	4.5		4.5	4.5		4.5	4.5	
Vehicle Extension (s)	3.0	3.0		3.0	3.0		2.0	2.0		2.0	2.0	
Lane Grp Cap (vph)	252	365		272	381		176	928		326	941	
v/s Ratio Prot		0.04			0.08			0.41			c0.60	
v/s Ratio Perm	0.03			c0.11			0.11			0.06		
v/c Ratio	0.12	0.17		0.47	0.32		0.18	0.64		0.10	0.94	
Uniform Delay, d1	22.3	22.6		24.4	23.5		5.5	8.2		5.2	12.3	
Progression Factor	1.00	1.00		1.00	1.00		1.00	1.00		0.13	0.11	
Incremental Delay, d2	0.2	0.2		1.3	0.5		2.2	3.4		0.3	10.3	
Delay (s)	22.5	22.8		25.7	24.0		7.7	11.6		0.9	11.7	
Level of Service	С	С		С	С		Α	В		Α	В	
Approach Delay (s)		22.8			24.8			11.4			11.3	
Approach LOS		С			С			В			В	
Intersection Summary												
HCM 2000 Control Delay			14.0	H	CM 2000	Level of S	Service		В			
HCM 2000 Volume to Capac	city ratio		0.82									
Actuated Cycle Length (s)		75.0		Sı	um of lost	time (s)			9.0			
Intersection Capacity Utilizat	ion	84.6%			U Level o		:		E			
Analysis Period (min)			15									
c Critical Lane Group												

HCM Signalized Intersection Capacity Analysis

S. Willamette Street Corridor
4: Willamette Street & Willamette Street Plaza Driveway^{2018 PM Peak - Alt 3} (Modified 3-lane) - Double Bike Vol

Movement		٠	•	4	†	↓	✓	
Total Configurations	Movement	EBL	EBR	NBL	NBT	SBT	SBR	
Volume (ph) 125 150 35 465 735 170 Ideal Flow (vphpl) 1750 1750 1750 1750 1750 1750 Total Lost time (s) 4.0 4.0 4.0 4.0 4.0 4.0 Lane Util: Factor 1.00 1.00 1.00 1.00 0.95 1.00 0.95 1.00 0.95 1.00 0.95 1.00 0.96 Flpp ped/bikes 1.00 Satd. Flow (prot) 1646 1400 1614 1688 3071 1.00 1.00 Satd. Flow (prm) 1646 1400 1614 1688 3071 1.00 1.00 Satd. Flow (prm) 129 155 36 479 758 175 RTOR Reduction (vph) 129 155 36 479 758 175							-	
Ideal Flow (vphpl)							170	
Total Lost time (s) Lane Util. Factor Lane Group Flow (wph) Lane Group Flow Lane Lane Lane Lane Lane Lane Lane Lane								
Lane Util. Factor							1700	
Frpb, ped/bikes								
Fipb, ped/bikes								
Frit Protected 0.95 1.00 0.85 1.00 1.00 0.97 FIt Protected 0.95 1.00 0.95 1.00 1.00 Satd. Flow (prot) 1646 14400 1614 1688 3071 FIt Permitted 0.95 1.00 0.95 1.00 1.00 Satd. Flow (perm) 1646 1400 1614 1688 3071 Peak-hour factor, PHF 0.97 0.97 0.97 0.97 0.97 0.97 0.97 Adj. Flow (vph) 129 155 36 479 758 175 RTOR Reduction (vph) 0 116 0 0 15 0 Lane Group Flow (vph) 129 39 36 479 918 0 Confl. Peds. (#/hr) 15 15 50 50 50 Confl. Bikes (#/hr) 5 20 Heavy Vehicles (%) 1% 1% 3% 2% 0% 1% Bus Blockages (#/hr) 0 0 0 4 4 0 0 Turn Type NA Perm Prot NA NA Protected Phases 2 3 8 4 Permitted Phases 2 3 8 4 Permitted Phases 2 Seffective Green, g (s) 30.0 30.0 5.7 82.0 72.3 Effective Green, g (s) 30.0 30.0 5.7 82.0 72.3 Actuated g/C Ratio 0.25 0.25 0.05 0.68 0.60 Clearance Time (s) 4.0 4.0 4.0 4.0 4.0 4.0 Vehicle Extension (s) 3.0 3.0 3.0 3.0 3.0 3.0 3.0 3.0 Lane Grp Cap (vph) 411 350 76 1153 1850 V/s Ratio Perm 0.03 V/s Ratio Perm 0.03 V/c Ratio Design Factor 1.00 1.00 1.47 0.42 0.50 Uniform Delay, d1 36.6 34.7 55.7 8.4 13.5 Progression Factor 1.00 1.00 1.47 0.42 0.50 Longersion Factor 1.00 1.00 1.47 0.42 0.50 Longersion Factor 1.00 1.00 1.47 0.44 1.00 Longersion Factor 1.00 1.00 1.47 0.44 1.5 Longersion Factor 1.00 1.00 1.47 0.44 1.5 Longersion Factor 1.00 1.00 1.47 0.44 1.5 Longersion Factor 1.00 1.00 1.47 0.44								
Fit Protected 0.95 1.00 0.95 1.00 1.00 Satd. Flow (prot) 1646 1400 1614 1688 3071 Flt Permitted 0.95 1.00 0.95 1.00 1.00 Satd. Flow (perm) 1646 1400 1614 1688 3071 Flt Permitted 0.95 1.00 0.95 1.00 1.00 Satd. Flow (perm) 1646 1400 1614 1688 3071 Peak-hour factor, PHF 0.97 0.97 0.97 0.97 0.97 0.97 0.97 0.97								
Satd. Flow (prot) 1646 1400 1614 1688 3071 Fit Permitted 0.95 1.00 0.95 1.00 1.00 Satd. Flow (perm) 1646 1400 1614 1688 3071 Peak-hour factor, PHF 0.97 0.97 0.97 0.97 0.97 Adj. Flow (vph) 129 155 36 479 758 175 RTOR Reduction (vph) 0 116 0 0 15 0 Lane Group Flow (vph) 129 39 36 479 918 0 Confl. Peds. (#/hr) 15 15 50 50 50 Confl. Bikes (#/hr) 5 20 50 50 50 50 Confl. Bikes (#/hr) 0 0 0 4 4 0 1% 80 90% 1% 1% 33% 2% 0% 1% 1% 38% 2% 0% 1% 1% 3% 2% 0%								
Fit Permitted 0.95 1.00 0.95 1.00 1.00 Satd. Flow (perm) 1646 1400 1614 1688 3071 Peak-hour factor, PHF 0.97 0.97 0.97 0.97 0.97 0.97 0.97 0.97 Adj. Flow (vph) 129 155 36 479 758 175 RTOR Reduction (vph) 0 116 0 0 15 0 Lane Group Flow (vph) 129 39 36 479 918 0 Confl. Peds. (#/hr) 15 15 50 50 50 Confl. Peds. (#/hr) 15 15 50 50 50 Confl. Bikes (#/hr) 5 20 72 20 14 4 0 Heavy Vehicles (%) 1% 1% 3% 2% 0% 1% Bus Blockages (#/hr) 0 0 0 4 4 0 Turn Type NA Perm Prot								
Satd. Flow (perm) 1646 1400 1614 1688 3071 Peak-hour factor, PHF 0.97								
Peak-hour factor, PHF 0.97 0.8 0.0 0 15 0								
Adj. Flow (vph) 129 155 36 479 758 175 RTOR Reduction (vph) 0 116 0 0 15 0 Lane Group Flow (vph) 129 39 36 479 918 0 Confl. Peds. (#/hr) 15 15 50 50 Confl. Bikes (#/hr) 5 20 196 196 196 196 196 196 196 196 196 198 20 196 198 198 0 0 0 196 198 198 0 0 0 196 198 198 198 198 0 0 0 0 0 0 0 198 198 198 198 0 0 0 0 0 0 198 198 4 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0							0.97	
RTOR Reduction (vph)								
Lane Group Flow (vph) 129 39 36 479 918 0 Confl. Peds. (#/hr) 15 15 50 50 Confl. Bikes (#/hr) 5 20 Heavy Vehicles (%) 1% 1% 3% 2% 0% 1% Bus Blockages (#/hr) 0 0 0 4 4 4 0 Turn Type NA Perm Prot NA NA Protected Phases 2 3 8 4 Permitted Phases 2 Actuated Green, G (s) 30.0 30.0 5.7 82.0 72.3 Effective Green, g (s) 30.0 30.0 5.7 82.0 72.3 Actuated g/C Ratio 0.25 0.25 0.05 0.68 0.60 Clearance Time (s) 4.0 4.0 4.0 4.0 4.0 4.0 Vehicle Extension (s) 3.0 3.0 3.0 3.0 3.0 3.0 Lane Gro Cap (vph) 411 350 76 1153 1850 v/s Ratio Prot c0.08 0.02 c0.28 c0.30 v/s Ratio Perm 0.03 v/c Ratio 0.31 0.11 0.47 0.42 0.50 Uniform Delay, d1 36.6 34.7 55.7 8.4 13.5 Progression Factor 1.00 1.00 1.47 0.14 1.00 Incremental Delay, d2 2.0 0.6 3.4 0.8 1.0 Delay (s) 38.6 35.3 85.4 2.0 14.5 Level of Service D D F A B Approach LOS D Intersection Summary								
Confl. Peds. (#/hr) 15 15 50 50 Confl. Bikes (#/hr) 5 20 Heavy Vehicles (%) 1% 1% 3% 2% 0% 1% Bus Blockages (#/hr) 0 0 0 0 4 4 0 Turn Type NA Perm Prot NA NA Protected Phases 2 3 8 4 Permitted Phases 2 Actuated Green, G (s) 30.0 30.0 5.7 82.0 72.3 Effective Green, g (s) 30.0 30.0 5.7 82.0 72.3 Actuated g/C Ratio 0.25 0.25 0.05 0.68 0.60 Clearance Time (s) 4.0 4.0 4.0 4.0 4.0 Vehicle Extension (s) 3.0 3.0 3.0 3.0 3.0 3.0 Lane Grp Cap (vph) 411 350 76 1153 1850 v/s Ratio Prot c0.08 0.02 c0.28 c0.30 v/s Ratio Perm 0.03 v/c Ratio 0.31 0.11 0.47 0.42 0.50 Uniform Delay, d1 36.6 34.7 55.7 8.4 13.5 Progression Factor 1.00 1.00 1.47 0.14 1.00 Incremental Delay, d2 2.0 0.6 3.4 0.8 1.0 Delay (s) 38.6 35.3 85.4 2.0 14.5 Level of Service D D F A B Approach LOS D MARM Perm 20 Two NA NA Perm Prot NA NA NA NA Perm Prot NA NA NA NA Perm Prot NA NA NA NA Perm Prot NA NA NA NA NA Perm Prot NA NA NA Perm Prot NA NA NA NA Perm Prot NA NA NA NA Perm Prot NA NA NA Perm Prot NA NA NA NA NA NA Perm Prot NA NA NA NA NA Perm Prot NA NA NA NA NA Perm Prot NA NA NA NA NA NA Perm Prot NA NA NA NA NA Perm Prot NA NA NA NA NA NA Perm Prot NA N	• • • • • • • • • • • • • • • • • • • •							
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Heavy Vehicles (%) 1% 1% 3% 2% 0% 1% Bus Blockages (#/hr) 0 0 0 0 4 4 4 0 Turn Type NA Perm Prot NA NA Perm Protected Phases 2 3 8 4 Permitted Phases 2 3 8 4 Permitted Phases 2 3 8 4 Actuated Green, G (s) 30.0 30.0 5.7 82.0 72.3 Effective Green, g (s) 30.0 30.0 5.7 82.0 72.3 Actuated g/C Ratio 0.25 0.25 0.05 0.68 0.60 Clearance Time (s) 4.0 4.0 4.0 4.0 4.0 Vehicle Extension (s) 3.0 3.0 3.0 3.0 3.0 Lane Grp Cap (vph) 411 350 76 1153 1850 v/s Ratio Prot c0.08 0.02 c0.28 c0.30 v/s Ratio Perm 0.03 v/c Ratio 0.31 0.11 0.47 0.42 0.50 Uniform Delay, d1 36.6 34.7 55.7 8.4 13.5 Progression Factor 1.00 1.00 1.47 0.14 1.00 Incremental Delay, d2 2.0 0.6 3.4 0.8 1.0 Delay (s) 38.6 35.3 85.4 2.0 14.5 Level of Service D D F A B Approach LOS D A B		10		30				
Bus Blockages (#/hr) 0 0 0 4 4 0 Turn Type NA Perm Prot NA NA Protected Phases 2 3 8 4 Permitted Phases 2 30.0 30.0 5.7 82.0 72.3 Actuated Green, G (s) 30.0 30.0 5.7 82.0 72.3 Actuated g/C Ratio 0.25 0.25 0.05 0.68 0.60 Clearance Time (s) 4.0 4.0 4.0 4.0 4.0 Vehicle Extension (s) 3.0 3.0 3.0 3.0 3.0 Lane Grp Cap (vph) 411 350 76 1153 1850 v/s Ratio Prot c0.08 0.02 c0.28 c0.30 v/s Ratio Perm 0.03 0.02 c0.28 c0.30 v/c Ratio 0.31 0.11 0.47 0.42 0.50 Uniform Delay, d1 36.6 34.7 55.7 8.4 13.5 <		1%		3%	2%	0%		
Turn Type								
Protected Phases 2 3 8 4 Permitted Phases 2 Actuated Green, G (s) 30.0 30.0 5.7 82.0 72.3 Effective Green, g (s) 30.0 30.0 5.7 82.0 72.3 Actuated g/C Ratio 0.25 0.25 0.05 0.68 0.60 Clearance Time (s) 4.0 4.0 4.0 4.0 4.0 Vehicle Extension (s) 3.0 3.0 3.0 3.0 3.0 Lane Grp Cap (vph) 411 350 76 1153 1850 v/s Ratio Prot c0.08 0.02 c0.28 c0.30 v/s Ratio Perm 0.03 v/c Ratio 0.31 0.11 0.47 0.42 0.50 Uniform Delay, d1 36.6 34.7 55.7 8.4 13.5 Progression Factor 1.00 1.00 1.47 0.14 1.00 Incremental Delay, d2 2.0 0.6 3.4 0.8 1.0 Delay (s) 38.6 35.3 85.4 2.0 14.5 Level of Service D D F A B Approach LOS D A B Intersection Summary							<u> </u>	
Permitted Phases 2 Actuated Green, G (s) 30.0 30.0 5.7 82.0 72.3 Effective Green, g (s) 30.0 30.0 5.7 82.0 72.3 Actuated g/C Ratio 0.25 0.25 0.05 0.68 0.60 Clearance Time (s) 4.0 4.0 4.0 4.0 4.0 Vehicle Extension (s) 3.0 3.0 3.0 3.0 3.0 Lane Grp Cap (vph) 411 350 76 1153 1850 v/s Ratio Prot c0.08 0.02 c0.28 c0.30 v/s Ratio Perm 0.03 v/c Ratio 0.31 0.11 0.47 0.42 0.50 Uniform Delay, d1 36.6 34.7 55.7 8.4 13.5 Progression Factor 1.00 1.00 1.47 0.14 1.00 Incremental Delay, d2 2.0 0.6 3.4 0.8 1.0 Delay (s) 38.6 35.3 85.4 2.0 14.5 Level of Service D D F A			1 01111					
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Actuated g/C Ratio 0.25 0.25 0.05 0.68 0.60 Clearance Time (s) 4.0 4.0 4.0 4.0 4.0 4.0 Vehicle Extension (s) 3.0 3.0 3.0 3.0 3.0 3.0 3.0 3.0 3.0 Lane Grp Cap (vph) 411 350 76 1153 1850 v/s Ratio Prot co.08 0.02 co.28 co.30 v/s Ratio Perm 0.03 v/c Ratio 0.31 0.11 0.47 0.42 0.50 Uniform Delay, d1 36.6 34.7 55.7 8.4 13.5 Progression Factor 1.00 1.00 1.47 0.14 1.00 Incremental Delay, d2 2.0 0.6 3.4 0.8 1.0 Delay (s) 38.6 35.3 85.4 2.0 14.5 Level of Service D D F A B Approach Delay (s) 36.8 7.8 14.5 Approach LOS D A B Intersection Summary								
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Vehicle Extension (s) 3.0 2.0 0.30 9.0 9.0 9.0 3.0 3.0 3.5 3.4 3.5 3.5 3.4 3.5 3.4 3.5 3.4 3.5 3.4 3.0 3.1 3.0 3.1 3.0 3.1 3.0 3.1 3.0 3.1 3.0 3.1								
Lane Grp Cap (vph) 411 350 76 1153 1850 v/s Ratio Prot c0.08 0.02 c0.28 c0.30 v/s Ratio Perm 0.03 v/c Ratio 0.31 0.11 0.47 0.42 0.50 Uniform Delay, d1 36.6 34.7 55.7 8.4 13.5 Progression Factor 1.00 1.00 1.47 0.14 1.00 Incremental Delay, d2 2.0 0.6 3.4 0.8 1.0 Delay (s) 38.6 35.3 85.4 2.0 14.5 Level of Service D D F A B Approach Delay (s) 36.8 7.8 14.5 Approach LOS D A B Intersection Summary	, ,							
v/s Ratio Prot c0.08 0.02 c0.28 c0.30 v/s Ratio Perm 0.03 0.01 0.47 0.42 0.50 Uniform Delay, d1 36.6 34.7 55.7 8.4 13.5 Progression Factor 1.00 1.00 1.47 0.14 1.00 Incremental Delay, d2 2.0 0.6 3.4 0.8 1.0 Delay (s) 38.6 35.3 85.4 2.0 14.5 Level of Service D D F A B Approach Delay (s) 36.8 7.8 14.5 Approach LOS D A B Intersection Summary								
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v/c Ratio 0.31 0.11 0.47 0.42 0.50 Uniform Delay, d1 36.6 34.7 55.7 8.4 13.5 Progression Factor 1.00 1.00 1.47 0.14 1.00 Incremental Delay, d2 2.0 0.6 3.4 0.8 1.0 Delay (s) 38.6 35.3 85.4 2.0 14.5 Level of Service D D F A B Approach Delay (s) 36.8 7.8 14.5 Approach LOS D A B Intersection Summary		00.00	0.03	0.02	00.20	00.00		
Uniform Delay, d1 36.6 34.7 55.7 8.4 13.5 Progression Factor 1.00 1.00 1.47 0.14 1.00 Incremental Delay, d2 2.0 0.6 3.4 0.8 1.0 Delay (s) 38.6 35.3 85.4 2.0 14.5 Level of Service D D F A B Approach Delay (s) 36.8 7.8 14.5 Approach LOS D A B Intersection Summary		0.31		0.47	0.42	0.50		
Progression Factor 1.00 1.00 1.47 0.14 1.00 Incremental Delay, d2 2.0 0.6 3.4 0.8 1.0 Delay (s) 38.6 35.3 85.4 2.0 14.5 Level of Service D D F A B Approach Delay (s) 36.8 7.8 14.5 Approach LOS D A B Intersection Summary								
Incremental Delay, d2								
Delay (s) 38.6 35.3 85.4 2.0 14.5 Level of Service D D F A B Approach Delay (s) 36.8 7.8 14.5 Approach LOS D A B Intersection Summary								
Level of Service D D F A B Approach Delay (s) 36.8 7.8 14.5 Approach LOS D A B Intersection Summary								
Approach Delay (s) 36.8 7.8 14.5 Approach LOS D A B Intersection Summary								
Approach LOS D A B Intersection Summary			D	'				
Intersection Summary								
					/\			
HCM 2000 Control Delay 16.2 HCM 2000 Level of Service								
J	HCM 2000 Control Delay			16.2	Н	CM 2000	Level of Service	
HCM 2000 Volume to Capacity ratio 0.45		icity ratio						
Actuated Cycle Length (s) 120.0 Sum of lost time (s)							. ,	
Intersection Capacity Utilization 51.6% ICU Level of Service		ation			IC	CU Level c	of Service	
Analysis Period (min) 15				15				
c Critical Lane Group	c Critical Lane Group							

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	7	†	7	ሻ	†	7	ሻ	f)		7	ħβ	
Volume (vph)	120	280	330	120	370	70	275	310	45	155	575	105
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1500	1500	1500	1750	1750	1750
Lane Width	12	10	10	10	10	11	11	10	11	12	12	12
Total Lost time (s)	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5		4.5	4.5	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00		1.00	0.95	
Frpb, ped/bikes	1.00	1.00	0.90	1.00	1.00	0.81	1.00	0.99		1.00	0.97	
Flpb, ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00		1.00	1.00	
Frt	1.00	1.00	0.85	1.00	1.00	0.85	1.00	0.98		1.00	0.98	
Flt Protected	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00		0.95	1.00	
Satd. Flow (prot)	1662	1601	1235	1492	1617	1144	1364	1313		1607	3135	
Flt Permitted	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00		0.95	1.00	
Satd. Flow (perm)	1662	1601	1235	1492	1617	1144	1364	1313		1607	3135	
Peak-hour factor, PHF	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Adj. Flow (vph)	126	295	347	126	389	74	289	326	47	163	605	111
RTOR Reduction (vph)	0	0	196	0	0	54	0	3	0	0	12	0
Lane Group Flow (vph)	126	295	151	126	389	20	289	370	0	163	704	0
Confl. Peds. (#/hr)	45		30	30		45	40		25	25		40
Confl. Bikes (#/hr)			50			35			10			30
Heavy Vehicles (%)	0%	2%	1%	4%	1%	2%	1%	3%	5%	1%	1%	0%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	10	6	0	0
Turn Type	Prot	NA	Perm	Prot	NA	Perm	Prot	NA		Prot	NA	
Protected Phases	5	2		1	6		3	8		7	4	
Permitted Phases			2			6						
Actuated Green, G (s)	13.3	31.4	31.4	14.0	32.1	32.1	27.1	40.0		16.6	29.5	
Effective Green, g (s)	13.3	31.4	31.4	14.0	32.1	32.1	27.1	40.0		16.6	29.5	
Actuated g/C Ratio	0.11	0.26	0.26	0.12	0.27	0.27	0.23	0.33		0.14	0.25	
Clearance Time (s)	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5		4.5	4.5	
Vehicle Extension (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0		2.0	2.0	
Lane Grp Cap (vph)	184	418	323	174	432	306	308	437		222	770	
v/s Ratio Prot	0.08	0.18		c0.08	c0.24		c0.21	c0.28		0.10	0.22	
v/s Ratio Perm			0.12			0.02						
v/c Ratio	0.68	0.71	0.47	0.72	0.90	0.06	0.94	0.85		0.73	0.91	
Uniform Delay, d1	51.3	40.1	37.3	51.1	42.4	32.8	45.6	37.1		49.6	44.0	
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00		1.22	0.91	
Incremental Delay, d2	8.1	4.4	0.4	11.9	21.0	0.0	34.6	18.0		9.4	15.9	
Delay (s)	59.5	44.5	37.7	63.0	63.4	32.8	80.2	55.1		69.8	55.8	
Level of Service	Е	D	D	Е	Е	С	F	Е		Е	Е	
Approach Delay (s)		43.9			59.5			66.1			58.4	
Approach LOS		D			Е			Е			Е	
Intersection Summary												
HCM 2000 Control Delay			56.5	H	CM 2000	Level of S	Service		Е			
HCM 2000 Volume to Capac	ity ratio											
Actuated Cycle Length (s)		120.0		Sı	um of lost	time (s)			18.0			
Intersection Capacity Utilizat	ion	84.1%				of Service	<u>,</u>		Е			
Analysis Period (min)		15										
c Critical Lane Group												

HCM Signalized Intersection Capacity Analysis
S. Willamette Street Corridor
6: S. Willamette & Donald St/Willamette Street & 32nd Alle PM Peak - Alt 3 (Modified 3-lane) - Double Bike Vol

	•	/	•	*1	†	<i>></i>	\	ļ	لړ	†	<i>></i>	4
Movement	WBL2	WBL	WBR	NBL	NBT	NBR	SBL	SBT	SBR	NEL	NER	NER2
Lane Configurations		M			4			र्स	7	M		
Volume (vph)	5	10	20	5	245	5	30	320	615	365	5	5
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Lane Width	12	12	12	12	14	12	12	12	12	12	12	12
Total Lost time (s)		4.0			4.0			4.0	5.0	5.0		
Lane Util. Factor		1.00			1.00			1.00	1.00	1.00		
Frpb, ped/bikes		0.98			1.00			1.00	0.98	1.00		
Flpb, ped/bikes		1.00			1.00			1.00	1.00	1.00		
Frt		0.92			1.00			1.00	0.85	1.00		
Flt Protected		0.98			1.00			1.00	1.00	0.95		
Satd. Flow (prot)		1491			1824			1725	1446	1642		
Flt Permitted		0.98			0.99			0.96	1.00	1.00		
Satd. Flow (perm)		1491			1812			1662	1446	1722		
Peak-hour factor, PHF	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Adj. Flow (vph)	5	11	21	5	258	5	32	337	647	384	5	5
RTOR Reduction (vph)	0	28	0	0	1	0	0	0	0 47	11	0	0
Lane Group Flow (vph)	0	9	0	0	267	0	0	369	647	383	0	0
Confl. Peds. (#/hr)	5	,	5	5	207	15	15	307	5	5	U	5
Confl. Bikes (#/hr)	J		5	J		5	13		10	J		10
Heavy Vehicles (%)	0%	0%	6%	0%	2%	0%	0%	1%	1%	1%	0%	0%
Turn Type	Perm	NA	070	Perm	NA	070	Perm	NA	custom	NA	070	070
Protected Phases	reiiii	NA 8		reiiii	2		reiiii	6	Custom	IVA		
Permitted Phases	8	0		2	Z		6	0	3 6	3		
Actuated Green, G (s)	0	10.0			15.9		U	15.9	39.4	14.5		
Effective Green, g (s)		10.0			15.9			15.9	35.4	14.5		
		0.25			0.40			0.40	0.90	0.37		
Actuated g/C Ratio								4.0	0.90	5.0		
Clearance Time (s)		4.0			4.0							
Vehicle Extension (s)		2.5			2.5			2.5	1000	2.0		
Lane Grp Cap (vph)		378			731			670	1299	633		
v/s Ratio Prot		0.01			0.15			0.00	-0.45	-0.00		
v/s Ratio Perm		0.01			0.15			0.22	c0.45	c0.22		
v/c Ratio		0.02			0.36			0.55	0.50	0.61		
Uniform Delay, d1		11.0			8.2			9.0	0.4	10.1		
Progression Factor		1.00			1.00			1.00	1.00	1.00		
Incremental Delay, d2		0.0			0.2			0.8	0.1	1.1		
Delay (s)		11.1			8.4			9.8	0.5	11.3		
Level of Service		В			Α			А	Α	В		
Approach Delay (s)		11.1			8.4			3.9		11.3		
Approach LOS		В			Α			Α		В		
Intersection Summary												
HCM 2000 Control Delay			6.4	H	CM 2000	Level of S	Service		Α			
HCM 2000 Volume to Capac	ity ratio		0.63									
Actuated Cycle Length (s)		39.4			um of lost	time (s)			11.0			
Intersection Capacity Utilizati	on		72.6%		CU Level c		:		С			
Analysis Period (min)			15									
c Critical Lane Group												

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S. Willamette Street Corridor 2018 PM Peak - Alt 3&5 (Modified 3-lane) - Bus Pullouts

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4	7	*	f _r		ሻ	f)	
Volume (vph)	15	45	15	220	80	40	5	410	190	55	690	15
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Lane Width	12	14	12	12	12	12	12	12	12	12	12	12
Total Lost time (s)		4.5			4.5	4.5	4.5	4.5		4.5	4.5	
Lane Util. Factor		1.00			1.00	1.00	1.00	1.00		1.00	1.00	
Frpb, ped/bikes		0.99			1.00	0.94	1.00	0.98		1.00	1.00	
Flpb, ped/bikes		1.00			0.99	1.00	1.00	1.00		0.99	1.00	
Frt		0.97			1.00	0.85	1.00	0.95		1.00	1.00	
Flt Protected		0.99			0.96	1.00	0.95	1.00		0.95	1.00	
Satd. Flow (prot)		1768			1661	1402	1662	1624		1654	1725	
Flt Permitted		0.92			0.76	1.00	0.20	1.00		0.28	1.00	
Satd. Flow (perm)		1634			1310	1402	347	1624		480	1725	
Peak-hour factor, PHF	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Adj. Flow (vph)	16	49	16	242	88	44	5	451	209	60	758	16
RTOR Reduction (vph)	0	11	0	0	0	30	0	20	0	0	1	0
Lane Group Flow (vph)	0	70	0	0	330	14	5	640	0	60	773	0
Confl. Peds. (#/hr)	15		10	10		15	20		10	10		20
Confl. Bikes (#/hr)			26			6			17			11
Heavy Vehicles (%)	0%	0%	0%	0%	1%	0%	0%	1%	1%	0%	1%	0%
Turn Type	Perm	NA		Perm	NA	Perm	Perm	NA		Perm	NA	
Protected Phases		2			6			8			4	
Permitted Phases	2			6		6	8			4		
Actuated Green, G (s)		23.4			23.4	23.4	42.6	42.6		42.6	42.6	
Effective Green, g (s)		23.4			23.4	23.4	42.6	42.6		42.6	42.6	
Actuated g/C Ratio		0.31			0.31	0.31	0.57	0.57		0.57	0.57	
Clearance Time (s)		4.5			4.5	4.5	4.5	4.5		4.5	4.5	
Vehicle Extension (s)		3.0			3.0	3.0	2.0	2.0		2.0	2.0	
Lane Grp Cap (vph)		509			408	437	197	922		272	979	
v/s Ratio Prot								0.39			c0.45	
v/s Ratio Perm		0.04			c0.25	0.01	0.01			0.13		
v/c Ratio		0.14			0.81	0.03	0.03	0.69		0.22	0.79	
Uniform Delay, d1		18.5			23.7	17.9	7.1	11.6		8.0	12.7	
Progression Factor		1.00			1.00	1.00	1.64	1.54		1.00	1.00	
Incremental Delay, d2		0.1			11.2	0.0	0.2	3.7		1.9	6.5	
Delay (s)		18.7			35.0	18.0	11.9	21.5		9.9	19.2	
Level of Service		В			С	В	В	С		Α	В	
Approach Delay (s)		18.7			33.0			21.5			18.5	
Approach LOS		В			С			С			В	
Intersection Summary												
HCM 2000 Control Delay			22.3	Н	CM 2000	Level of S	Service		С			
HCM 2000 Volume to Capaci	ty ratio		0.80									
Actuated Cycle Length (s)			75.0		um of los				9.0			
Intersection Capacity Utilization	on		81.6%	IC	CU Level	of Service			D			
Analysis Period (min)			15									
c Critical Lane Group												

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4		ሻ	f)		ሻ	f)	
Volume (vph)	30	5	10	25	5	15	15	560	15	5	895	25
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Lane Width	12	14	12	12	14	12	12	12	12	12	12	12
Total Lost time (s)		4.5			4.5		4.5	4.5		4.5	4.5	
Lane Util. Factor		1.00			1.00		1.00	1.00		1.00	1.00	
Frpb, ped/bikes		0.99			0.96		1.00	1.00		1.00	1.00	
Flpb, ped/bikes		0.95			0.98		1.00	1.00		0.99	1.00	
Frt		0.97			0.95		1.00	1.00		1.00	1.00	
Flt Protected		0.97			0.97		0.95	1.00		0.95	1.00	
Satd. Flow (prot)		1605			1642		1662	1724		1652	1723	
Flt Permitted		0.83			0.86		0.12	1.00		0.34	1.00	
Satd. Flow (perm)		1378			1450		217	1724		599	1723	
Peak-hour factor, PHF	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Adj. Flow (vph)	32	5	11	27	5	16	16	602	16	5	962	27
RTOR Reduction (vph)	0	8	0	0	12	0	0	1	0	0	1	0
Lane Group Flow (vph)	0	40	0	0	36	0	16	617	0	5	988	0
Confl. Peds. (#/hr)	40	10	15	15	30	40	15	017	10	10	700	15
Confl. Bikes (#/hr)	10		6	10		7	10		17	10		17
Heavy Vehicles (%)	3%	0%	0%	0%	0%	0%	0%	1%	0%	0%	1%	0%
Turn Type	Perm	NA	070	Perm	NA	070	Perm	NA	070	Perm	NA	070
Protected Phases	I CIIII	2		I CIIII	6		I CIIII	4		I CIIII	8	
Permitted Phases	2	2		6	U		4	4		8	U	
Actuated Green, G (s)		18.0		U	18.0		48.0	48.0		48.0	48.0	
Effective Green, g (s)		18.0			18.0		48.0	48.0		48.0	48.0	
Actuated g/C Ratio		0.24			0.24		0.64	0.64		0.64	0.64	
Clearance Time (s)		4.5			4.5		4.5	4.5		4.5	4.5	
Vehicle Extension (s)		3.0			3.0		2.0	2.0		2.0	2.0	
Lane Grp Cap (vph)		330			348		138	1103		383	1102	
v/s Ratio Prot		330			340		130	0.36		303	c0.57	
v/s Ratio Prot v/s Ratio Perm		c0.03			0.02		0.07	0.30		0.01	CU.57	
		0.12			0.02			0.56			0.90	
v/c Ratio Uniform Delay, d1		22.3			22.2		0.12 5.2	7.6		0.01 4.9		
											11.4	
Progression Factor		1.00			1.00		1.94	1.97		0.91	0.73	
Incremental Delay, d2		0.2			0.1		1.4	1.7		0.0	7.6	
Delay (s) Level of Service		22.5			22.3		11.6	16.6		4.5	15.9	
		C			C		В	B		А	1F.0	
Approach Delay (s)		22.5			22.3			16.4			15.9	
Approach LOS		С			С			В			В	
Intersection Summary												
HCM 2000 Control Delay			16.4	Н	CM 2000	Level of	Service		В			
HCM 2000 Volume to Capacit	y ratio		0.68									
Actuated Cycle Length (s)			75.0		um of lost				9.0			
Intersection Capacity Utilization	on		75.3%	IC	CU Level of	of Service)		D			
Analysis Period (min)			15									
c Critical Lane Group												

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EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
ሻ	f)		ሻ	1>		ሻ	1>		ሻ	f)	
30	50	45	125	100	35	30	540	40	30	810	55
1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
	4.5		4.5	4.5		4.5	4.5		4.5	4.5	
1.00	1.00		1.00	1.00		1.00	1.00		1.00	1.00	
1.00	0.96		1.00	0.97		1.00	0.99		1.00	1.00	
0.91	1.00		0.95	1.00		1.00	1.00		0.98	1.00	
1.00	0.93		1.00	0.96		1.00	0.99		1.00	0.99	
0.95	1.00		0.95	1.00		0.95	1.00		0.95	1.00	
1521	1533		1556	1591		1455	1476		1357	1496	
0.66	1.00		0.69	1.00		0.18	1.00		0.36	1.00	
1054	1533		1136	1591		276	1476		510	1496	
0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
		46									57
											0
											0
											20
		7									13
0%	3%	0%	1%	3%		0%	2%		5%	1%	0%
											5
Perm	NA		Perm	NA				-			
			. 0								
2	_		6	_		4	•		8		
	18.0			18.0			48.0			48.0	
0.03			c0.11			0.11			0.06		
	0.17			0.32			0.63			0.93	
	С			С			В			В	
		13.3	H	CM 2000	Level of S	Service		В			
ratio		0.80									
		75.0	Si	ım of lost	time (s)			9.0			
		75.0	30	וכטו וט וווג	(0)						
n		84.6%			of Service			E			
n											
	EBL 30 1750 4.5 1.00 1.00 0.91 1.00 0.95 1521 0.66 1054 0.97 31 0 31 50 Perm 2 18.0 18.0 0.24 4.5 3.0 252 0.03 0.12 22.3 1.00 0.2 22.5 C	EBL EBT 30 50 1750 1750 4.5 4.5 1.00 1.00 1.00 0.96 0.91 1.00 1.00 0.93 0.95 1.00 1521 1533 0.66 1.00 1054 1533 0.97 0.97 31 52 0 35 31 63 50 Perm NA 2 18.0 18.0 18.0 18.0 18.0 18.0 0.24 0.24 4.5 4.5 3.0 3.0 252 367 0.04 0.03 0.12 0.17 22.3 22.6 1.00 0.2 0.2 22.5 22.8 C C 22.7 C	EBL EBT EBR 30 50 45 1750 1750 1750 4.5 4.5 1.00 1.00 1.00 0.96 0.91 1.00 1.00 0.93 0.95 1.00 1521 1533 0.66 1.00 1054 1533 0.97 0.97 0.97 31 52 46 0 35 0 31 63 0 50 30 7 0% 3% 0% Perm NA 2 2 18.0 18.0 18.0 18.0 18.0 18.0 0.24 0.24 4.5 4.5 3.0 3.0 252 367 0.04 0.03 0.12 0.17 22.3 22.6 1.00 1.00 0.2 0.2 22.5 22.8 C C 22.7 C	EBL EBT EBR WBL 30 50 45 125 1750 1750 1750 1750 4.5 4.5 4.5 1.00 1.00 1.00 1.00 0.96 1.00 0.91 1.00 0.93 1.00 0.95 1.00 0.93 1.00 0.95 1.00 0.95 1521 1533 1556 0.66 1.00 0.69 1054 1533 1136 0.97 0.97 0.97 0.97 31 52 46 129 0 35 0 0 31 63 0 129 50 30 30 7 0% 3% 0% 1% Perm NA Perm 2 2 6 18.0 18.0 18.0 18.0 18.0 18.0 18.0 0.24 0.24 4.5 4.5 4.5 3.0 3.0 3.0 252 367 272 0.04 0.03 C0.11 0.12 0.17 0.47 22.3 22.6 24.4 1.00 1.00 1.00 0.2 0.2 1.3 22.5 22.8 25.7 C C C 22.7 C C Tratio 0.80	EBL EBT EBR WBL WBT 30 50 45 125 100 1750 1750 1750 1750 1750 4.5 4.5 4.5 4.5 1.00 1.00 1.00 0.97 0.91 1.00 0.95 1.00 1.00 0.93 1.00 0.96 0.95 1.00 0.95 1.00 1.521 1533 1556 1591 0.66 1.00 0.69 1.00 1054 1533 1136 1591 0.97 0.97 0.97 0.97 0.97 31 52 46 129 103 0 35 0 0 17 31 63 0 129 122 50 30 30 30 7 0% 3% 0% 1% 3% Perm NA Perm NA	EBL EBT EBR WBL WBT WBR 30 50 45 125 100 35 1750 1750 1750 1750 1750 1750 4.5 4.5 4.5 4.5 1.00 1.00 1.00 1.00 1.00 1.00 0.96 1.00 0.97 0.91 1.00 0.93 1.00 0.96 0.95 1.00 0.95 1.00 1521 1533 1556 1591 0.66 1.00 0.69 1.00 1054 1533 1136 1591 0.97 0.97 0.97 0.97 0.97 0.97 31 52 46 129 103 36 0 35 0 0 17 0 0 31 63 0 129 122 0 50 30 30 50 0% 3% 0% 1% 3% 0% Perm NA Perm NA 2 6 2 6 18.0 18.0 18.0 18.0 18.0 18.0 18.0 18.0 18.0 18.0 18.0 18.0 18.0 18.0 18.0 18.0 18.0 0.24 0.24 0.24 0.24 4.5 4.5 4.5 4.5 3.0 3.0 3.0 3.0 252 367 272 381 0.04 0.08 0.03 C0.11 0.12 0.17 0.47 0.32 22.3 22.6 24.4 23.5 1.00 1.00 1.00 1.00 0.2 0.2 1.3 0.5 22.5 22.8 25.7 24.0 C C C C 22.7 24.8 C C C C 22.7 24.8 C C C C 13.3 HCM 2000 Level of Serial Control	EBI EBT EBR WBL WBT WBR NBL 1750 1750 1750 1750 1750 1750 1750 1750	FBL	EBL EBT EBR WBL WBT WBR NBL NBT NBR 30 50 45 125 100 35 30 540 40 1750 1750 1750 1750 1750 1750 1750 1750	EBL EBT EBR WBL WBT WBR NBL NBT NBR SBL N750 1750 1750 1750 1750 1750 1750 1750 1	BBL BBT BBR WBL WBT WBR NBL NBT NBR SBL SBT NBT NBR SBL SBT NBT NBT

HCM Signalized Intersection Capacity Analysis S. Willamette Street Corridor 4: Willamette Street & Willamette Street Plaza Driveway 2018 PM Peak - Alt 3&5 (Modified 3-lane) - Bus Pullouts

Movement		۶	•	•	†	↓	4	
Lane Configurations	Movement	EBL	EBR	NBL	NBT	SBT	SBR	
Volume (vph) 125 150 35 465 735 170 Ideal Flow (vphp) 1750 1750 1750 1750 1750 1750 Total Lost time (s) 4.0	Lane Configurations	ሻ	7	ሻ	†	↑ ↑		
Ideal Flow (yphpl)		125	150	35	465	735	170	
Lane Util. Factor 1.00 1.00 1.00 1.00 0.96 Frpb. ped/bikes 1.00 0.95 1.00 1.00 1.00 1.00 Frt 1.00 0.85 1.00 1.00 0.97 FIT Fil Protected 0.95 1.00 0.95 1.00 1.00 Satd. Flow (prort) 1646 1403 1614 1716 3099 Filt Promitted 0.95 1.00 0.95 1.00 1.00 Satd. Flow (perm) 1646 1403 1614 1716 3099 Peak-hour factor, PHF 0.97 0.97 0.97 0.97 0.97 0.97 Adj. Flow (vph) 129 155 36 479 758 175 RTOR Reduction (vph) 0 116 0 0 15 0 Lane Group Flow (vph) 129 39 36 479 918 0 Confl. Bikes (#hr) 3 9 1 1 1 1<		1750	1750	1750	1750	1750	1750	
Frpb, ped/bikes 1.00 0.95 1.00	Total Lost time (s)	4.0	4.0	4.0	4.0	4.0		
Fipb, ped/bikes	Lane Util. Factor	1.00	1.00	1.00	1.00	0.95		
Fit Protected	Frpb, ped/bikes	1.00	0.95	1.00	1.00	0.96		
Fit Protected 0.95 1.00 0.95 1.00 1.00 Sald. Flow (prot) 1646 1403 1614 1716 3099 Fit Permitted 0.95 1.00 0.95 1.00 1.00 Sald. Flow (perm) 1646 1403 1614 1716 3099 Sald. Flow (perm) 1646 1403 1614 1716 3099 Sald. Flow (perm) 1646 1403 1614 1716 3099 Sald. Flow (perm) 129 155 36 479 758 175 Sald. Flow (pph) 129 155 36 479 758 175 Sald. Flow (pph) 129 39 36 479 918 0 Sald. Flow (pph) 129 39 36 479 918 0 Sald. Flow (pph) 155 50 Sald. Flow (pph) 155 50 Sald. Flow (pph) 155 155 50 Sald. Flow (pph) 155 155 50 Sald. Flow (pph) 160 Sald	Flpb, ped/bikes	1.00	1.00	1.00	1.00	1.00		
Satd. Flow (prot) 1646 1403 1614 1716 3099 Flt Permitted 0.95 1.00 0.95 1.00 1.00 Satd. Flow (perm) 1646 1403 1614 1716 3099 Peak-hour factor, PHF 0.97 0.97 0.97 0.97 0.97 0.97 Adj. Flow (vph) 129 155 36 479 758 175 RTOR Reduction (vph) 10 116 0 0 15 0 Lane Group Flow (vph) 129 39 36 479 918 0 Confl. Beds. (#/hr) 15 15 50 50 50 Confl. Bikes (#/hr) 15 18 38 2% 0% 1% Turn Type NA Perm Prot NA NA Permetected Phases 2 2 3 8 4 Permitted Phases 2 2 3 8 4 4 4 4 4	Frt	1.00	0.85	1.00	1.00	0.97		
Fit Permitted	Flt Protected	0.95	1.00	0.95	1.00	1.00		
Sald. Flow (perm) 1646 1403 1614 1716 3099 Peak-hour factor, PHF 0.97 0.97 0.97 0.97 0.97 0.97 Adj. Flow (vph) 129 155 36 479 758 175 RTOR Reduction (vph) 10 116 0 0 15 0 Lane Group Flow (vph) 129 39 36 479 918 0 Confl. Peds. (#/hr) 15 15 50 50 50 Confl. Bikes (#/hr) 3 9 14 38 2% 0% 19 Heavy Vehicles (%) 1% 1% 3% 2% 0% 19 Heavy Vehicles (%) 1% 1% 3% 2% 0% 19 Turn Type NA Perm Prot NA NA NA Protected Phases 2 3 8 4 4 4 4 4 4 4 4 4 4 <td>Satd. Flow (prot)</td> <td>1646</td> <td>1403</td> <td>1614</td> <td>1716</td> <td>3099</td> <td></td> <td></td>	Satd. Flow (prot)	1646	1403	1614	1716	3099		
Peak-hour factor, PHF 0.97 0.97 0.97 0.97 0.97 0.97 Adj, Flow (vph) 129 155 36 479 758 175 RTOR Reduction (vph) 0 116 0 0 15 0 Lane Group Flow (vph) 129 39 36 479 918 0 Confl. Peds. (#/hr) 15 15 50 50 50 Confl. Bikes (#/hr) 3 9 9 9 Heavy Vehicles (%) 1% 1% 3% 2% 0% 1% Turn Type NA Perm Prot NA NA Protected Phases 2 3 8 4 Permitted Phases 2 3 8 4	Flt Permitted	0.95	1.00	0.95	1.00	1.00		
Adj. Flow (vph) 129 155 36 479 758 175 RTOR Reduction (vph) 0 116 0 0 15 0 Lane Group Flow (vph) 129 39 36 479 918 0 Confl. Peds. (#hr) 15 15 50 50 50 Confl. Bikes (#hr) 3 9 9 14 </td <td>Satd. Flow (perm)</td> <td>1646</td> <td>1403</td> <td>1614</td> <td>1716</td> <td>3099</td> <td></td> <td></td>	Satd. Flow (perm)	1646	1403	1614	1716	3099		
RTOR Reduction (vph) 0	Peak-hour factor, PHF	0.97	0.97	0.97	0.97	0.97	0.97	
Lane Group Flow (vph) 129 39 36 479 918 0	Adj. Flow (vph)	129	155	36	479	758	175	
Confl. Peds. (#/hr) 15 15 50 50 Confl. Bikes (#/hr) 3 9 9 Heavy Vehicles (%) 1% 1% 3% 2% 0% 1% Turn Type NA Perm Prot NA NA Permeted Phases 2 3 8 4 Permitted Phases 2 2 3 8 4		0	116	0	0	15	0	
Confl. Bikes (#/hr) 3 y 9 Heavy Vehicles (%) 1% 1% 3% 2% 0% 1% Turn Type NA Perm Prot NA NA Protected Phases 2 3 8 4 Permitted Phases 2 30.0 5.7 82.0 72.3 Actuated Green, G (s) 30.0 30.0 5.7 82.0 72.3 Actuated g/C Ratio 0.25 0.25 0.05 0.68 0.60 Clearance Time (s) 4.0 4.0 4.0 4.0 4.0 Vehicle Extension (s) 3.0 3.0 3.0 3.0 3.0 Lane Grp Cap (vph) 411 350 76 1172 1867 v/s Ratio Prot c0.08 0.02 c0.28 c0.30 v/s Ratio Prot 0.08 0.02 c0.28 c0.30 V/s Ratio 0.31 0.11 0.47 0.41 0.49 Uniform Delay, d1 36.6 </td <td>Lane Group Flow (vph)</td> <td>129</td> <td>39</td> <td>36</td> <td>479</td> <td>918</td> <td>0</td> <td></td>	Lane Group Flow (vph)	129	39	36	479	918	0	
Heavy Vehicles (%) 1% 1% 3% 2% 0% 1% Turn Type NA Perm Prot NA NA Protected Phases 2 3 8 4 Permitted Phases 2 30.0 30.0 5.7 82.0 72.3 Actuated Green, G (s) 30.0 30.0 5.7 82.0 72.3 Actuated g/C Ratio 0.25 0.25 0.05 0.68 0.60 Clearance Time (s) 4.0 4.0 4.0 4.0 4.0 Vehicle Extension (s) 3.0 3.0 3.0 3.0 3.0 Lane Grp Cap (vph) 411 350 76 1172 1867 v/s Ratio Prot c0.08 0.02 c0.28 c0.30 v/s Ratio Perm 0.03 0.01 0.47 0.41 0.49 Uniform Delay, d1 36.6 34.7 55.7 8.3 13.5 Progression Factor 1.00 1.47 0.14 1.0	Confl. Peds. (#/hr)	15		50			50	
Turn Type	Confl. Bikes (#/hr)							
Protected Phases 2 3 8 4	Heavy Vehicles (%)		1%	3%	2%		1%	
Permitted Phases 2	Turn Type	NA	Perm	Prot	NA	NA		
Actuated Green, G (s) 30.0 30.0 5.7 82.0 72.3 Effective Green, g (s) 30.0 30.0 5.7 82.0 72.3 Actuated g/C Ratio 0.25 0.25 0.05 0.68 0.60 Clearance Time (s) 4.0 4.0 4.0 4.0 4.0 Vehicle Extension (s) 3.0 3.0 3.0 3.0 3.0 Lane Grp Cap (vph) 411 350 76 1172 1867 v/s Ratio Prot c0.08 0.02 c0.28 c0.30 v/s Ratio Perm 0.03 v/c Ratio 0.31 0.11 0.47 0.41 0.49 Uniform Delay, d1 36.6 34.7 55.7 8.3 13.5 Progression Factor 1.00 1.00 1.47 0.14 1.00 Incremental Delay, d2 2.0 0.6 3.4 0.8 0.9 Delay (s) 38.6 35.3 85.6 2.0 14.4 Level of Service D D D F A B Approach Delay (s) 36.8 7.8 14.4 Approach LOS D A B Intersection Summary HCM 2000 Control Delay 16.1 HCM 2000 Level of Service B HCM 2000 Volume to Capacity ratio 0.44 Actuated Cycle Length (s) 12.0 Intersection Capacity Utilization 51.6% ICU Level of Service A Analysis Period (min) 15	Protected Phases	2		3	8	4		
Effective Green, g (s) 30.0 30.0 5.7 82.0 72.3 Actuated g/C Ratio 0.25 0.25 0.05 0.68 0.60 Clearance Time (s) 4.0 4.0 4.0 4.0 Vehicle Extension (s) 3.0 3.0 3.0 3.0 Lane Grp Cap (vph) 411 350 76 1172 1867 V/s Ratio Prot c0.08 0.02 c0.28 c0.30 V/s Ratio Perm 0.03 0.02 c0.28 c0.30 V/s Ratio Perm 0.03 0.04 0.49 Uniform Delay, d1 36.6 34.7 55.7 8.3 13.5 Progression Factor 1.00 1.00 1.47 0.14 1.00 Incremental Delay, d2 2.0 0.6 3.4 0.8 0.9 Delay (s) 38.6 35.3 85.6 2.0 14.4 Level of Service D D F A B Approach LOS D								
Actuated g/C Ratio 0.25 0.25 0.05 0.68 0.60 Clearance Time (s) 4.0 4.0 4.0 4.0 4.0 Vehicle Extension (s) 3.0 3.0 3.0 3.0 3.0 Lane Grp Cap (vph) 411 350 76 1172 1867 v/s Ratio Prot c0.08 0.02 c0.28 c0.30 v/s Ratio Perm 0.03 v/c Ratio 0.31 0.11 0.47 0.41 0.49 Uniform Delay, d1 36.6 34.7 55.7 8.3 13.5 Progression Factor 1.00 1.00 1.47 0.14 1.00 Incremental Delay, d2 2.0 0.6 3.4 0.8 0.9 Delay (s) 38.6 35.3 85.6 2.0 14.4 Level of Service D D F A B Approach Delay (s) 36.8 7.8 14.4 Approach LOS D A B Intersection Summary HCM 2000 Control Delay 16.1 HCM 2000 Level of Service B HCM 2000 Volume to Capacity ratio 0.44 Actuated Cycle Length (s) 120.0 Sum of lost time (s) 12.0 Intersection Capacity Utilization 51.6% ICU Level of Service A Analysis Period (min)	Actuated Green, G (s)		30.0		82.0			
Clearance Time (s) 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 Vehicle Extension (s) 3.0								
Vehicle Extension (s) 3.0 3.0 3.0 3.0 3.0 3.0 Lane Grp Cap (vph) 411 350 76 1172 1867 v/s Ratio Prot c0.08 0.02 c0.28 c0.30 v/s Ratio Perm 0.03 0.04 0.41 0.49 Uniform Delay, d1 36.6 34.7 55.7 8.3 13.5 Progression Factor 1.00 1.00 1.47 0.14 1.00 Incremental Delay, d2 2.0 0.6 3.4 0.8 0.9 Delay (s) 38.6 35.3 85.6 2.0 14.4 Level of Service D D F A B Approach Delay (s) 36.8 7.8 14.4 Approach LOS D A B Intersection Summary B HCM 2000 Control Delay 16.1 HCM 2000 Level of Service B HCM 2000 Volume to Capacity ratio 0.44 Actuated Cycle Length (s) 12.0 Sum of lost time (s) 1								
Lane Grp Cap (vph) 411 350 76 1172 1867 v/s Ratio Prot c0.08 0.02 c0.28 c0.30 v/s Ratio Perm 0.03 v/c Ratio 0.31 0.11 0.47 0.41 0.49 Uniform Delay, d1 36.6 34.7 55.7 8.3 13.5 Progression Factor 1.00 1.00 1.47 0.14 1.00 Incremental Delay, d2 2.0 0.6 3.4 0.8 0.9 Delay (s) 38.6 35.3 85.6 2.0 14.4 Level of Service D D F A B Approach Delay (s) 36.8 7.8 14.4 A Approach LOS D A B B Intersection Summary B HCM 2000 Control Delay 16.1 HCM 2000 Level of Service B HCM 2000 Volume to Capacity ratio 0.44 A A ICU Level of Service A Analysis Period (min) 15	, ,							
v/s Ratio Prot c0.08 0.02 c0.28 c0.30 v/s Ratio Perm 0.03 0.11 0.47 0.41 0.49 Uniform Delay, d1 36.6 34.7 55.7 8.3 13.5 Progression Factor 1.00 1.00 1.47 0.14 1.00 Incremental Delay, d2 2.0 0.6 3.4 0.8 0.9 Delay (s) 38.6 35.3 85.6 2.0 14.4 Level of Service D D F A B Approach Delay (s) 36.8 7.8 14.4 Approach LOS D A B Intersection Summary B HCM 2000 Level of Service B HCM 2000 Volume to Capacity ratio 0.44 A Actuated Cycle Length (s) 12.0 Sum of lost time (s) 12.0 Intersection Capacity Utilization 51.6% ICU Level of Service A Analysis Period (min) 15 ICU Level of Service A	Vehicle Extension (s)	3.0	3.0		3.0	3.0		
v/s Ratio Perm 0.03 v/c Ratio 0.31 0.11 0.47 0.41 0.49 Uniform Delay, d1 36.6 34.7 55.7 8.3 13.5 Progression Factor 1.00 1.00 1.47 0.14 1.00 Incremental Delay, d2 2.0 0.6 3.4 0.8 0.9 Delay (s) 38.6 35.3 85.6 2.0 14.4 Level of Service D D F A B Approach Delay (s) 36.8 7.8 14.4 Approach LOS D A B Intersection Summary Intersection Summary Intersection Control Delay Intersection Summary Intersection Summary Intersection Summary Intersection Summary Intersection Summary Intersection Sum of lost time (s) Intersection Summary		411	350	76	1172	1867		
V/c Ratio 0.31 0.11 0.47 0.41 0.49 Uniform Delay, d1 36.6 34.7 55.7 8.3 13.5 Progression Factor 1.00 1.00 1.47 0.14 1.00 Incremental Delay, d2 2.0 0.6 3.4 0.8 0.9 Delay (s) 38.6 35.3 85.6 2.0 14.4 Level of Service D D F A B Approach Delay (s) 36.8 7.8 14.4 Approach LOS D A B Intersection Summary B HCM 2000 Level of Service B HCM 2000 Volume to Capacity ratio 0.44 Actuated Cycle Length (s) 12.0 Sum of lost time (s) 12.0 Intersection Capacity Utilization 51.6% ICU Level of Service A Analysis Period (min) 15	v/s Ratio Prot	c0.08		0.02	c0.28	c0.30		
Uniform Delay, d1 36.6 34.7 55.7 8.3 13.5 Progression Factor 1.00 1.00 1.47 0.14 1.00 Incremental Delay, d2 2.0 0.6 3.4 0.8 0.9 Delay (s) 38.6 35.3 85.6 2.0 14.4 Level of Service D D F A B Approach Delay (s) 36.8 7.8 14.4 Approach LOS D A B Intersection Summary B HCM 2000 Level of Service B HCM 2000 Volume to Capacity ratio 0.44 Actuated Cycle Length (s) 120.0 Sum of lost time (s) 12.0 Intersection Capacity Utilization 51.6% ICU Level of Service A Analysis Period (min) 15								
Progression Factor 1.00 1.47 0.14 1.00 Incremental Delay, d2 2.0 0.6 3.4 0.8 0.9 Delay (s) 38.6 35.3 85.6 2.0 14.4 Level of Service D D F A B Approach Delay (s) 36.8 7.8 14.4 Approach LOS D A B Intersection Summary B HCM 2000 Control Delay 16.1 HCM 2000 Level of Service B HCM 2000 Volume to Capacity ratio 0.44 Actuated Cycle Length (s) 120.0 Sum of lost time (s) 12.0 Intersection Capacity Utilization 51.6% ICU Level of Service A Analysis Period (min) 15								
Incremental Delay, d2								
Delay (s) 38.6 35.3 85.6 2.0 14.4 Level of Service D D F A B Approach Delay (s) 36.8 7.8 14.4 Approach LOS D A B Intersection Summary HCM 2000 Control Delay 16.1 HCM 2000 Level of Service B HCM 2000 Volume to Capacity ratio 0.44 Capacity Capacity (s) 120.0 Sum of lost time (s) 12.0 Intersection Capacity Utilization 51.6% ICU Level of Service A Analysis Period (min) 15			1.00		0.14			
Level of Service D D F A B Approach Delay (s) 36.8 7.8 14.4 Approach LOS D A B Intersection Summary HCM 2000 Control Delay 16.1 HCM 2000 Level of Service B HCM 2000 Volume to Capacity ratio 0.44 Actuated Cycle Length (s) 120.0 Sum of lost time (s) 12.0 Intersection Capacity Utilization 51.6% ICU Level of Service A Analysis Period (min) 15								
Approach Delay (s) 36.8 7.8 14.4 Approach LOS D A B Intersection Summary HCM 2000 Control Delay 16.1 HCM 2000 Level of Service B HCM 2000 Volume to Capacity ratio 0.44 Actuated Cycle Length (s) 120.0 Sum of lost time (s) 12.0 Intersection Capacity Utilization 51.6% ICU Level of Service A Analysis Period (min) 15				85.6		14.4		
Approach LOS D A B Intersection Summary HCM 2000 Control Delay 16.1 HCM 2000 Level of Service B HCM 2000 Volume to Capacity ratio 0.44 Actuated Cycle Length (s) 120.0 Sum of lost time (s) 12.0 Intersection Capacity Utilization 51.6% ICU Level of Service A Analysis Period (min) 15			D	F				
Intersection Summary HCM 2000 Control Delay HCM 2000 Control Delay HCM 2000 Volume to Capacity ratio Actuated Cycle Length (s) Intersection Capacity Utilization Analysis Period (min) 16.1 HCM 2000 Level of Service B HCM 2000 Level of Service B ICU Level of Service A								
HCM 2000 Control Delay HCM 2000 Control Delay HCM 2000 Volume to Capacity ratio Actuated Cycle Length (s) Intersection Capacity Utilization Analysis Period (min) 15 HCM 2000 Level of Service B HCM 2000 Level of Service B CU Level of Service A ICU Level of Service A	Approach LOS	D			А	В		
HCM 2000 Control Delay HCM 2000 Control Delay HCM 2000 Volume to Capacity ratio Actuated Cycle Length (s) Intersection Capacity Utilization Analysis Period (min) 15 HCM 2000 Level of Service B HCM 2000 Level of Service B CU Level of Service A ICU Level of Service A	Intersection Summary							
HCM 2000 Volume to Capacity ratio0.44Actuated Cycle Length (s)120.0Sum of lost time (s)12.0Intersection Capacity Utilization51.6%ICU Level of ServiceAAnalysis Period (min)15				16.1	Н	CM 2000	Level of Service	В
Actuated Cycle Length (s) 120.0 Sum of lost time (s) 12.0 Intersection Capacity Utilization 51.6% ICU Level of Service A Analysis Period (min) 15		city ratio				SIVI 2000	2010101001100	J
Intersection Capacity Utilization 51.6% ICU Level of Service A Analysis Period (min) 15		, 14110			Si	um of lost	time (s)	12.0
Analysis Period (min) 15		ion						
					10	. 5 250010	. 55, 1,65	, ,
	c Critical Lane Group							

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	†	7	ሻ	†	7	ሻ	4		*	↑ ↑	
Volume (vph)	120	280	330	120	370	70	275	310	45	155	575	105
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1500	1500	1500	1750	1750	1750
Lane Width	12	10	10	10	10	11	11	10	11	12	12	12
Total Lost time (s)	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5		4.5	4.5	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00		1.00	0.95	
Frpb, ped/bikes	1.00	1.00	0.92	1.00	1.00	0.83	1.00	0.99		1.00	0.98	
Flpb, ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00		1.00	1.00	
Frt	1.00	1.00	0.85	1.00	1.00	0.85	1.00	0.98		1.00	0.98	
Flt Protected	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00		0.95	1.00	
Satd. Flow (prot)	1662	1601	1262	1492	1617	1175	1364	1314		1646	3145	
Flt Permitted	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00		0.95	1.00	
Satd. Flow (perm)	1662	1601	1262	1492	1617	1175	1364	1314		1646	3145	
Peak-hour factor, PHF	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Adj. Flow (vph)	126	295	347	126	389	74	289	326	47	163	605	111
RTOR Reduction (vph)	0	0	196	0	0	54	0	3	0	0	12	0
Lane Group Flow (vph)	126	295	151	126	389	20	289	370	0	163	704	0
Confl. Peds. (#/hr)	45		30	30		45	40		25	25		40
Confl. Bikes (#/hr)			25			17			5			14
Heavy Vehicles (%)	0%	2%	1%	4%	1%	2%	1%	3%	5%	1%	1%	0%
Turn Type	Prot	NA	Perm	Prot	NA	Perm	Prot	NA		Prot	NA	
Protected Phases	5	2		1	6		3	8		7	4	
Permitted Phases			2			6						
Actuated Green, G (s)	13.3	31.4	31.4	14.0	32.1	32.1	27.1	40.3		16.3	29.5	
Effective Green, g (s)	13.3	31.4	31.4	14.0	32.1	32.1	27.1	40.3		16.3	29.5	
Actuated g/C Ratio	0.11	0.26	0.26	0.12	0.27	0.27	0.23	0.34		0.14	0.25	
Clearance Time (s)	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5		4.5	4.5	
Vehicle Extension (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0		2.0	2.0	
Lane Grp Cap (vph)	184	418	330	174	432	314	308	441		223	773	
v/s Ratio Prot	0.08	0.18		c0.08	c0.24		c0.21	0.28		0.10	c0.22	
v/s Ratio Perm			0.12			0.02						
v/c Ratio	0.68	0.71	0.46	0.72	0.90	0.06	0.94	0.84		0.73	0.91	
Uniform Delay, d1	51.3	40.1	37.2	51.1	42.4	32.7	45.6	36.8		49.7	44.0	
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00		1.21	0.90	
Incremental Delay, d2	8.1	4.4	0.4	11.9	21.0	0.0	34.6	17.1		9.2	15.5	
Delay (s)	59.5	44.5	37.5	63.0	63.4	32.8	80.2	54.0		69.6	55.1	
Level of Service	Е	D	D	Е	Е	С	F	D		Е	Е	
Approach Delay (s)		43.8			59.5			65.4			57.8	
Approach LOS		D			Е			Ε			Е	
Intersection Summary												
HCM 2000 Control Delay		56.2			CM 2000	Level of	Service		Е			
HCM 2000 Volume to Capa	apacity ratio 0.89											
Actuated Cycle Length (s)					um of los				18.0			
Intersection Capacity Utiliza					CU Level	of Service	;		E			
Analysis Period (min)			15									
c Critical Lane Group												

HCM Signalized Intersection Capacity Analysis

S. Willamette Street Corridor

6: S. Willamette & Donald St/Willamette Street & 32nd AV Peak - Alt 3&5 (Modified 3-lane) - Bus Pullouts

	•	/	•	*1	†	<i>></i>	>		لِر	†	<i>></i>	4
Movement	WBL2	WBL	WBR	NBL	NBT	NBR	SBL	SBT	SBR	NEL	NER	NER2
Lane Configurations		M			4			4	7	M		
Volume (vph)	5	10	20	5	245	5	30	320	615	365	5	5
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Lane Width	12	12	12	12	14	12	12	12	12	12	12	12
Total Lost time (s)		4.0			4.0			4.0	5.0	5.0		
Lane Util. Factor		1.00			1.00			1.00	1.00	1.00		
Frpb, ped/bikes		0.98			1.00			1.00	0.98	1.00		
Flpb, ped/bikes		1.00			1.00			1.00	1.00	1.00		
Frt		0.92			1.00			1.00	0.85	1.00		
Flt Protected		0.98			1.00			1.00	1.00	0.95		
Satd. Flow (prot)		1495			1824			1725	1449	1642		
Flt Permitted		0.98			0.99			0.96	1.00	1.00		
Satd. Flow (perm)		1495			1812			1662	1449	1722		
Peak-hour factor, PHF	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Adj. Flow (vph)	5	11	21	5	258	5	32	337	647	384	5	5
RTOR Reduction (vph)	0	28	0	0	1	0	0	0	0	11	0	0
Lane Group Flow (vph)	0	9	0	0	267	0	0	369	647	383	0	0
Confl. Peds. (#/hr)	5		5	5		15	15		5	5		5
Confl. Bikes (#/hr)			2			2			4			4
Heavy Vehicles (%)	0%	0%	6%	0%	2%	0%	0%	1%	1%	1%	0%	0%
Turn Type	Perm	NA		Perm	NA		Perm	NA	custom	NA		
Protected Phases	1 01111	8		1 01111	2		1 01111	6	ouston			
Permitted Phases	8			2	_		6	<u> </u>	3 6	3		
Actuated Green, G (s)	, in the second	10.0			15.9			15.9	39.4	14.5		
Effective Green, g (s)		10.0			15.9			15.9	35.4	14.5		
Actuated g/C Ratio		0.25			0.40			0.40	0.90	0.37		
Clearance Time (s)		4.0			4.0			4.0	0.70	5.0		
Vehicle Extension (s)		2.5			2.5			2.5		2.0		
Lane Grp Cap (vph)		379			731			670	1301	633		
v/s Ratio Prot		377			731			070	1301	033		
v/s Ratio Perm		0.01			0.15			0.22	c0.45	c0.22		
v/c Ratio		0.02			0.36			0.55	0.50	0.61		
Uniform Delay, d1		11.0			8.2			9.0	0.4	10.1		
Progression Factor		1.00			1.00			1.00	1.00	1.00		
Incremental Delay, d2		0.0			0.2			0.8	0.1	1.1		
Delay (s)		11.1			8.4			9.8	0.5	11.3		
Level of Service		В			A			Α.	A	В		
Approach Delay (s)		11.1			8.4			3.9	,,	11.3		
Approach LOS		В			A			Α		В		
Intersection Summary												
HCM 2000 Control Delay			6.4	Н	CM 2000	Level of 9	Service		A			
HCM 2000 Volume to Capa	city ratio		0.4	11	CIVI ZUUU	Level of v	JCI VICC					
Actuated Cycle Length (s)	ionly ratio		39.4	Ç,	um of lost	time (s)			11.0			
Intersection Capacity Utiliza	ation		72.6%		CU Level o				C C			
Analysis Period (min)	HUUTT		15	IC	O LEVEL	J SCI VICE						
c Critical Lane Group			10									
c Chilical Latte Group												

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4	7		र्स	7		4TÞ	
Volume (vph)	20	85	15	115	55	35	5	520	210	30	325	15
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Lane Width	12	14	12	12	12	12	12	12	12	12	12	12
Total Lost time (s)		4.5			4.5	4.5		4.5	4.5		4.5	
Lane Util. Factor		1.00			1.00	1.00		1.00	1.00		0.95	
Frpb, ped/bikes		1.00			1.00	0.91		1.00	0.88		1.00	
Flpb, ped/bikes		1.00			0.99	1.00		1.00	1.00		1.00	
Frt		0.98			1.00	0.85		1.00	0.85		0.99	
Flt Protected		0.99			0.97	1.00		1.00	1.00		1.00	
Satd. Flow (prot)		1797			1672	1359		1715	1278		3060	
Flt Permitted		0.94			0.73	1.00		1.00	1.00		0.89	
Satd. Flow (perm)		1695			1266	1359		1711	1278		2740	
Peak-hour factor, PHF	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Adj. Flow (vph)	22	93	16	126	60	38	5	571	231	33	357	16
RTOR Reduction (vph)	0	7	0	0	0	28	0	0	91	0	4	0
Lane Group Flow (vph)	0	124	0	0	186	10	0	576	140	0	402	0
Confl. Peds. (#/hr)	10		10	10		10	25		50	50		25
Confl. Bikes (#/hr)			5			35			5			5
Heavy Vehicles (%)	0%	1%	0%	0%	2%	0%	0%	2%	1%	4%	7%	13%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	4	0	0	0
Turn Type	Perm	NA		Perm	NA	Perm	Perm	NA	Perm	Perm	NA	
Protected Phases		2			6			8			4	
Permitted Phases	2			6		6	8		8	4		
Actuated Green, G (s)		18.7			18.7	18.7		42.3	42.3		42.3	
Effective Green, g (s)		18.7			18.7	18.7		42.3	42.3		42.3	
Actuated g/C Ratio		0.27			0.27	0.27		0.60	0.60		0.60	
Clearance Time (s)		4.5			4.5	4.5		4.5	4.5		4.5	
Vehicle Extension (s)		3.0			3.0	3.0		2.0	2.0		2.0	
Lane Grp Cap (vph)		452			338	363		1033	772		1655	
v/s Ratio Prot		0.07			0.45	0.04		0.04	0.44		0.45	
v/s Ratio Perm		0.07			c0.15	0.01		c0.34	0.11		0.15	
v/c Ratio		0.27			0.55	0.03		0.56	0.18		0.24	
Uniform Delay, d1		20.3			22.0	18.9		8.3	6.2		6.4	
Progression Factor		1.00			1.00	1.00		0.37	0.26		1.00	
Incremental Delay, d2		0.3			1.9	0.0		2.0	0.5		0.3	
Delay (s)		20.6			24.0	19.0		5.0	2.1		6.8	
Level of Service		C			C	В		A	А		A	
Approach LOS		20.6 C			23.1 C			4.2			6.8	
Approach LOS		C			C			Α			А	
Intersection Summary												
HCM 2000 Control Delay			8.9	H	CM 2000	Level of	Service		Α			
HCM 2000 Volume to Capac	ity ratio		0.55									
Actuated Cycle Length (s)			70.0	Sı	um of los	t time (s)			9.0			
Intersection Capacity Utilizati	on		71.3%	IC	U Level	of Service			С			
Analysis Period (min)			15									
c Critical Lane Group												

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4Th			414	
Volume (vph)	15	5	5	15	5	15	10	725	15	15	430	20
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Lane Width	12	14	12	12	14	12	12	12	12	12	12	12
Total Lost time (s)		4.5			4.5			4.5			4.5	
Lane Util. Factor		1.00			1.00			0.95			0.95	
Frpb, ped/bikes		0.99			0.98			1.00			1.00	
Flpb, ped/bikes		0.98			0.99			1.00			1.00	
Frt		0.97			0.94			1.00			0.99	
Flt Protected		0.97			0.98			1.00			1.00	
Satd. Flow (prot)		1704			1658			3244			3175	
Flt Permitted		0.87			0.91			0.95			0.92	
Satd. Flow (perm)		1532			1536			3075			2925	
Peak-hour factor, PHF	0.83	0.83	0.83	0.83	0.83	0.83	0.83	0.83	0.83	0.83	0.83	0.83
Adj. Flow (vph)	18	6	6	18	6	18	12	873	18	18	518	24
RTOR Reduction (vph)	0	4	0	0	13	0	0	2	0	0	4	0
Lane Group Flow (vph)	0	26	0	0	29	0	0	901	0	0	556	0
Confl. Peds. (#/hr)	35		25	25		35	15		30	30		15
Confl. Bikes (#/hr)			5			5			10			10
Heavy Vehicles (%)	0%	1%	0%	0%	1%	0%	0%	1%	8%	0%	3%	0%
Bus Blockages (#/hr)	0	0	4	0	0	4	0	4	0	0	4	0
Turn Type	Perm	NA		Perm	NA		Perm	NA		Perm	NA	
Protected Phases		2			6			4			8	
Permitted Phases	2			6			4			8		
Actuated Green, G (s)		18.0			18.0			43.0			43.0	
Effective Green, g (s)		18.0			18.0			43.0			43.0	
Actuated g/C Ratio		0.26			0.26			0.61			0.61	
Clearance Time (s)		4.5			4.5			4.5			4.5	
Vehicle Extension (s)		3.0			3.0			2.0			2.0	
Lane Grp Cap (vph)		393			394			1888			1796	
v/s Ratio Prot												
v/s Ratio Perm		0.02			c0.02			c0.29			0.19	
v/c Ratio		0.06			0.07			0.48			0.31	
Uniform Delay, d1		19.6			19.7			7.4			6.4	
Progression Factor		1.00			1.00			0.62			1.03	
Incremental Delay, d2		0.1			0.1			0.8			0.4	
Delay (s)		19.7			19.8			5.3			7.1	
Level of Service		B			B			A			A	
Approach LOS		19.7			19.8 B			5.3 A			7.1 A	
Approach LOS		В			Ď			А			А	
Intersection Summary												
HCM 2000 Control Delay			6.6	H	CM 2000	Level of S	Service		Α			
HCM 2000 Volume to Capac	ity ratio		0.36									
Actuated Cycle Length (s)			70.0		um of lost				9.0			
Intersection Capacity Utilizati	on		52.5%	IC	U Level o	of Service	:		Α			
Analysis Period (min)			15									
c Critical Lane Group												

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	Ŋ.	1>		¥	₽			4Th			4Th	
Volume (vph)	25	45	30	25	35	15	20	695	35	15	395	15
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Total Lost time (s)	4.5	4.5		4.5	4.5			4.5			4.5	
Lane Util. Factor	1.00	1.00		1.00	1.00			0.95			0.95	
Frpb, ped/bikes	1.00	0.99		1.00	0.99			1.00			1.00	
Flpb, ped/bikes	0.99	1.00		0.98	1.00			1.00			1.00	
Frt	1.00	0.94		1.00	0.96			0.99			0.99	
Flt Protected	0.95	1.00		0.95	1.00			1.00			1.00	
Satd. Flow (prot)	1581	1574		1637	1655			3024			2920	
Flt Permitted	0.72	1.00		0.70	1.00			0.94			0.92	
Satd. Flow (perm)	1198	1574		1208	1655			2837			2694	
Peak-hour factor, PHF	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87
Adj. Flow (vph)	29	52	34	29	40	17	23	799	40	17	454	17
RTOR Reduction (vph)	0	27	0	0	14	0	0	4	0	0	3	0
Lane Group Flow (vph)	29	59	0	29	43	0	0	858	0	0	485	0
Confl. Peds. (#/hr)	10		15	15		10	10		40	40		10
Confl. Bikes (#/hr)			5			10			5			10
Heavy Vehicles (%)	4%	0%	8%	0%	0%	0%	0%	1%	0%	0%	5%	8%
Bus Blockages (#/hr)	0	0	4	0	0	4	0	4	0	0	4	0
Parking (#/hr)							5	5	5	5	5	5
Turn Type	Perm	NA		Perm	NA		Perm	NA		Perm	NA	
Protected Phases		2			6			4			8	
Permitted Phases	2			6			4			8		
Actuated Green, G (s)	14.4	14.4		14.4	14.4			46.6			46.6	
Effective Green, g (s)	14.4	14.4		14.4	14.4			46.6			46.6	
Actuated g/C Ratio	0.21	0.21		0.21	0.21			0.67			0.67	
Clearance Time (s)	4.5	4.5		4.5	4.5			4.5			4.5	
Vehicle Extension (s)	3.0	3.0		3.0	3.0			2.0			2.0	
Lane Grp Cap (vph)	246	323		248	340			1888			1793	
v/s Ratio Prot		c0.04			0.03							
v/s Ratio Perm	0.02			0.02				c0.30			0.18	
v/c Ratio	0.12	0.18		0.12	0.13			0.45			0.27	
Uniform Delay, d1	22.6	22.9		22.6	22.7			5.6			4.8	
Progression Factor	1.00	1.00		1.00	1.00			1.00			1.00	
Incremental Delay, d2	0.2	0.3		0.2	0.2			0.8			0.4	
Delay (s)	22.8	23.2		22.8	22.8			6.4			5.1	
Level of Service	С	С		С	С			Α			Α	
Approach Delay (s)		23.1			22.8			6.4			5.1	
Approach LOS		С			С			А			Α	
Intersection Summary												
HCM 2000 Control Delay			8.1	H	CM 2000	Level of :	Service		А			
HCM 2000 Volume to Capac	city ratio		0.39									
Actuated Cycle Length (s)			70.0	Sı	um of lost	time (s)			9.0			
Intersection Capacity Utilizat	tion		60.5%		U Level c		:		В			
Analysis Period (min)			15									
c Critical Lane Group												

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Movement	EBL	EBR	NBL	NBT	SBT	SBR	
Lane Configurations	**	7		4₽	† 1>		
Volume (vph)	50	20	20	685	340	55	
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	
Total Lost time (s)	4.0	4.0		4.0	4.0		
Lane Util. Factor	1.00	1.00		0.95	0.95		
Frpb, ped/bikes	1.00	0.94		1.00	0.99		
Flpb, ped/bikes	1.00	1.00		1.00	1.00		
Frt	1.00	0.85		1.00	0.98		
Flt Protected	0.95	1.00		1.00	1.00		
Satd. Flow (prot)	1614	1397		3258	3107		
Flt Permitted	0.95	1.00		0.93	1.00		
Satd. Flow (perm)	1614	1397		3045	3107		
Peak-hour factor, PHF	0.87	0.87	0.87	0.87	0.87	0.87	
Adj. Flow (vph)	57	23	23	787	391	63	
RTOR Reduction (vph)	0	18	0	0	11	0	
Lane Group Flow (vph)	57	5	0	810	443	0	
Confl. Peds. (#/hr)	20	20	20			20	
Confl. Bikes (#/hr)		5				5	
Heavy Vehicles (%)	3%	0%	0%	1%	3%	0%	
Bus Blockages (#/hr)	0	0	0	4	4	0	
Turn Type	NA	Perm	Perm	NA	NA		
Protected Phases	2			8	4		
Permitted Phases		2	8				
Actuated Green, G (s)	25.0	25.0		87.0	87.0		
Effective Green, g (s)	25.0	25.0		87.0	87.0		
Actuated g/C Ratio	0.21	0.21		0.72	0.72		
Clearance Time (s)	4.0	4.0		4.0	4.0		
Lane Grp Cap (vph)	336	291		2207	2252		
v/s Ratio Prot	c0.04				0.14		
v/s Ratio Perm		0.00		c0.27			
v/c Ratio	0.17	0.02		0.37	0.20		
Uniform Delay, d1	39.0	37.7		6.2	5.3		
Progression Factor	1.00	1.00		0.25	1.00		
Incremental Delay, d2	1.1	0.1		0.4	0.2		
Delay (s)	40.1	37.8		2.0	5.5		
Level of Service	D	D		A	A		
Approach Delay (s)	39.4			2.0	5.5		
Approach LOS	D			A	A		
Intersection Summary	_						
HCM 2000 Control Delay			5.4	Н	CM 2000	Level of Service	
HCM 2000 Volume to Capac	ity ratio		0.32		ON 2000	LOVEL OF SCIVICE	
Actuated Cycle Length (s)	ity ratio		120.0	Şı	um of lost	time (s)	
Intersection Capacity Utilizat	ion		56.2%		U Level c		
Analysis Period (min)	1011		15	IC	O LOVEI C	JUI VICE	
c Critical Lang Group			13				

c Critical Lane Group

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	†	7	ሻ	†	7	Ĭ	∱ Ъ		7	∱ Ъ	
Volume (vph)	105	335	185	50	230	60	315	570	60	80	225	40
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Lane Width	12	10	10	10	10	11	11	10	11	12	12	12
Total Lost time (s)	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5		4.5	4.5	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95		1.00	0.95	
Frpb, ped/bikes	1.00	1.00	0.97	1.00	1.00	0.94	1.00	0.99		1.00	0.98	
Flpb, ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00		1.00	1.00	
Frt	1.00	1.00	0.85	1.00	1.00	0.85	1.00	0.99		1.00	0.98	
Flt Protected	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00		0.95	1.00	
Satd. Flow (prot)	1630	1601	1249	1349	1586	1351	1545	2977		1502	2992	
Flt Permitted	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00		0.95	1.00	
Satd. Flow (perm)	1630	1601	1249	1349	1586	1351	1545	2977		1502	2992	
Peak-hour factor, PHF	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88
Adj. Flow (vph)	119	381	210	57	261	68	358	648	68	91	256	45
RTOR Reduction (vph)	0	0	80	0	0	52	0	6	0	0	11	0
Lane Group Flow (vph)	119	381	130	57	261	16	358	710	0	91	290	0
Confl. Peds. (#/hr)	15		20	20		15	30		20	20		30
Confl. Bikes (#/hr)			25			25			10			10
Heavy Vehicles (%)	2%	2%	8%	15%	3%	0%	4%	1%	11%	8%	6%	11%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	10	6	0	0
Turn Type	Prot	NA	pm+ov	Prot	NA	Perm	Prot	NA		Prot	NA	
Protected Phases	5	2	3	1	6		3	8		7	4	
Permitted Phases			2			6						
Actuated Green, G (s)	11.4	32.8	63.9	6.0	27.4	27.4	31.1	52.1		11.1	32.1	
Effective Green, g (s)	11.4	32.8	63.9	6.0	27.4	27.4	31.1	52.1		11.1	32.1	
Actuated g/C Ratio	0.10	0.27	0.53	0.05	0.23	0.23	0.26	0.43		0.09	0.27	
Clearance Time (s)	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5		4.5	4.5	
Vehicle Extension (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0		2.0	2.0	
Lane Grp Cap (vph)	154	437	711	67	362	308	400	1292		138	800	
v/s Ratio Prot	c0.07	c0.24	0.05	0.04	0.16		c0.23	c0.24		0.06	0.10	
v/s Ratio Perm			0.06			0.01						
v/c Ratio	0.77	0.87	0.18	0.85	0.72	0.05	0.90	0.55		0.66	0.36	
Uniform Delay, d1	53.0	41.6	14.5	56.6	42.8	36.1	42.9	25.2		52.6	35.7	
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00		1.22	0.89	
Incremental Delay, d2	19.4	16.7	0.0	59.3	5.9	0.0	21.2	1.7		8.3	1.3	
Delay (s)	72.4	58.3	14.6	115.9	48.7	36.2	64.1	26.9		72.6	32.9	
Level of Service	E	E	В	F	D	D	Е	С		E	С	
Approach Delay (s)		47.7			56.4			39.3			42.1	
Approach LOS		D			E			D			D	
Intersection Summary												
HCM 2000 Control Delay			44.6	Н	CM 2000	Level of	Service		D			
HCM 2000 Volume to Capa	city ratio		0.80									
Actuated Cycle Length (s)			120.0		um of los				18.0			
Intersection Capacity Utiliza	ition		73.9%	IC	CU Level	of Service	!		D			
Analysis Period (min)			15									
c Critical Lane Group												

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Movement	WBL2	WBL	WBR	NBL	NBT	NBR	SBL	SBT	SBR	NEL	NER	NER2
Lane Configurations		M			4			4	7	M		
Volume (vph)	5	5	20	5	300	5	10	195	280	630	5	5
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Lane Width	12	12	12	12	14	12	12	12	12	12	12	12
Total Lost time (s)		4.0			4.0			4.0	5.0	5.0		
Lane Util. Factor		1.00			1.00			1.00	1.00	1.00		
Frpb, ped/bikes		0.98			1.00			1.00	0.98	1.00		
Flpb, ped/bikes		1.00			1.00			1.00	1.00	0.99		
Frt		0.91			1.00			1.00	0.85	1.00		
Flt Protected		0.98			1.00			1.00	1.00	0.95		
Satd. Flow (prot)		1535			1790			1666	1398	1623		
Flt Permitted		0.98			0.99			0.98	1.00	1.00		
Satd. Flow (perm)		1535			1783			1631	1398	1703		
Peak-hour factor, PHF	0.84	0.84	0.84	0.84	0.84	0.84	0.84	0.84	0.84	0.84	0.84	0.84
Adj. Flow (vph)	6	6	24	6	357	6	12	232	333	750	6	6
RTOR Reduction (vph)	0	20	0	0	1	0	0	0	0	6	0	0
Lane Group Flow (vph)	0	16	0	0	368	0	0	244	333	756	0	0
Confl. Peds. (#/hr)	5		5	10	000	10	10		10	5		5
Confl. Bikes (#/hr)			2	. •		2			4	· ·		4
Heavy Vehicles (%)	0%	0%	0%	0%	4%	0%	0%	5%	4%	2%	0%	0%
Turn Type	Perm	NA	070	Perm	NA	070	Perm	NA	custom	NA	070	070
Protected Phases	T CITII	8		1 Cilli	2		1 CIIII	6	Custom	147 (
Permitted Phases	8	U		2	2		6	U	3 6	3		
Actuated Green, G (s)	U	26.0			17.4		U	17.4	56.9	30.5		
Effective Green, g (s)		26.0			17.4			17.4	52.9	30.5		
Actuated g/C Ratio		0.46			0.31			0.31	0.93	0.54		
Clearance Time (s)		4.0			4.0			4.0	0.75	5.0		
Vehicle Extension (s)		2.5			2.5			2.5		2.0		
Lane Grp Cap (vph)		701			545			498	1299	912		
v/s Ratio Prot		701			343			470	1277	712		
v/s Ratio Perm		0.01			c0.21			0.15	0.24	c0.44		
v/c Ratio		0.01			0.68			0.13	0.24	0.83		
Uniform Delay, d1		8.5			17.3			16.1	0.20	11.0		
Progression Factor		1.00			1.00			1.00	1.00	1.00		
Incremental Delay, d2		0.0			3.0			0.6	0.0	6.0		
Delay (s)		8.5			20.3			16.7	0.0	17.0		
Level of Service		6.5 A			20.3 C			10.7 B	0.2 A	17.0 B		
Approach Delay (s)		8.5			20.3			7.2	A	17.0		
Approach LOS		6.5 A			20.3 C			7.2 A		17.0 B		
Intersection Summary												
HCM 2000 Control Delay			14.3	Н	CM 2000	Level of	Service		В			
HCM 2000 Volume to Capaci	tv ratio		0.81									
Actuated Cycle Length (s)	,		56.9	Sı	um of lost	time (s)			11.0			
Intersection Capacity Utilization	on		74.6%		CU Level of		<u> </u>		D			
Analysis Period (min)			15									
c Critical Lane Group												

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4	7	ሻ	f)		ሻ	f)	
Volume (vph)	20	85	15	115	55	35	5	520	210	30	325	15
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Lane Width	12	14	12	12	12	12	12	12	12	12	12	12
Total Lost time (s)		4.5			4.5	4.5	4.5	4.5		4.5	4.5	
Lane Util. Factor		1.00			1.00	1.00	1.00	1.00		1.00	1.00	
Frpb, ped/bikes		0.99			1.00	0.91	1.00	0.97		1.00	1.00	
Flpb, ped/bikes		1.00			0.99	1.00	0.97	1.00		0.98	1.00	
Frt		0.98			1.00	0.85	1.00	0.96		1.00	0.99	
Flt Protected		0.99			0.97	1.00	0.95	1.00		0.95	1.00	
Satd. Flow (prot)		1793			1665	1356	1618	1590		1572	1616	
Flt Permitted		0.93			0.73	1.00	0.51	1.00		0.22	1.00	
Satd. Flow (perm)		1690			1258	1356	864	1590		358	1616	
Peak-hour factor, PHF	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Adj. Flow (vph)	22	93	16	126	60	38	5	571	231	33	357	16
RTOR Reduction (vph)	0	7	0	0	0	28	0	21	0	0	2	0
Lane Group Flow (vph)	0	124	0	0	186	10	5	781	0	33	371	0
Confl. Peds. (#/hr)	10		10	10		10	25		50	50		25
Confl. Bikes (#/hr)			5			35			5			5
Heavy Vehicles (%)	0%	1%	0%	0%	2%	0%	0%	2%	1%	4%	7%	13%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	4	0	0	0
Turn Type	Perm	NA		Perm	NA	Perm	Perm	NA		Perm	NA	
Protected Phases		2			6			8			4	
Permitted Phases	2			6		6	8			4		
Actuated Green, G (s)		18.1			18.1	18.1	42.9	42.9		42.9	42.9	
Effective Green, g (s)		18.1			18.1	18.1	42.9	42.9		42.9	42.9	
Actuated g/C Ratio		0.26			0.26	0.26	0.61	0.61		0.61	0.61	
Clearance Time (s)		4.5			4.5	4.5	4.5	4.5		4.5	4.5	
Vehicle Extension (s)		3.0			3.0	3.0	2.0	2.0		2.0	2.0	
Lane Grp Cap (vph)		436			325	350	529	974		219	990	
v/s Ratio Prot		0.07			0.45	0.04	0.01	c0.49		0.00	0.23	
v/s Ratio Perm		0.07			c0.15	0.01	0.01	0.00		0.09	0.07	
v/c Ratio		0.28			0.57	0.03	0.01	0.80		0.15	0.37	
Uniform Delay, d1		20.8			22.6	19.4	5.3	10.3		5.8	6.8	
Progression Factor		1.00			1.00	1.00	0.86	0.62		1.00	1.00	
Incremental Delay, d2		0.4			2.4	0.0	0.0	3.9		1.5	1.1	
Delay (s)		21.1 C			25.0	19.4	4.6	10.2		7.2	7.9	
Level of Service		21.1			C 24.1	В	А	B 10.2		А	A 7.8	
Approach LOS		21.1 C			24.1 C							
Approach LOS		C			C			В			А	
Intersection Summary												
HCM 2000 Control Delay			12.5	Н	CM 2000	Level of	Service		В			
HCM 2000 Volume to Capaci	ity ratio		0.73									
Actuated Cycle Length (s)			70.0	S	um of los	t time (s)			9.0			
Intersection Capacity Utilizati	on		86.0%	IC	CU Level	of Service)		Е			
Analysis Period (min)			15									
c Critical Lane Group												

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4		۲	f)		ሻ	f)	
Volume (vph)	15	5	5	15	5	15	10	725	15	15	430	20
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Lane Width	12	14	12	12	14	12	12	12	12	12	12	12
Total Lost time (s)		4.5			4.5		4.5	4.5		4.5	4.5	
Lane Util. Factor		1.00			1.00		1.00	1.00		1.00	1.00	
Frpb, ped/bikes		0.99			0.96		1.00	1.00		1.00	1.00	
Flpb, ped/bikes		0.96			0.98		0.99	1.00		1.00	1.00	
Frt		0.97			0.94		1.00	1.00		1.00	0.99	
Flt Protected		0.97			0.98		0.95	1.00		0.95	1.00	
Satd. Flow (prot)		1667			1618		1644	1695		1662	1659	
Flt Permitted		0.87			0.91		0.39	1.00		0.16	1.00	
Satd. Flow (perm)		1499			1499		667	1695		286	1659	
Peak-hour factor, PHF	0.83	0.83	0.83	0.83	0.83	0.83	0.83	0.83	0.83	0.83	0.83	0.83
Adj. Flow (vph)	18	6	6	18	6	18	12	873	18	18	518	24
RTOR Reduction (vph)	0	4	0	0	13	0	0	1	0	0	2	0
Lane Group Flow (vph)	0	26	0	0	29	0	12	890	0	18	540	0
Confl. Peds. (#/hr)	35		25	25		35	15		30	30		15
Confl. Bikes (#/hr)			5			5			10			10
Heavy Vehicles (%)	0%	1%	0%	0%	1%	0%	0%	1%	8%	0%	3%	0%
Bus Blockages (#/hr)	0	0	4	0	0	4	0	4	0	0	4	0
Turn Type	Perm	NA		Perm	NA		Perm	NA		Perm	NA	
Protected Phases		2			6			4			8	
Permitted Phases	2			6			4			8		
Actuated Green, G (s)		18.0			18.0		43.0	43.0		43.0	43.0	
Effective Green, g (s)		18.0			18.0		43.0	43.0		43.0	43.0	
Actuated g/C Ratio		0.26			0.26		0.61	0.61		0.61	0.61	
Clearance Time (s)		4.5			4.5		4.5	4.5		4.5	4.5	
Vehicle Extension (s)		3.0			3.0		2.0	2.0		2.0	2.0	
Lane Grp Cap (vph)		385			385		409	1041		175	1019	
v/s Ratio Prot								c0.53			0.33	
v/s Ratio Perm		0.02			c0.02		0.02			0.06		
v/c Ratio		0.07			0.07		0.03	0.85		0.10	0.53	
Uniform Delay, d1		19.6			19.7		5.3	11.0		5.6	7.7	
Progression Factor		1.00			1.00		1.11	0.68		1.11	0.87	
Incremental Delay, d2		0.1			0.1		0.1	6.2		1.1	1.9	
Delay (s)		19.7			19.8		6.0	13.6		7.3	8.6	
Level of Service		В			В		Α	В		Α	Α	
Approach Delay (s)		19.7			19.8			13.5			8.5	
Approach LOS		В			В			В			А	
Intersection Summary												
HCM 2000 Control Delay			12.0	Н	CM 2000	Level of S	Service		В			
HCM 2000 Volume to Capaci	ty ratio		0.62									
Actuated Cycle Length (s)			70.0	Sı	um of lost	time (s)			9.0			
Intersection Capacity Utilization	on		65.0%		U Level o		:		С			
Analysis Period (min)			15									
c Critical Lane Group												

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ħ	₽		ሻ	₽		Ŋ	ĵ÷		ሻ	f)	
Volume (vph)	25	45	30	25	35	15	20	695	35	15	395	15
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Total Lost time (s)	4.5	4.5		4.5	4.5		4.5	4.5		4.5	4.5	
Lane Util. Factor	1.00	1.00		1.00	1.00		1.00	1.00		1.00	1.00	
Frpb, ped/bikes	1.00	0.98		1.00	0.98		1.00	1.00		1.00	1.00	
Flpb, ped/bikes	0.98	1.00		0.97	1.00		0.99	1.00		0.99	1.00	
Frt	1.00	0.94		1.00	0.96		1.00	0.99		1.00	0.99	
Flt Protected	0.95	1.00		0.95	1.00		0.95	1.00		0.95	1.00	
Satd. Flow (prot)	1570	1560		1619	1643		1442	1475		1435	1423	
Flt Permitted	0.72	1.00		0.70	1.00		0.45	1.00		0.23	1.00	
Satd. Flow (perm)	1189	1560		1195	1643		687	1475		348	1423	
Peak-hour factor, PHF	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87	0.87
Adj. Flow (vph)	29	52	34	29	40	17	23	799	40	17	454	17
RTOR Reduction (vph)	0	27	0	0	14	0	0	2	0	0	2	0
Lane Group Flow (vph)	29	59	0	29	43	0	23	837	0	17	469	0
Confl. Peds. (#/hr)	10		15	15		10	10		40	40		10
Confl. Bikes (#/hr)			5			10			5			10
Heavy Vehicles (%)	4%	0%	8%	0%	0%	0%	0%	1%	0%	0%	5%	8%
Bus Blockages (#/hr)	0	0	4	0	0	4	0	4	0	0	4	0
Parking (#/hr)							5	5	5	5	5	5
Turn Type	Perm	NA		Perm	NA		Perm	NA		Perm	NA	
Protected Phases		2			6			4			8	
Permitted Phases	2			6			4			8		
Actuated Green, G (s)	14.4	14.4		14.4	14.4		46.6	46.6		46.6	46.6	
Effective Green, g (s)	14.4	14.4		14.4	14.4		46.6	46.6		46.6	46.6	
Actuated g/C Ratio	0.21	0.21		0.21	0.21		0.67	0.67		0.67	0.67	
Clearance Time (s)	4.5	4.5		4.5	4.5		4.5	4.5		4.5	4.5	
Vehicle Extension (s)	3.0	3.0		3.0	3.0		2.0	2.0		2.0	2.0	
Lane Grp Cap (vph)	244	320		245	337		457	981		231	947	
v/s Ratio Prot		c0.04			0.03			c0.57			0.33	
v/s Ratio Perm	0.02			0.02			0.03			0.05		
v/c Ratio	0.12	0.18		0.12	0.13		0.05	0.85		0.07	0.50	
Uniform Delay, d1	22.6	23.0		22.6	22.7		4.0	9.0		4.1	5.8	
Progression Factor	1.00	1.00		1.00	1.00		1.00	1.00		1.42	1.27	
Incremental Delay, d2	0.2	0.3		0.2	0.2		0.2	9.3		0.5	1.6	
Delay (s)	22.9	23.2		22.8	22.9		4.3	18.4		6.4	9.0	
Level of Service	С	С		С	С		Α	В		Α	Α	
Approach Delay (s)		23.1			22.9			18.0			8.9	
Approach LOS		С			С			В			Α	
Intersection Summary												
HCM 2000 Control Delay			15.8	Н	CM 2000	Level of	Service		В			
HCM 2000 Volume to Capac	city ratio		0.69									
Actuated Cycle Length (s)	, 		70.0	Sı	um of lost	time (s)			9.0			
Intersection Capacity Utilizat	tion		64.7%		U Level o				С			
Analysis Period (min)			15									
c Critical Lane Group												

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Movement	EBL	EBR	NBL	NBT	SBT	SBR			
Lane Configurations	*	7	ሻ	↑	† 1>	02.1			
Volume (vph)	50	20	20	685	340	55			
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750			
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	1700			
Lane Util. Factor	1.00	1.00	1.00	1.00	0.95				
Frpb, ped/bikes	1.00	0.94	1.00	1.00	0.99				
Flpb, ped/bikes	1.00	1.00	1.00	1.00	1.00				
Frt	1.00	0.85	1.00	1.00	0.98				
Flt Protected	0.95	1.00	0.95	1.00	1.00				
Satd. Flow (prot)	1614	1394	1662	1705	3106				
Flt Permitted	0.95	1.00	0.95	1.00	1.00				
Satd. Flow (perm)	1614	1394	1662	1705	3106				
Peak-hour factor, PHF	0.87	0.87	0.87	0.87	0.87	0.87			
Adj. Flow (vph)	57	23	23	787	391	63			
RTOR Reduction (vph)	0	20	0	0	10	0			
Lane Group Flow (vph)	57	3	23	787	444	0			
Confl. Peds. (#/hr)	20	20	20	707	444	20			
Confl. Bikes (#/hr)	20	5	20			5			
Heavy Vehicles (%)	3%	0%	0%	1%	3%	0%			
Bus Blockages (#/hr)	0	0 %	0 %	4	4	0			
						U			
Turn Type	NA 2	Perm	Prot	NA	NA				
Protected Phases	2	2	3	8	4				
Permitted Phases	10.0		1 /	04.0	OF 4				
Actuated Green, G (s)	18.0	18.0	4.6	94.0	85.4 85.4				
Effective Green, g (s)	18.0 0.15	18.0	4.6 0.04	94.0	0.71				
Actuated g/C Ratio		0.15		0.78					
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0				
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0				
Lane Grp Cap (vph)	242	209	63	1335	2210				
v/s Ratio Prot	c0.04	2.00	0.01	c0.46	0.14				
v/s Ratio Perm	0.04	0.00	0.07	0.50	0.00				
v/c Ratio	0.24	0.02	0.37	0.59	0.20				
Uniform Delay, d1	44.9	43.5	56.3	5.2	5.8				
Progression Factor	1.00	1.00	1.39	0.06	1.00				
Incremental Delay, d2	2.3	0.1	1.7	0.9	0.2				
Delay (s)	47.2	43.6	79.9	1.2	6.0				
Level of Service	D	D	E	A	A				
Approach Delay (s)	46.2			3.4	6.0				
Approach LOS	D			Α	Α				
Intersection Summary									
HCM 2000 Control Delay			6.9	Н	CM 2000	Level of Service		A	
HCM 2000 Volume to Capa	acity ratio		0.55						
Actuated Cycle Length (s)	,		120.0	Sı	um of lost	time (s)	12.	0	
Intersection Capacity Utiliza	ation		59.1%		CU Level o			В	
Analysis Period (min)			15						
c Critical Lane Group									

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	†	7	ሻ	†	7	ሻ	f)		ሻ	∱ ∱	
Volume (vph)	105	335	185	50	230	60	315	570	60	80	225	40
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Lane Width	12	10	10	10	10	11	11	10	11	12	12	12
Total Lost time (s)	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5		4.5	4.5	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00		1.00	0.95	
Frpb, ped/bikes	1.00	1.00	0.93	1.00	1.00	0.90	1.00	0.99		1.00	0.98	
Flpb, ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00		1.00	1.00	
Frt	1.00	1.00	0.85	1.00	1.00	0.85	1.00	0.99		1.00	0.98	
Flt Protected	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00		0.95	1.00	
Satd. Flow (prot)	1630	1601	1200	1349	1586	1289	1545	1567		1502	2992	
Flt Permitted	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00		0.95	1.00	
Satd. Flow (perm)	1630	1601	1200	1349	1586	1289	1545	1567		1502	2992	
Peak-hour factor, PHF	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88
Adj. Flow (vph)	119	381	210	57	261	68	358	648	68	91	256	45
RTOR Reduction (vph)	0	0	102	0	0	53	0	3	0	0	11	0
Lane Group Flow (vph)	119	381	108	57	261	15	358	713	0	91	290	0
Confl. Peds. (#/hr)	15		20	20		15	30		20	20		30
Confl. Bikes (#/hr)			25			25			10			10
Heavy Vehicles (%)	2%	2%	8%	15%	3%	0%	4%	1%	11%	8%	6%	11%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	10	6	0	0
Turn Type	Prot	NA	Perm	Prot	NA	Perm	Prot	NA		Prot	NA	
Protected Phases	5	2		1	6		3	8		7	4	
Permitted Phases			2			6						
Actuated Green, G (s)	9.5	29.8	29.8	5.5	25.8	25.8	32.1	58.5		8.2	34.6	
Effective Green, g (s)	9.5	29.8	29.8	5.5	25.8	25.8	32.1	58.5		8.2	34.6	
Actuated g/C Ratio	0.08	0.25	0.25	0.05	0.22	0.22	0.27	0.49		0.07	0.29	
Clearance Time (s)	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5		4.5	4.5	
Vehicle Extension (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0		2.0	2.0	
Lane Grp Cap (vph)	129	397	298	61	340	277	413	763		102	862	
v/s Ratio Prot	c0.07	c0.24	0.00	0.04	0.16	0.04	c0.23	c0.46		0.06	0.10	
v/s Ratio Perm	0.00	0.07	0.09	0.00		0.01	0.07	0.00		0.00	0.04	
v/c Ratio	0.92	0.96	0.36	0.93	0.77	0.05	0.87	0.93		0.89	0.34	
Uniform Delay, d1	54.9	44.5	37.2	57.1	44.3	37.4	41.9	28.9		55.5	33.7	
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00		1.01	0.85	
Incremental Delay, d2	55.1	34.1	0.3	90.4	9.0	0.0	16.6	20.0		54.6	1.0	
Delay (s)	110.0	78.6	37.5	147.5	53.3	37.4	58.5	49.0		110.5	29.7	
Level of Service	F	E 71.7	D	F	D	D	Е	D		F	C	
Approach LOS		71.7			64.4			52.1			48.5	
Approach LOS		E			Е			D			D	
Intersection Summary												
HCM 2000 Control Delay			58.9	H	CM 2000	Level of	Service		Е			
HCM 2000 Volume to Capa	city ratio		0.97									
Actuated Cycle Length (s)			120.0		um of lost				18.0			
Intersection Capacity Utiliza	tion		79.0%	IC	:U Level	of Service	<u> </u>		D			
Analysis Period (min)			15									
c Critical Lane Group												

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Movement	WBL2	WBL	WBR	NBL	NBT	NBR	SBL	SBT	SBR	NEL	NER	NER2
Lane Configurations		M			4			4	7	M		
Volume (vph)	5	5	20	5	300	5	10	195	280	630	5	5
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Lane Width	12	12	12	12	14	12	12	12	12	12	12	12
Total Lost time (s)		4.0			4.0			4.0	5.0	5.0		
Lane Util. Factor		1.00			1.00			1.00	1.00	1.00		
Frpb, ped/bikes		0.98			1.00			1.00	0.98	1.00		
Flpb, ped/bikes		1.00			1.00			1.00	1.00	0.99		
Frt		0.91			1.00			1.00	0.85	1.00		
Flt Protected		0.98			1.00			1.00	1.00	0.95		
Satd. Flow (prot)		1529			1790			1666	1396	1623		
Flt Permitted		0.98			0.99			0.98	1.00	1.00		
Satd. Flow (perm)		1529			1782			1631	1396	1703		
Peak-hour factor, PHF	0.84	0.84	0.84	0.84	0.84	0.84	0.84	0.84	0.84	0.84	0.84	0.84
Adj. Flow (vph)	6	6	24	6	357	6	12	232	333	750	6	6
RTOR Reduction (vph)	0	20	0	0	1	0	0	0	0	6	0	0
Lane Group Flow (vph)	0	16	0	0	368	0	0	244	333	756	0	0
Confl. Peds. (#/hr)	5	10	5	10	000	10	10	211	10	5		5
Confl. Bikes (#/hr)	O .		10	10		10	10		10	J		10
Heavy Vehicles (%)	0%	0%	0%	0%	4%	0%	0%	5%	4%	2%	0%	0%
Turn Type	Perm	NA	070	Perm	NA	070	Perm	NA	custom	NA	070	070
Protected Phases	I CIIII	8		I CIIII	2		I CIIII	6	cusioni	INA		
Permitted Phases	8	U		2	Z		6	U	3 6	3		
Actuated Green, G (s)	U	26.0		2	17.4		U	17.4	56.9	30.5		
Effective Green, g (s)		26.0			17.4			17.4	52.9	30.5		
Actuated g/C Ratio		0.46			0.31			0.31	0.93	0.54		
Clearance Time (s)		4.0			4.0			4.0	0.75	5.0		
Vehicle Extension (s)		2.5			2.5			2.5		2.0		
Lane Grp Cap (vph)		698			544			498	1297	912		
v/s Ratio Prot		090			344			490	1297	912		
v/s Ratio Prot v/s Ratio Perm		0.01			c0.21			0.15	0.24	c0.44		
v/c Ratio		0.01			0.68			0.15	0.24	0.83		
Uniform Delay, d1		8.5			17.3				0.26	11.0		
J ·								16.1				
Progression Factor		1.00			1.00			1.00	1.00	1.00		
Incremental Delay, d2		0.0			3.0			0.6	0.0	6.0		
Delay (s)		8.5			20.3			16.7	0.2	17.0		
Level of Service		A			C			B	Α	B		
Approach LOS		8.5			20.3			7.2		17.0		
Approach LOS		Α			С			А		В		
Intersection Summary												
HCM 2000 Control Delay			14.3	Н	CM 2000	Level of S	Service		В			
HCM 2000 Volume to Capac	ity ratio		0.81									
Actuated Cycle Length (s)			56.9		um of lost				11.0			
Intersection Capacity Utilizati	on		74.6%	IC	CU Level of	of Service	!		D			
Analysis Period (min)			15									
c Critical Lane Group												

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4	7		4	7		ፋቤ	
Volume (vph)	15	55	30	160	55	30	5	485	180	35	530	10
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Lane Width	12	14	12	12	12	12	12	12	12	12	12	12
Total Lost time (s)		4.5			4.5	4.5		4.5	4.5		4.5	
Lane Util. Factor		1.00			1.00	1.00		1.00	1.00		0.95	
Frpb, ped/bikes		0.98			1.00	0.92		1.00	0.93		1.00	
Flpb, ped/bikes		1.00			0.98	1.00		1.00	1.00		1.00	
Frt		0.96			1.00	0.85		1.00	0.85		1.00	
Flt Protected		0.99			0.96	1.00		1.00	1.00		1.00	
Satd. Flow (prot)		1727			1645	1348		1731	1343		3267	
Flt Permitted		0.94			0.74	1.00		0.99	1.00		0.90	
Satd. Flow (perm)		1637			1258	1348		1722	1343		2953	
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	17	61	33	178	61	33	6	539	200	39	589	11
RTOR Reduction (vph)	0	23	0	0	0	23	0	0	83	0	1	0
Lane Group Flow (vph)	0	88	0	0	239	10	0	545	117	0	638	0
Confl. Peds. (#/hr)	20		20	20		20	20		20	20		20
Confl. Bikes (#/hr)			20			20			20			20
Heavy Vehicles (%)	1%	1%	1%	1%	1%	1%	1%	1%	1%	1%	1%	1%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	4	0	0	0
Turn Type	Perm	NA		Perm	NA	Perm	Perm	NA	Perm	Perm	NA	
Protected Phases		2			6			8			4	
Permitted Phases	2			6		6	8		8	4		
Actuated Green, G (s)		22.1			22.1	22.1		43.9	43.9		43.9	
Effective Green, g (s)		22.1			22.1	22.1		43.9	43.9		43.9	
Actuated g/C Ratio		0.29			0.29	0.29		0.59	0.59		0.59	
Clearance Time (s)		4.5			4.5	4.5		4.5	4.5		4.5	
Vehicle Extension (s)		3.0			3.0	3.0		2.0	2.0		2.0	
Lane Grp Cap (vph)		482			370	397		1007	786		1728	
v/s Ratio Prot		0.05			0.10	0.04		0.00	0.00		0.00	
v/s Ratio Perm		0.05			c0.19	0.01		c0.32	0.09		0.22	
v/c Ratio		0.18			0.65	0.02		0.54	0.15		0.37	
Uniform Delay, d1		19.7			23.0	18.8		9.4	7.1		8.2	
Progression Factor		1.00			1.00	1.00		0.36	0.02		1.00	
Incremental Delay, d2		0.2			3.8	0.0		2.0	0.4		0.6	
Delay (s)		19.9			26.9	18.8		5.4	0.5		8.8	
Level of Service		B			C	В		A	А		A	
Approach LOS		19.9			25.9			4.1			8.8	
Approach LOS		В			С			Α			A	
Intersection Summary												
HCM 2000 Control Delay			10.1	H	CM 2000	Level of	Service		В			
HCM 2000 Volume to Capaci	ity ratio		0.58									
Actuated Cycle Length (s)			75.0		um of lost				9.0			
Intersection Capacity Utilizati	on		71.9%	IC	U Level	of Service	<u> </u>		С			
Analysis Period (min)			15									
c Critical Lane Group												

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			€Î}•			4Th	
Volume (vph)	25	5	10	15	5	15	10	675	10	15	645	25
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Lane Width	12	14	12	12	14	12	12	12	12	12	12	12
Total Lost time (s)		4.5			4.5			4.5			4.5	
Lane Util. Factor		1.00			1.00			0.95			0.95	
Frpb, ped/bikes		0.99			0.98			1.00			1.00	
Flpb, ped/bikes		0.99			0.99			1.00			1.00	
Frt		0.97			0.94			1.00			0.99	
Flt Protected		0.97			0.98			1.00			1.00	
Satd. Flow (prot)		1687			1653			3253			3235	
Flt Permitted		0.85			0.90			0.94			0.93	
Satd. Flow (perm)		1477			1525			3073			3025	
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	28	6	11	17	6	17	11	750	11	17	717	28
RTOR Reduction (vph)	0	8	0	0	13	0	0	1	0	0	3	0
Lane Group Flow (vph)	0	37	0	0	27	0	0	771	0	0	759	0
Confl. Peds. (#/hr)	20		20	20		20	20		20	20		20
Confl. Bikes (#/hr)			15			15			15			15
Heavy Vehicles (%)	1%	1%	1%	1%	1%	1%	1%	1%	1%	1%	1%	1%
Bus Blockages (#/hr)	0	0	4	0	0	4	0	4	0	0	4	0
Turn Type	Perm	NA		Perm	NA		Perm	NA		Perm	NA	
Protected Phases		2			6			4			8	
Permitted Phases	2			6			4			8		
Actuated Green, G (s)		18.0			18.0			48.0			48.0	
Effective Green, g (s)		18.0			18.0			48.0			48.0	
Actuated g/C Ratio		0.24			0.24			0.64			0.64	
Clearance Time (s)		4.5			4.5			4.5			4.5	
Vehicle Extension (s)		3.0			3.0			2.0			2.0	
Lane Grp Cap (vph)		354			366			1966			1936	
v/s Ratio Prot												
v/s Ratio Perm		c0.02			0.02			c0.25			0.25	
v/c Ratio		0.10			0.07			0.39			0.39	
Uniform Delay, d1		22.2			22.1			6.5			6.5	
Progression Factor		1.00			1.00			1.55			1.13	
Incremental Delay, d2		0.1			0.1			0.6			0.6	
Delay (s)		22.3			22.1			10.6			7.9	
Level of Service		С			С			В			Α	
Approach Delay (s)		22.3			22.1			10.6			7.9	
Approach LOS		С			С			В			Α	
Intersection Summary												
HCM 2000 Control Delay			10.0	Н	CM 2000	Level of	Service		А			
HCM 2000 Volume to Capacity	y ratio		0.31									
Actuated Cycle Length (s)			75.0	Sı	um of lost	time (s)			9.0			
Intersection Capacity Utilizatio	n		54.5%		:U Level o		:		А			
Analysis Period (min)			15									
c Critical Lane Group												

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	۲	₽		۲	₽			€Î}•			4T)	
Volume (vph)	20	30	25	70	45	30	25	660	30	15	610	30
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Total Lost time (s)	4.5	4.5		4.5	4.5			4.5			4.5	
Lane Util. Factor	1.00	1.00		1.00	1.00			0.95			0.95	
Frpb, ped/bikes	1.00	0.98		1.00	0.98			1.00			1.00	
Flpb, ped/bikes	0.98	1.00		0.98	1.00			1.00			1.00	
Frt	1.00	0.93		1.00	0.94			0.99			0.99	
Flt Protected	0.95	1.00		0.95	1.00			1.00			1.00	
Satd. Flow (prot)	1594	1559		1608	1595			2973			2972	
Flt Permitted	0.70	1.00		0.72	1.00			0.92			0.93	
Satd. Flow (perm)	1179	1559		1214	1595			2730			2775	
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	22	33	28	78	50	33	28	733	33	17	678	33
RTOR Reduction (vph)	0	23	0	0	27	0	0	2	0	0	3	0
Lane Group Flow (vph)	22	38	0	78	56	0	0	792	0	0	725	0
Confl. Peds. (#/hr)	20		20	20		20	20		20	20		20
Confl. Bikes (#/hr)			15			15			15			15
Heavy Vehicles (%)	2%	2%	2%	1%	1%	1%	1%	3%	1%	1%	3%	1%
Bus Blockages (#/hr)	0	0	4	0	0	4	0	4	0	0	4	0
Parking (#/hr)							5	5	5	5	5	5
Turn Type	Perm	NA		Perm	NA		Perm	NA		Perm	NA	
Protected Phases		2			6			4			8	
Permitted Phases	2			6			4			8		
Actuated Green, G (s)	14.4	14.4		14.4	14.4			51.6			51.6	
Effective Green, g (s)	14.4	14.4		14.4	14.4			51.6			51.6	
Actuated g/C Ratio	0.19	0.19		0.19	0.19			0.69			0.69	
Clearance Time (s)	4.5	4.5		4.5	4.5			4.5			4.5	
Vehicle Extension (s)	3.0	3.0		3.0	3.0			2.0			2.0	
Lane Grp Cap (vph)	226	299		233	306			1878			1909	
v/s Ratio Prot		0.02			0.04							
v/s Ratio Perm	0.02			c0.06				c0.29			0.26	
v/c Ratio	0.10	0.13		0.33	0.18			0.42			0.38	
Uniform Delay, d1	24.9	25.1		26.2	25.4			5.1			4.9	
Progression Factor	1.00	1.00		1.00	1.00			1.00			0.98	
Incremental Delay, d2	0.2	0.2		0.9	0.3			0.7			0.5	
Delay (s)	25.1	25.3		27.0	25.7			5.8			5.4	
Level of Service	С	С		С	С			Α			Α	
Approach Delay (s)		25.3			26.3			5.8			5.4	
Approach LOS		С			С			Α			А	
Intersection Summary												
HCM 2000 Control Delay			8.4	Н	CM 2000	Level of S	Service		А			
HCM 2000 Volume to Capac	city ratio		0.40									
Actuated Cycle Length (s)	,		75.0	Sı	um of lost	time (s)			9.0			
Intersection Capacity Utilizat	tion		63.3%		U Level o		:		В			
Analysis Period (min)			15									
c Critical Lane Group												

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Movement	EBL	EBR	NBL	NBT	SBT	SBR		
Lane Configurations	*	7		414	† 1>			
Volume (vph)	95	90	25	605	510	105		
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750		
Total Lost time (s)	4.0	4.0		4.0	4.0			
Lane Util. Factor	1.00	1.00		0.95	0.95			
Frpb, ped/bikes	1.00	0.93		1.00	0.98			
Flpb, ped/bikes	1.00	1.00		1.00	1.00			
Frt	1.00	0.85		1.00	0.97			
Flt Protected	0.95	1.00		1.00	1.00			
Satd. Flow (prot)	1630	1360		3256	3127			
Flt Permitted	0.95	1.00		0.91	1.00			
Satd. Flow (perm)	1630	1360		2956	3127			
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90		
Adj. Flow (vph)	106	100	28	672	567	117		
RTOR Reduction (vph)	0	73	0	0	14	0		
Lane Group Flow (vph)	106	27	0	700	670	0		
Confl. Peds. (#/hr)	20	20	20	700	070	20		
Confl. Bikes (#/hr)	20	15	20			15		
Heavy Vehicles (%)	2%	2%	1%	1%	1%	1%		
Bus Blockages (#/hr)	0	0	0	4	4	0		
Turn Type	NA	Perm	Perm	NA	NA	0		
Protected Phases	2	I CIIII	I CIIII	8	4			
Permitted Phases	Z	2	8	U	7			
Actuated Green, G (s)	32.0	32.0	U	80.0	80.0			
Effective Green, g (s)	32.0	32.0		80.0	80.0			
Actuated g/C Ratio	0.27	0.27		0.67	0.67			
Clearance Time (s)	4.0	4.0		4.0	4.0			
Vehicle Extension (s)	3.0	3.0		3.0	3.0			
Lane Grp Cap (vph)	434	362		1970	2084			
v/s Ratio Prot	c0.07	302		1770	0.21			
v/s Ratio Perm	60.07	0.02		c0.24	0.21			
v/c Ratio	0.24	0.02		0.36	0.32			
Uniform Delay, d1	34.5	32.9		8.7	8.5			
Progression Factor	1.00	1.00		0.44	1.00			
Incremental Delay, d2	1.3	0.4		0.5	0.4			
Delay (s)	35.8	33.3		4.3	8.9			
Level of Service	D	C		Α.5	Α			
Approach Delay (s)	34.6			4.3	8.9			
Approach LOS	C			A	A			
• • •								
Intersection Summary								
HCM 2000 Control Delay			10.2	H	CM 2000	Level of Service	В	
HCM 2000 Volume to Capa	icity ratio		0.32					
Actuated Cycle Length (s)			120.0		um of lost		8.0	
Intersection Capacity Utiliza	ation		58.0%	IC	U Level c	of Service	В	
Analysis Period (min)			15					
c Critical Lane Group								

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	↑	7	¥	†	7	ħ.	↑ 1>		ř	↑ Ъ	
Volume (vph)	105	245	215	75	265	65	250	385	45	115	355	70
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Lane Width	12	10	10	10	10	11	11	10	11	12	12	12
Total Lost time (s)	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5		4.5	4.5	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95		1.00	0.95	
Frpb, ped/bikes	1.00	1.00	0.97	1.00	1.00	0.94	1.00	0.99		1.00	0.98	
Flpb, ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00		1.00	1.00	
Frt	1.00	1.00	0.85	1.00	1.00	0.85	1.00	0.98		1.00	0.98	
Flt Protected	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00		0.95	1.00	
Satd. Flow (prot)	1646	1601	1334	1492	1617	1318	1560	2937		1607	3159	
Flt Permitted	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00		0.95	1.00	
Satd. Flow (perm)	1646	1601	1334	1492	1617	1318	1560	2937		1607	3159	
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	117	272	239	83	294	72	278	428	50	128	394	78
RTOR Reduction (vph)	0	0	97	0	0	56	0	6	0	0	12	0
Lane Group Flow (vph)	117	272	142	83	294	16	278	472	0	128	460	0
Confl. Peds. (#/hr)	20		20	20		20	20		20	20		20
Confl. Bikes (#/hr)			20			20			20			20
Heavy Vehicles (%)	1%	2%	1%	4%	1%	2%	3%	3%	3%	1%	1%	1%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	10	6	0	0
Turn Type	Prot	NA	pm+ov	Prot	NA	Perm	Prot	NA		Prot	NA	
Protected Phases	5	2	3	1	6		3	8		7	4	
Permitted Phases			2			6						
Actuated Green, G (s)	12.0	29.3	54.2	9.0	26.3	26.3	24.9	50.2		13.5	38.8	
Effective Green, g (s)	12.0	29.3	54.2	9.0	26.3	26.3	24.9	50.2		13.5	38.8	
Actuated g/C Ratio	0.10	0.24	0.45	0.08	0.22	0.22	0.21	0.42		0.11	0.32	
Clearance Time (s)	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5		4.5	4.5	
Vehicle Extension (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0		2.0	2.0	
Lane Grp Cap (vph)	164	390	652	111	354	288	323	1228		180	1021	
v/s Ratio Prot	c0.07	c0.17	0.05	0.06	c0.18	0.04	c0.18	0.16		0.08	c0.15	
v/s Ratio Perm	0.74	0.70	0.06	0.75	0.00	0.01	0.07	0.00		0.74	0.45	
v/c Ratio	0.71	0.70	0.22	0.75	0.83	0.05	0.86	0.38		0.71	0.45	
Uniform Delay, d1	52.3	41.3	20.0	54.4	44.7	37.0	45.9	24.2		51.4	32.2	
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00		1.17	0.84	
Incremental Delay, d2	11.5	4.4	0.1	21.1	14.5	0.0	19.6	0.9		10.1	1.4	
Delay (s)	63.9	45.7	20.1	75.5	59.2	37.1	65.5	25.1		70.3	28.4	
Level of Service	E	D	С	E	E	D	E	C		E	C	
Approach Delay (s)		39.3			58.7			40.0			37.3	
Approach LOS		D			E			D			D	
Intersection Summary												
HCM 2000 Control Delay			42.6	Н	CM 2000	Level of	Service		D			
HCM 2000 Volume to Capac	ity ratio		0.68									
Actuated Cycle Length (s)			120.0	S	um of lost	t time (s)			18.0			
Intersection Capacity Utilizat	ion		70.1%		CU Level		:		С			
Analysis Period (min)			15									
c Critical Lane Group												

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Movement	WBL2	WBL	WBR	NBL	NBT	NBR	SBL	SBT	SBR	NEL	NER	NER2
Lane Configurations		M			4			4	7	M		
Volume (vph)	5	10	20	5	245	5	30	335	620	370	5	5
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Lane Width	12	12	12	12	14	12	12	12	12	12	12	12
Total Lost time (s)		4.0			4.0			4.0	5.0	5.0		
Lane Util. Factor		1.00			1.00			1.00	1.00	1.00		
Frpb, ped/bikes		0.97			1.00			1.00	0.98	1.00		
Flpb, ped/bikes		0.99			1.00			1.00	1.00	1.00		
Frt		0.92			1.00			1.00	0.85	1.00		
Flt Protected		0.98			1.00			1.00	1.00	0.95		
Satd. Flow (prot)		1507			1822			1724	1441	1636		
Flt Permitted		0.98			0.99			0.96	1.00	1.00		
Satd. Flow (perm)		1507			1808			1663	1441	1716		
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	6	11	22	6	272	6	33	372	689	411	6	6
RTOR Reduction (vph)	0	29	0	0	1	0	0	0	0	11	0	0
Lane Group Flow (vph)	0	10	0	0	283	0	0	405	689	412	0	0
Confl. Peds. (#/hr)	10	10	10	10	200	10	10	100	10	10		10
Confl. Bikes (#/hr)	10		10	10		10	10		10	10		10
Heavy Vehicles (%)	1%	1%	1%	1%	2%	1%	1%	1%	1%	1%	2%	1%
Turn Type	Perm	NA	170	Perm	NA	170	Perm	NA	custom	NA	270	170
Protected Phases	I CIIII	8		I CIIII	2		I CIIII	6	Custom	INA		
Permitted Phases	8	U		2	Z		6	U	3 6	3		
Actuated Green, G (s)	U	10.8		2	17.3		U	17.3	41.7	15.4		
Effective Green, g (s)		10.8			17.3			17.3	37.7	15.4		
Actuated g/C Ratio		0.26			0.41			0.41	0.90	0.37		
Clearance Time (s)		4.0			4.0			4.0	0.70	5.0		
Vehicle Extension (s)		2.5			2.5			2.5		2.0		
Lane Grp Cap (vph)		390			750			689	1302	633		
v/s Ratio Prot		390			750			009	1302	033		
v/s Ratio Prot v/s Ratio Perm		0.01			0.16			0.24	c0.48	c0.24		
v/c Ratio		0.01			0.16			0.24	0.53			
Uniform Delay, d1		11.5			8.5			9.4		0.65 10.9		
J ·									0.4			
Progression Factor		1.00			1.00			1.00	1.00	1.00		
Incremental Delay, d2		0.0			0.2 8.7			1.1	0.2	1.8		
Delay (s)		11.5						10.5	0.5	12.8		
Level of Service		11 F			Α			B	Α	B		
Approach LOS		11.5			8.7			4.2		12.8		
Approach LOS		В			А			А		В		
Intersection Summary												
HCM 2000 Control Delay			7.0	Н	CM 2000	Level of S	Service		Α			
HCM 2000 Volume to Capac	city ratio		0.67									
Actuated Cycle Length (s)			41.7		um of lost				11.0			
Intersection Capacity Utilizat	ion		73.8%	IC	CU Level of	of Service	!		D			
Analysis Period (min)			15									
c Critical Lane Group												

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4	7	ሻ	4î		ሻ	4î	
Volume (vph)	15	55	30	160	55	30	5	485	180	35	530	10
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Lane Width	12	14	12	12	12	12	12	12	12	12	12	12
Total Lost time (s)		4.5			4.5	4.5	4.5	4.5		4.5	4.5	
Lane Util. Factor		1.00			1.00	1.00	1.00	1.00		1.00	1.00	
Frpb, ped/bikes		0.97			1.00	0.91	1.00	0.98		1.00	1.00	
Flpb, ped/bikes		1.00			0.98	1.00	0.99	1.00		0.99	1.00	
Frt		0.96			1.00	0.85	1.00	0.96		1.00	1.00	
Flt Protected		0.99			0.96	1.00	0.95	1.00		0.95	1.00	
Satd. Flow (prot)		1708			1629	1344	1625	1630		1632	1726	
Flt Permitted		0.94			0.73	1.00	0.34	1.00		0.25	1.00	
Satd. Flow (perm)		1615			1241	1344	586	1630		435	1726	
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	17	61	33	178	61	33	6	539	200	39	589	11
RTOR Reduction (vph)	0	21	0	0	0	24	0	17	0	0	1	0
Lane Group Flow (vph)	0	90	0	0	239	9	6	722	0	39	599	0
Confl. Peds. (#/hr)	20		20	20		20	20		20	20		20
Confl. Bikes (#/hr)			20			20			20			20
Heavy Vehicles (%)	1%	1%	1%	1%	1%	1%	1%	1%	1%	1%	1%	1%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	4	0	0	0
Turn Type	Perm	NA		Perm	NA	Perm	Perm	NA		Perm	NA	
Protected Phases		2			6			8			4	
Permitted Phases	2			6		6	8			4		
Actuated Green, G (s)		20.0			20.0	20.0	46.0	46.0		46.0	46.0	
Effective Green, g (s)		20.0			20.0	20.0	46.0	46.0		46.0	46.0	
Actuated g/C Ratio		0.27			0.27	0.27	0.61	0.61		0.61	0.61	
Clearance Time (s)		4.5			4.5	4.5	4.5	4.5		4.5	4.5	
Vehicle Extension (s)		3.0			3.0	3.0	2.0	2.0		2.0	2.0	
Lane Grp Cap (vph)		430			330	358	359	999		266	1058	
v/s Ratio Prot								c0.44			0.35	
v/s Ratio Perm		0.06			c0.19	0.01	0.01			0.09		
v/c Ratio		0.21			0.72	0.02	0.02	0.72		0.15	0.57	
Uniform Delay, d1		21.4			25.0	20.3	5.7	10.1		6.2	8.6	
Progression Factor		1.00			1.00	1.00	1.72	1.41		1.00	1.00	
Incremental Delay, d2		0.2			7.7	0.0	0.1	3.4		1.2	2.2	
Delay (s)		21.6			32.7	20.3	9.8	17.6		7.3	10.8	
Level of Service		С			С	С	Α	В		Α	В	
Approach Delay (s)		21.6			31.2			17.5			10.6	
Approach LOS		С			С			В			В	
Intersection Summary												
HCM 2000 Control Delay			17.4	H	CM 2000	Level of	Service		В			
HCM 2000 Volume to Capaci	ity ratio		0.72									
Actuated Cycle Length (s)			75.0	Sı	um of los	t time (s)			9.0			
Intersection Capacity Utilizati	on		81.4%			of Service	<u> </u>		D			
Analysis Period (min)			15									
c Critical Lane Group												

Lane Configurations		•	-	•	•	←	•	1	†	~	-	ţ	4
Volume (vph) 25 5 10 15 5 15 10 675 10 15 645 Ideal Flow (vphpl) 1750 17	Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Ideal Flow (vphpl)	ane Configurations		4			4		Ŋ	f÷		ሻ	f)	
Lane Width	/olume (vph)	25	5	10	15	5	15	10	675	10	15	645	25
Total Lost time (s) 4.5	deal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Lane Util. Factor 1.00 0.99 1.00 0.99 1.00 0.99 1.00 0.99 1.00 0.99 1.00 0.99 1.00 0.99 1.00 0.99 1.00 0.99 1.00 0.95 1.00 0.95 1.00 0.95 1.00 0.95 1.00 0.95 1.00 0.95 1.00 0.95 1.00 0.95 1.00 0.90 0.90 0.95 1.00 0.95 1.00 0.90 0.90 0.90 0.90 0.90 0.90 0.90 0.90 0.90 0.90 0.90 0.90 0.90 0.90 0.90 0.90 0.90 <td>ane Width</td> <td>12</td> <td>14</td> <td>12</td> <td>12</td> <td>14</td> <td>12</td> <td>12</td> <td>12</td> <td>12</td> <td>12</td> <td>12</td> <td>12</td>	ane Width	12	14	12	12	14	12	12	12	12	12	12	12
Frpb, ped/bikes 0.98 0.96 1.00 1.00 1.00 1.00 Flpb, ped/bikes 0.98 0.98 0.99 1.00 0.99 1.00 Frt 0.97 0.94 1.00 1.00 0.99 1.00 Fit Protected 0.97 0.98 0.95 1.00 0.95 1.00 Satd. Flow (prot) 1653 1613 1632 1699 1632 1691 Fit Permitted 0.85 0.90 0.27 1.00 0.26 1.00 Satd. Flow (perm) 1448 1488 458 1699 441 1691 Peak-hour factor, PHF 0.90 </td <td>otal Lost time (s)</td> <td></td> <td>4.5</td> <td></td> <td></td> <td>4.5</td> <td></td> <td>4.5</td> <td>4.5</td> <td></td> <td>4.5</td> <td>4.5</td> <td></td>	otal Lost time (s)		4.5			4.5		4.5	4.5		4.5	4.5	
Flpb, ped/bikes 0.98 0.98 0.99 1.00 0.99 1.00 Frt 0.97 0.94 1.00 1.00 1.00 0.99 Flt Protected 0.97 0.98 0.95 1.00 0.95 1.00 Satd. Flow (prot) 1653 1613 1632 1699 1632 1691 Flt Permitted 0.85 0.90 0.27 1.00 0.26 1.00 Satd. Flow (perm) 1448 1488 458 1699 441 1691 Peak-hour factor, PHF 0.90			1.00			1.00		1.00	1.00		1.00	1.00	
Frit 0.97 0.94 1.00 1.00 1.00 0.99 Fit Protected 0.97 0.98 0.95 1.00 0.95 1.00 Satd. Flow (prot) 1653 1613 1632 1699 1632 1691 Fit Permitted 0.85 0.90 0.27 1.00 0.26 1.00 Satd. Flow (perm) 1448 1488 458 1699 441 1691 Peak-hour factor, PHF 0.90 <td< td=""><td>rpb, ped/bikes</td><td></td><td>0.98</td><td></td><td></td><td>0.96</td><td></td><td>1.00</td><td>1.00</td><td></td><td>1.00</td><td>1.00</td><td></td></td<>	rpb, ped/bikes		0.98			0.96		1.00	1.00		1.00	1.00	
Fit Protected 0.97 0.98 0.95 1.00 0.95 1.00 Satd. Flow (prot) 1653 1613 1632 1699 1632 1691 Fit Permitted 0.85 0.90 0.27 1.00 0.26 1.00 Satd. Flow (perm) 1448 1488 458 1699 441 1691 Peak-hour factor, PHF 0.90	Flpb, ped/bikes		0.98			0.98		0.99	1.00		0.99	1.00	
Satd. Flow (prot) 1653 1613 1632 1699 1632 1691 Fit Permitted 0.85 0.90 0.27 1.00 0.26 1.00 Satd. Flow (perm) 1448 1488 458 1699 441 1691 Peak-hour factor, PHF 0.90 0.	-rt		0.97			0.94		1.00	1.00		1.00	0.99	
Fit Permitted 0.85 0.90 0.27 1.00 0.26 1.00 Satd. Flow (perm) 1448 1488 458 1699 441 1691 Peak-hour factor, PHF 0.90	It Protected		0.97			0.98		0.95	1.00		0.95	1.00	
Satd. Flow (perm) 1448 1488 458 1699 441 1691 Peak-hour factor, PHF 0.90 11 0 0 0 2 20 20 20 20 20 20 20 20 20 </td <td>Satd. Flow (prot)</td> <td></td> <td>1653</td> <td></td> <td></td> <td>1613</td> <td></td> <td>1632</td> <td>1699</td> <td></td> <td>1632</td> <td>1691</td> <td></td>	Satd. Flow (prot)		1653			1613		1632	1699		1632	1691	
Peak-hour factor, PHF 0.90 2 2 1.80 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	It Permitted		0.85			0.90		0.27	1.00		0.26	1.00	
Adj. Flow (vph) 28 6 11 17 6 17 11 750 11 17 717 RTOR Reduction (vph) 0 8 0 0 13 0 0 1 0 0 2 Lane Group Flow (vph) 0 37 0 0 27 0 11 760 0 17 743 Confl. Peds. (#/hr) 20	Satd. Flow (perm)		1448			1488		458	1699		441	1691	
Adj. Flow (vph) 28 6 11 17 6 17 11 750 11 17 717 RTOR Reduction (vph) 0 8 0 0 13 0 0 1 0 0 2 Lane Group Flow (vph) 0 37 0 0 27 0 11 760 0 17 743 Confl. Peds. (#/hr) 20	Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
RTOR Reduction (vph) 0 8 0 0 13 0 0 1 0 0 2 Lane Group Flow (vph) 0 37 0 0 27 0 11 760 0 17 743 Confl. Peds. (#/hr) 20 </td <td></td> <td>28</td> <td>6</td> <td>11</td> <td>17</td> <td>6</td> <td>17</td> <td>11</td> <td>750</td> <td>11</td> <td>17</td> <td>717</td> <td>28</td>		28	6	11	17	6	17	11	750	11	17	717	28
Lane Group Flow (vph) 0 37 0 0 27 0 11 760 0 17 743 Confl. Peds. (#/hr) 20			8	0	0	13	0	0	1	0	0	2	0
Confl. Peds. (#/hr) 20 <td></td> <td></td> <td></td> <td>0</td> <td></td> <td>27</td> <td></td> <td></td> <td>760</td> <td>0</td> <td>17</td> <td>743</td> <td>0</td>				0		27			760	0	17	743	0
Confl. Bikes (#/hr) 20 20 Heavy Vehicles (%) 1%				20									20
Heavy Vehicles (%) 1% 0 4 0 0 4 0 0 4 Perm NA Perm													20
Bus Blockages (#/hr) 0 0 4 0 0 4 0 0 4 Turn Type Perm NA Perm NA Perm NA Perm NA Protected Phases 2 6 4 8 Permitted Phases 2 6 4 8 Actuated Green, G (s) 18.0 18.0 48.0 48.0 48.0 Effective Green, g (s) 18.0 18.0 48.0 48.0 48.0 Actuated g/C Ratio 0.24 0.24 0.64 0.64 0.64 Clearance Time (s) 4.5 4.5 4.5 4.5 4.5 Vehicle Extension (s) 3.0 3.0 2.0 2.0 2.0 Lane Grp Cap (vph) 347 357 293 1087 282 1082		1%	1%		1%	1%		1%	1%		1%	1%	1%
Turn Type Perm NA Perm NA Perm NA Perm NA Protected Phases 2 6 4 8 Permitted Phases 2 6 4 8 Actuated Green, G (s) 18.0 18.0 48.0 48.0 48.0 Effective Green, g (s) 18.0 18.0 48.0 48.0 48.0 48.0 Actuated g/C Ratio 0.24 0.24 0.64 0.64 0.64 0.64 Clearance Time (s) 4.5 4.5 4.5 4.5 4.5 4.5 Vehicle Extension (s) 3.0 3.0 2.0 2.0 2.0 2.0 Lane Grp Cap (vph) 347 357 293 1087 282 1082	• • •												0
Protected Phases 2 6 4 8 Permitted Phases 2 6 4 8 Actuated Green, G (s) 18.0 18.0 48.0 48.0 48.0 Effective Green, g (s) 18.0 18.0 48.0 48.0 48.0 Actuated g/C Ratio 0.24 0.24 0.64 0.64 0.64 Clearance Time (s) 4.5 4.5 4.5 4.5 4.5 Vehicle Extension (s) 3.0 3.0 2.0 2.0 2.0 Lane Grp Cap (vph) 347 357 293 1087 282 1082												NA	
Permitted Phases 2 6 4 8 Actuated Green, G (s) 18.0 18.0 48.0 48.0 48.0 48.0 Effective Green, g (s) 18.0 18.0 48.0 48.0 48.0 48.0 Actuated g/C Ratio 0.24 0.24 0.64 0.64 0.64 0.64 Clearance Time (s) 4.5 4.5 4.5 4.5 4.5 4.5 Vehicle Extension (s) 3.0 3.0 2.0 2.0 2.0 2.0 Lane Grp Cap (vph) 347 357 293 1087 282 1082													
Actuated Green, G (s) 18.0 18.0 48.0 48.0 48.0 48.0 Effective Green, g (s) 18.0 18.0 48.0 48.0 48.0 48.0 Actuated g/C Ratio 0.24 0.24 0.64 0.64 0.64 0.64 Clearance Time (s) 4.5 4.5 4.5 4.5 4.5 4.5 Vehicle Extension (s) 3.0 3.0 2.0 2.0 2.0 2.0 Lane Grp Cap (vph) 347 357 293 1087 282 1082		2			6			4			8		
Effective Green, g (s) 18.0 18.0 48.0 48.0 48.0 48.0 Actuated g/C Ratio 0.24 0.24 0.64 0.64 0.64 0.64 Clearance Time (s) 4.5 4.5 4.5 4.5 4.5 4.5 Vehicle Extension (s) 3.0 3.0 2.0 2.0 2.0 2.0 Lane Grp Cap (vph) 347 357 293 1087 282 1082			18.0			18.0			48.0			48.0	
Actuated g/C Ratio 0.24 0.24 0.64 0.64 0.64 Clearance Time (s) 4.5 4.5 4.5 4.5 4.5 Vehicle Extension (s) 3.0 3.0 2.0 2.0 2.0 2.0 Lane Grp Cap (vph) 347 357 293 1087 282 1082	` ,					18.0			48.0				
Clearance Time (s) 4.5 4.5 4.5 4.5 4.5 Vehicle Extension (s) 3.0 3.0 2.0 2.0 2.0 2.0 Lane Grp Cap (vph) 347 357 293 1087 282 1082													
Vehicle Extension (s) 3.0 3.0 2.0 2.0 2.0 2.0 Lane Grp Cap (vph) 347 357 293 1087 282 1082			4.5					4.5	4.5		4.5	4.5	
Lane Grp Cap (vph) 347 357 293 1087 282 1082													
			347			357		293	1087		282	1082	
V/S KATIO PTOT CU.45 0.44	/s Ratio Prot								c0.45			0.44	
v/s Ratio Perm c0.03 0.02 0.02 0.04			c0.03			0.02		0.02			0.04		
v/c Ratio 0.11 0.08 0.04 0.70 0.06 0.69									0.70			0.69	
Uniform Delay, d1 22.2 22.1 5.0 8.8 5.1 8.7													
Progression Factor 1.00 1.00 1.41 1.81 0.71 0.66													
Incremental Delay, d2 0.1 0.1 0.2 2.7 0.3 3.0													
Delay (s) 22.4 22.2 7.2 18.6 3.9 8.7	3												
Level of Service C C A B A A	3												
Approach Delay (s) 22.4 22.2 18.4 8.6			22.4										
Approach LOS C C B A													
Intersection Summary	ntersection Summary												
HCM 2000 Control Delay 14.0 HCM 2000 Level of Service B				14.0	Н	CM 2000	Level of	Service		В			
HCM 2000 Volume to Capacity ratio 0.54	•	ity ratio				2111 2000	2010101	2311100					
Actuated Cycle Length (s) 75.0 Sum of lost time (s) 9.0					Sı	ım of lost	time (s)			9.0			
Intersection Capacity Utilization 61.8% ICU Level of Service B		ion											
Analysis Period (min) 15					10	2 20001	J. 001 VIOC	·					
c Critical Lane Group													

	٠	→	•	•	←	•	4	†	<i>></i>	/	ţ	- ✓
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ň	(Î		۲	₽		Ŋ	ĵ÷		ሻ	f)	
Volume (vph)	20	30	25	70	45	30	25	660	30	15	610	30
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Total Lost time (s)	4.5	4.5		4.5	4.5		4.5	4.5		4.5	4.5	
Lane Util. Factor	1.00	1.00		1.00	1.00		1.00	1.00		1.00	1.00	
Frpb, ped/bikes	1.00	0.96		1.00	0.96		1.00	1.00		1.00	1.00	
Flpb, ped/bikes	0.96	1.00		0.96	1.00		0.99	1.00		0.99	1.00	
Frt	1.00	0.93		1.00	0.94		1.00	0.99		1.00	0.99	
Flt Protected	0.95	1.00		0.95	1.00		0.95	1.00		0.95	1.00	
Satd. Flow (prot)	1569	1532		1583	1572		1425	1450		1427	1449	
Flt Permitted	0.70	1.00		0.72	1.00		0.31	1.00		0.28	1.00	
Satd. Flow (perm)	1161	1532		1195	1572		470	1450		425	1449	
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	22	33	28	78	50	33	28	733	33	17	678	33
RTOR Reduction (vph)	0	23	0	0	27	0	0	2	0	0	2	0
Lane Group Flow (vph)	22	38	0	78	56	0	28	764	0	17	709	0
Confl. Peds. (#/hr)	20		20	20		20	20		20	20		20
Confl. Bikes (#/hr)			15			15			15			15
Heavy Vehicles (%)	2%	2%	2%	1%	1%	1%	1%	3%	1%	1%	3%	1%
Bus Blockages (#/hr)	0	0	4	0	0	4	0	4	0	0	4	0
Parking (#/hr)							5	5	5	5	5	5
Turn Type	Perm	NA		Perm	NA		Perm	NA		Perm	NA	
Protected Phases		2			6			4			8	
Permitted Phases	2			6			4			8		
Actuated Green, G (s)	14.2	14.2		14.2	14.2		51.8	51.8		51.8	51.8	
Effective Green, g (s)	14.2	14.2		14.2	14.2		51.8	51.8		51.8	51.8	
Actuated g/C Ratio	0.19	0.19		0.19	0.19		0.69	0.69		0.69	0.69	
Clearance Time (s)	4.5	4.5		4.5	4.5		4.5	4.5		4.5	4.5	
Vehicle Extension (s)	3.0	3.0		3.0	3.0		2.0	2.0		2.0	2.0	
Lane Grp Cap (vph)	219	290		226	297		324	1001		293	1000	
v/s Ratio Prot		0.02			0.04			c0.53			0.49	
v/s Ratio Perm	0.02			c0.07			0.06			0.04		
v/c Ratio	0.10	0.13		0.35	0.19		0.09	0.76		0.06	0.71	
Uniform Delay, d1	25.1	25.3		26.4	25.6		3.8	7.6		3.7	7.0	
Progression Factor	1.00	1.00		1.00	1.00		1.00	1.00		0.16	0.52	
Incremental Delay, d2	0.2	0.2		0.9	0.3		0.5	5.5		0.3	3.2	
Delay (s)	25.3	25.5		27.3	25.9		4.3	13.1		0.9	6.8	
Level of Service	С	C		С	С		Α	В		А	A	
Approach Delay (s)		25.4			26.6			12.8			6.7	
Approach LOS		С			С			В			Α	
Intersection Summary												
HCM 2000 Control Delay			12.1	H	CM 2000	Level of S	Service		В			
HCM 2000 Volume to Capac	city ratio		0.67									
Actuated Cycle Length (s)	-		75.0	Sı	um of lost	time (s)			9.0			
Intersection Capacity Utilizat	tion		62.3%		U Level o	. ,			В			
Analysis Period (min)			15									
c Critical Lane Group												

Movement
Lane Configurations
Volume (vph) 95 90 25 605 510 105 Ideal Flow (vphpl) 1750 1750 1750 1750 1750 1750 Total Lost time (s) 4.0 4.0 4.0 4.0 0.0 1.00 1.00 0.95 Frpb, ped/bikes 1.00 0.93 1.00 1.00 0.95 1.00 0.95 Flpb, ped/bikes 1.00 1.00 1.00 1.00 0.98 1.00 1.00 0.98 Flpb, ped/bikes 1.00 1.00 1.00 1.00 0.97 1.00 0.97 1.00 0.97 1.00 0.97 1.00 0.97 1.00 0.97 1.00 0.97 1.00 1.00 1.00 0.97 1.00 1.
Ideal Flow (vphpl)
Total Lost time (s)
Lane Util. Factor
Frpb, ped/bikes 1.00 0.93 1.00 Satd. Flow (prot) 1630 1352 1646 1705 3127 Tell Permitted 0.95 1.00 1.00 1.00 1.00 Satd. Flow (perm) 1630 1352 1646 1705 3127 Tell Permitted 0.95 1.00 1.00 1.00 1.00 Satd. Flow (perm) 1630 1352 1646 1705 3127 1.00 1.00 1.00 Satd. Flow (perm) 1630 1352 1646 1705 3127 1.00
Fipb, ped/bikes 1.00 1.00 1.00 1.00 1.00 0.97 Fit 1.00 0.85 1.00 1.00 0.97 Fit Protected 0.95 1.00 0.95 1.00 1.00 Satd. Flow (prot) 1630 1352 1646 1705 3127 Fit Permitted 0.95 1.00 0.95 1.00 1.00 Satd. Flow (perm) 1630 1352 1646 1705 3127 Peak-hour factor, PHF 0.90 0.90 0.90 0.90 0.90 0.90 Adj. Flow (vph) 106 100 28 672 567 117 RTOR Reduction (vph) 0 82 0 0 13 0 Lane Group Flow (vph) 106 18 28 672 671 0 Confl. Peds. (#/hr) 20 20 20 20 Confl. Bikes (#/hr) 15 15 Heavy Vehicles (%) 2% 2% 1% 1% 1% 1% Bus Blockages (#/hr) 0 0 0 4 4 0 Turn Type NA Perm Prot NA NA Protected Phases 2 3 8 4 Permitted Phases 2 3 8 4 Permitted Phases 2 2 3 8 4 Permitted Phases 2 2 0 4.7 90.0 81.3 Effective Green, g (s) 22.0 22.0 4.7 90.0 81.3 Actuated Green, G (s) 22.0 22.0 4.7 90.0 81.3 Actuated Green, G (s) 3.0 3.0 3.0 3.0 3.0 Lane Grp Cap (vph) 298 247 64 1278 2118 V/s Ratio Prot Co.07 0.02 c0.39 0.21 V/s Ratio Perm 0.01 V/c Ratio 0.36 0.07 0.44 0.53 0.32
Frt 1.00 0.85 1.00 1.00 0.97 Flt Protected 0.95 1.00 0.95 1.00 1.00 Satd. Flow (prot) 1630 1352 1646 1705 3127 Flt Permitted 0.95 1.00 0.95 1.00 1.00 Satd. Flow (perm) 1630 1352 1646 1705 3127 Peak-hour factor, PHF 0.90 0.90 0.90 0.90 0.90 0.90 Adj. Flow (vph) 106 100 28 672 567 117 RTOR Reduction (vph) 0 82 0 0 13 0 Lane Group Flow (vph) 106 18 28 672 671 0 Confl. Peds. (#/hr) 20 20 20 20 Confl. Bikes (#/hr) 15 15 15 Heavy Vehicles (%) 2% 2% 1% 1% 1% 1% Bus Blockages (#/hr) 0 0 0<
Fit Protected 0.95 1.00 0.95 1.00 1.00 Satd. Flow (prot) 1630 1352 1646 1705 3127 Fit Permitted 0.95 1.00 0.95 1.00 1.00 Satd. Flow (perm) 1630 1352 1646 1705 3127 Peak-hour factor, PHF 0.90 0.90 0.90 0.90 0.90 0.90 Adj. Flow (vph) 106 100 28 672 567 117 RTOR Reduction (vph) 0 82 0 0 13 0 Lane Group Flow (vph) 106 18 28 672 671 0 Confl. Peds. (#/hr) 20 20 20 20 Confl. Peds. (#/hr) 15 15 15 Heavy Vehicles (%) 2% 2% 1% 1% 1% 1% Bus Blockages (#/hr) 0 0 4 4 0 1 Turn Type NA Perm
Satd. Flow (prot) 1630 1352 1646 1705 3127 Flt Permitted 0.95 1.00 0.95 1.00 1.00 Satd. Flow (perm) 1630 1352 1646 1705 3127 Peak-hour factor, PHF 0.90 0.90 0.90 0.90 0.90 0.90 Adj. Flow (vph) 106 100 28 672 567 117 RTOR Reduction (vph) 0 82 0 0 13 0 Lane Group Flow (vph) 106 18 28 672 671 0 Confl. Peds. (#/hr) 20 20 20 20 Confl. Bikes (#/hr) 15 15 15 Heavy Vehicles (%) 2% 2% 1% 1% 1% Bus Blockages (#/hr) 0 0 0 4 4 0 Turn Type NA Perm Prot NA NA Protected Phases 2 3 8 4 Permitted Phases 2 2 2 4.7 90.0 </td
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Lane Group Flow (vph) 106 18 28 672 671 0 Confl. Peds. (#/hr) 20 20 20 20 Confl. Bikes (#/hr) 15 15 15 Heavy Vehicles (%) 2% 2% 1% 1% 1% Bus Blockages (#/hr) 0 0 0 4 4 0 Turn Type NA Perm Prot NA NA Permitted Phases 2 3 8 4 Permitted Phases 2 3 8 4 Permitted Phases 2 2 4.7 90.0 81.3 Effective Green, G (s) 22.0 22.0 4.7 90.0 81.3 Effective Green, g (s) 22.0 22.0 4.7 90.0 81.3 Actuated g/C Ratio 0.18 0.18 0.04 0.75 0.68 Clearance Time (s) 4.0 4.0 4.0 4.0 4.0 Vehicle Extension (s) 3.0 3.0 3.0 3.0 3.0 Lane Grp Cap (vph) <
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Effective Green, g (s) 22.0 22.0 4.7 90.0 81.3 Actuated g/C Ratio 0.18 0.18 0.04 0.75 0.68 Clearance Time (s) 4.0 4.0 4.0 4.0 Vehicle Extension (s) 3.0 3.0 3.0 3.0 Lane Grp Cap (vph) 298 247 64 1278 2118 v/s Ratio Prot c0.07 0.02 c0.39 0.21 v/s Ratio Perm 0.01 v/c Ratio 0.36 0.07 0.44 0.53 0.32
Actuated g/C Ratio 0.18 0.18 0.04 0.75 0.68 Clearance Time (s) 4.0 4.0 4.0 4.0 4.0 Vehicle Extension (s) 3.0 3.0 3.0 3.0 3.0 Lane Grp Cap (vph) 298 247 64 1278 2118 v/s Ratio Prot c0.07 0.02 c0.39 0.21 v/s Ratio Perm 0.01 v/c Ratio 0.36 0.07 0.44 0.53 0.32
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v/s Ratio Perm 0.01 v/c Ratio 0.36 0.07 0.44 0.53 0.32
v/c Ratio 0.36 0.07 0.44 0.53 0.32
Progression Factor 1.00 1.00 1.41 0.24 1.00
Incremental Delay, d2 3.3 0.6 4.0 1.3 0.4
Delay (s) 46.1 41.2 83.6 2.8 8.3
Level of Service D D F A A
Approach Delay (s) 43.7 6.0 8.3
Approach LOS D A A
Intersection Summary
HCM 2000 Control Delay 11.9 HCM 2000 Level of Service B
HCM 2000 Volume to Capacity ratio 0.51
Actuated Cycle Length (s) 12.0 Sum of lost time (s) 12.0
Intersection Capacity Utilization 54.6% ICU Level of Service A
Analysis Period (min) 15
c Critical Lane Group

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ř	†	7	ř	†	7	ሻ	f÷		۲	↑ Ъ	
Volume (vph)	105	245	215	75	265	65	250	385	45	115	355	70
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Lane Width	12	10	10	10	10	11	11	10	11	12	12	12
Total Lost time (s)	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5		4.5	4.5	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00		1.00	0.95	
Frpb, ped/bikes	1.00	1.00	0.94	1.00	1.00	0.89	1.00	0.99		1.00	0.98	
Flpb, ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00		1.00	1.00	
Frt	1.00	1.00	0.85	1.00	1.00	0.85	1.00	0.98		1.00	0.98	
Flt Protected	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00		0.95	1.00	
Satd. Flow (prot)	1646	1601	1286	1492	1617	1255	1560	1546		1607	3160	
Flt Permitted	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00		0.95	1.00	
Satd. Flow (perm)	1646	1601	1286	1492	1617	1255	1560	1546		1607	3160	
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	117	272	239	83	294	72	278	428	50	128	394	78
RTOR Reduction (vph)	0	0	154	0	0	57	0	3	0	0	12	0
Lane Group Flow (vph)	117	272	85	83	294	15	278	475	0	128	460	0
Confl. Peds. (#/hr)	20		20	20		20	20		20	20		20
Confl. Bikes (#/hr)			20			20			20			20
Heavy Vehicles (%)	1%	2%	1%	4%	1%	2%	3%	3%	3%	1%	1%	1%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	10	6	0	0
Turn Type	Prot	NA	Perm	Prot	NA	Perm	Prot	NA		Prot	NA	
Protected Phases	5	2		1	6		3	8		7	4	
Permitted Phases			2			6						
Actuated Green, G (s)	10.8	26.8	26.8	8.9	24.9	24.9	24.3	54.2		12.1	42.0	
Effective Green, g (s)	10.8	26.8	26.8	8.9	24.9	24.9	24.3	54.2		12.1	42.0	
Actuated g/C Ratio	0.09	0.22	0.22	0.07	0.21	0.21	0.20	0.45		0.10	0.35	
Clearance Time (s)	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5		4.5	4.5	
Vehicle Extension (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0		2.0	2.0	
Lane Grp Cap (vph)	148	357	287	110	335	260	315	698		162	1106	
v/s Ratio Prot	c0.07	0.17		0.06	c0.18		c0.18	c0.31		0.08	0.15	
v/s Ratio Perm			0.07			0.01						
v/c Ratio	0.79	0.76	0.30	0.75	0.88	0.06	0.88	0.68		0.79	0.42	
Uniform Delay, d1	53.5	43.6	38.8	54.5	46.1	38.1	46.5	26.0		52.7	29.7	
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00		1.07	0.81	
Incremental Delay, d2	22.9	8.4	0.2	22.6	21.3	0.0	23.3	5.3		20.6	1.1	
Delay (s)	76.4	52.0	39.0	77.0	67.4	38.2	69.8	31.3		76.7	25.2	
Level of Service	E	D	D	E	E	D	E	C		Е	C	
Approach LOS		51.6			64.5			45.5			36.2	
Approach LOS		D			E			D			D	
Intersection Summary												
HCM 2000 Control Delay			48.3	Н	CM 2000	Level of	Service		D			
HCM 2000 Volume to Capac	city ratio		0.80									
Actuated Cycle Length (s)			120.0		um of los				18.0			
Intersection Capacity Utilizat	tion		70.1%	IC	CU Level	of Service)		С			
Analysis Period (min)			15									
c Critical Lane Group												

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Movement	WBL2	WBL	WBR	NBL	NBT	NBR	SBL	SBT	SBR	NEL	NER	NER2
Lane Configurations		M			4			4	7	M		
Volume (vph)	5	10	20	5	245	5	30	335	620	370	5	5
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Lane Width	12	12	12	12	14	12	12	12	12	12	12	12
Total Lost time (s)		4.0			4.0			4.0	5.0	5.0		
Lane Util. Factor		1.00			1.00			1.00	1.00	1.00		
Frpb, ped/bikes		0.97			1.00			1.00	0.98	1.00		
Flpb, ped/bikes		0.99			1.00			1.00	1.00	1.00		
Frt		0.92			1.00			1.00	0.85	1.00		
Flt Protected		0.98			1.00			1.00	1.00	0.95		
Satd. Flow (prot)		1507			1822			1724	1441	1636		
Flt Permitted		0.98			0.99			0.96	1.00	1.00		
Satd. Flow (perm)		1507			1808			1663	1441	1716		
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	6	11	22	6	272	6	33	372	689	411	6	6
RTOR Reduction (vph)	0	29	0	0	1	0	0	0	0	11	0	0
Lane Group Flow (vph)	0	10	0	0	283	0	0	405	689	412	0	0
Confl. Peds. (#/hr)	10	10	10	10	200	10	10	100	10	10		10
Confl. Bikes (#/hr)	10		10	10		10	10		10	10		10
Heavy Vehicles (%)	1%	1%	1%	1%	2%	1%	1%	1%	1%	1%	2%	1%
Turn Type	Perm	NA	170	Perm	NA	170	Perm	NA	custom	NA	270	170
Protected Phases	I CIIII	8		I CIIII	2		I CIIII	6	Custom	INA		
Permitted Phases	8	U		2	Z		6	U	3 6	3		
Actuated Green, G (s)	U	10.8		2	17.3		U	17.3	41.7	15.4		
Effective Green, g (s)		10.8			17.3			17.3	37.7	15.4		
Actuated g/C Ratio		0.26			0.41			0.41	0.90	0.37		
Clearance Time (s)		4.0			4.0			4.0	0.70	5.0		
Vehicle Extension (s)		2.5			2.5			2.5		2.0		
Lane Grp Cap (vph)		390			750			689	1302	633		
v/s Ratio Prot		390			750			009	1302	033		
v/s Ratio Prot v/s Ratio Perm		0.01			0.16			0.24	c0.48	c0.24		
v/c Ratio		0.01			0.16			0.24	0.53			
Uniform Delay, d1		11.5			8.5			9.4		0.65 10.9		
J ·									0.4			
Progression Factor		1.00			1.00			1.00	1.00	1.00		
Incremental Delay, d2		0.0			0.2 8.7			1.1	0.2	1.8		
Delay (s)		11.5						10.5	0.5	12.8		
Level of Service		11 F			Α			B	Α	B		
Approach LOS		11.5			8.7			4.2		12.8		
Approach LOS		В			А			А		В		
Intersection Summary												
HCM 2000 Control Delay			7.0	Н	CM 2000	Level of S	Service		Α			
HCM 2000 Volume to Capac	city ratio		0.67									
Actuated Cycle Length (s)			41.7		um of lost				11.0			
Intersection Capacity Utilizat	ion		73.8%	IC	CU Level of	of Service	!		D			
Analysis Period (min)			15									
c Critical Lane Group												

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4	7		4	7		4T)	
Volume (vph)	15	55	15	165	55	35	5	455	155	35	630	15
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Lane Width	12	14	12	12	12	12	12	12	12	12	12	12
Total Lost time (s)		4.5			4.5	4.5		4.5	4.5		4.5	
Lane Util. Factor		1.00			1.00	1.00		1.00	1.00		0.95	
Frpb, ped/bikes		0.99			1.00	0.94		1.00	0.93		1.00	
Flpb, ped/bikes		1.00			0.99	1.00		1.00	1.00		1.00	
Frt		0.98			1.00	0.85		1.00	0.85		1.00	
Flt Protected		0.99			0.96	1.00		1.00	1.00		1.00	
Satd. Flow (prot)		1770			1657	1389		1732	1353		3267	
Flt Permitted		0.93			0.75	1.00		0.99	1.00		0.91	
Satd. Flow (perm)		1663			1296	1389		1720	1353		2983	
Peak-hour factor, PHF	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89
Adj. Flow (vph)	17	62	17	185	62	39	6	511	174	39	708	17
RTOR Reduction (vph)	0	12	0	0	0	27	0	0	73	0	1	0
Lane Group Flow (vph)	0	84	0	0	247	12	0	517	101	0	763	0
Confl. Peds. (#/hr)	15		10	10		15	10		15	15		10
Confl. Bikes (#/hr)			20			5			25			20
Heavy Vehicles (%)	1%	1%	1%	1%	1%	1%	1%	1%	1%	1%	1%	1%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	4	0	0	0
Turn Type	Perm	NA		Perm	NA	Perm	Perm	NA	Perm	Perm	NA	
Protected Phases		2			6			8			4	
Permitted Phases	2			6		6	8		8	4		
Actuated Green, G (s)		22.4			22.4	22.4		43.6	43.6		43.6	
Effective Green, g (s)		22.4			22.4	22.4		43.6	43.6		43.6	
Actuated g/C Ratio		0.30			0.30	0.30		0.58	0.58		0.58	
Clearance Time (s)		4.5			4.5	4.5		4.5	4.5		4.5	
Vehicle Extension (s)		3.0			3.0	3.0		2.0	2.0		2.0	
Lane Grp Cap (vph)		496			387	414		999	786		1734	
v/s Ratio Prot		0.05			0.40	0.01		0.00	0.07		0.07	
v/s Ratio Perm		0.05			c0.19	0.01		c0.30	0.07		0.26	
v/c Ratio		0.17			0.64	0.03		0.52	0.13		0.44	
Uniform Delay, d1		19.4			22.8	18.6		9.4	7.1		8.8	
Progression Factor		1.00			1.00	1.00		0.40	0.06		1.00	
Incremental Delay, d2		0.2			3.4	0.0		1.9	0.3		0.8	
Delay (s)		19.6			26.2	18.6		5.6	0.7		9.6	
Level of Service Approach Delay (s)		B 19.6			C 25.2	В		A 4.4	А		A 9.6	
7 . ,					25.2 C							
Approach LOS		В			C			А			А	
Intersection Summary												
HCM 2000 Control Delay			10.6	Н	CM 2000	Level of	Service		В			
HCM 2000 Volume to Capaci	ty ratio		0.56									
Actuated Cycle Length (s)			75.0		um of los				9.0			
Intersection Capacity Utilizati	on		75.0%	IC	U Level	of Service)		D			
Analysis Period (min)			15									
c Critical Lane Group												

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			414			4Th	
Volume (vph)	30	10	15	20	10	15	15	580	10	15	845	25
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Lane Width	12	14	12	12	14	12	12	12	12	12	12	12
Total Lost time (s)		4.5			4.5			4.5			4.5	
Lane Util. Factor		1.00			1.00			0.95			0.95	
Frpb, ped/bikes		0.99			0.99			1.00			1.00	
Flpb, ped/bikes		0.99			0.99			1.00			1.00	
Frt		0.96			0.95			1.00			1.00	
Flt Protected		0.97			0.98			1.00			1.00	
Satd. Flow (prot)		1702			1696			3250			3244	
Flt Permitted		0.86			0.89			0.93			0.94	
Satd. Flow (perm)		1496			1543			3016			3058	
Peak-hour factor, PHF	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Adj. Flow (vph)	32	11	16	21	11	16	16	617	11	16	899	27
RTOR Reduction (vph)	0	12	0	0	12	0	0	1	0	0	3	0
Lane Group Flow (vph)	0	47	0	0	36	0	0	643	0	0	939	0
Confl. Peds. (#/hr)	15		10	10		15	10		15	15		10
Confl. Bikes (#/hr)			10			10			15			15
Heavy Vehicles (%)	1%	1%	1%	1%	1%	1%	1%	1%	1%	1%	1%	1%
Bus Blockages (#/hr)	0	0	4	0	0	4	0	4	0	0	4	0
Turn Type	Perm	NA		Perm	NA		Perm	NA		Perm	NA	
Protected Phases		2			6			4			8	
Permitted Phases	2			6			4			8		
Actuated Green, G (s)		18.0			18.0			48.0			48.0	
Effective Green, g (s)		18.0			18.0			48.0			48.0	
Actuated g/C Ratio		0.24			0.24			0.64			0.64	
Clearance Time (s)		4.5			4.5			4.5			4.5	
Vehicle Extension (s)		3.0			3.0			2.0			2.0	
Lane Grp Cap (vph)		359			370			1930			1957	
v/s Ratio Prot												
v/s Ratio Perm		c0.03			0.02			0.21			c0.31	
v/c Ratio		0.13			0.10			0.33			0.48	
Uniform Delay, d1		22.4			22.2			6.2			7.0	
Progression Factor		1.00			1.00			1.47			0.93	
Incremental Delay, d2		0.2			0.1			0.4			0.8	
Delay (s)		22.5			22.3			9.5			7.3	
Level of Service		С			С			Α			А	
Approach Delay (s)		22.5			22.3			9.5			7.3	
Approach LOS		С			С			Α			Α	
Intersection Summary												
HCM 2000 Control Delay			9.1	H	CM 2000	Level of	Service		А			
HCM 2000 Volume to Capacit	y ratio		0.38									
Actuated Cycle Length (s)			75.0	Sı	um of lost	time (s)			9.0			
Intersection Capacity Utilization	n		60.4%		U Level o	. ,			В			
Analysis Period (min)			15									
c Critical Lane Group												

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	7	f)		ř	₽			€Î}•			4TÞ	
Volume (vph)	20	25	45	120	65	40	35	575	30	35	835	30
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Total Lost time (s)	4.5	4.5		4.5	4.5			4.5			4.5	
Lane Util. Factor	1.00	1.00		1.00	1.00			0.95			0.95	
Frpb, ped/bikes	1.00	0.97		1.00	0.98			1.00			1.00	
Flpb, ped/bikes	0.98	1.00		0.98	1.00			1.00			1.00	
Frt	1.00	0.90		1.00	0.94			0.99			0.99	
Flt Protected	0.95	1.00		0.95	1.00			1.00			1.00	
Satd. Flow (prot)	1595	1506		1609	1605			2971			2979	
Flt Permitted	0.68	1.00		0.71	1.00			0.87			0.91	
Satd. Flow (perm)	1150	1506		1199	1605			2592			2715	
Peak-hour factor, PHF	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Adj. Flow (vph)	21	27	48	128	69	43	37	612	32	37	888	32
RTOR Reduction (vph)	0	36	0	0	33	0	0	3	0	0	2	0
Lane Group Flow (vph)	21	39	0	128	79	0	0	678	0	0	955	0
Confl. Peds. (#/hr)	20		20	20		20	15		10	10		15
Confl. Bikes (#/hr)			10			10			10			10
Heavy Vehicles (%)	2%	2%	2%	1%	1%	1%	1%	3%	1%	1%	3%	1%
Bus Blockages (#/hr)	0	0	4	0	0	4	0	4	0	0	4	0
Parking (#/hr)							5	5	5	5	5	5
Turn Type	Perm	NA		Perm	NA		Perm	NA		Perm	NA	
Protected Phases		2			6			4			8	
Permitted Phases	2			6			4			8		
Actuated Green, G (s)	18.3	18.3		18.3	18.3			47.7			47.7	
Effective Green, g (s)	18.3	18.3		18.3	18.3			47.7			47.7	
Actuated g/C Ratio	0.24	0.24		0.24	0.24			0.64			0.64	
Clearance Time (s)	4.5	4.5		4.5	4.5			4.5			4.5	
Vehicle Extension (s)	3.0	3.0		3.0	3.0			2.0			2.0	
Lane Grp Cap (vph)	280	367		292	391			1648			1726	
v/s Ratio Prot	0.00	0.03		0.11	0.05			0.07			0.05	
v/s Ratio Perm	0.02	0.11		c0.11	0.00			0.26			c0.35	
v/c Ratio	0.07	0.11		0.44	0.20			0.41			0.55	
Uniform Delay, d1	21.8	22.0		24.0	22.6			6.7			7.7	
Progression Factor	1.00	1.00		1.00	1.00			1.00			0.92	
Incremental Delay, d2	0.1	0.1		1.1	0.3			0.8			1.2	
Delay (s)	21.9	22.1		25.1	22.8			7.5			8.2	
Level of Service	С	C 22.1		С	C 24.0			A 7.5			A 8.2	
Approach LOS		22.1 C			24.0 C							
Approach LOS		C			C			А			А	
Intersection Summary												
HCM 2000 Control Delay			10.6	H	CM 2000	Level of S	Service		В			
HCM 2000 Volume to Capac	city ratio		0.52									
Actuated Cycle Length (s)			75.0		um of lost				9.0			
Intersection Capacity Utiliza	tion		72.9%	IC	:U Level d	of Service			С			
Analysis Period (min)			15									
c Critical Lane Group												

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Movement	EBL	EBR	NBL	NBT	SBT	SBR	
Lane Configurations	ሻ	7		414	† ‡		
Volume (vph)	115	135	40	520	710	160	
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	
Total Lost time (s)	4.0	4.0		4.0	4.0	.,,,,	
Lane Util. Factor	1.00	1.00		0.95	0.95		
Frpb, ped/bikes	1.00	0.96		1.00	0.98		
Flpb, ped/bikes	1.00	1.00		1.00	1.00		
Frt	1.00	0.85		1.00	0.97		
Flt Protected	0.95	1.00		1.00	1.00		
Satd. Flow (prot)	1630	1397		3249	3097		
Flt Permitted	0.95	1.00		0.83	1.00		
Satd. Flow (perm)	1630	1397		2715	3097		
Peak-hour factor, PHF	0.97	0.97	0.97	0.97	0.97	0.97	
Adj. Flow (vph)	119	139	41	536	732	165	
RTOR Reduction (vph)	0	102	0	0	16	0	
Lane Group Flow (vph)	119	37	0	577	881	0	
Confl. Peds. (#/hr)	10	10	30			30	
Confl. Bikes (#/hr)		10				10	
Heavy Vehicles (%)	2%	2%	1%	1%	1%	1%	
Bus Blockages (#/hr)	0	0	0	4	4	0	
Turn Type	NA	Perm	Perm	NA	NA		
Protected Phases	2			8	4		
Permitted Phases		2	8				
Actuated Green, G (s)	32.0	32.0		80.0	80.0		
Effective Green, g (s)	32.0	32.0		80.0	80.0		
Actuated g/C Ratio	0.27	0.27		0.67	0.67		
Clearance Time (s)	4.0	4.0		4.0	4.0		
Vehicle Extension (s)	3.0	3.0		3.0	3.0		
Lane Grp Cap (vph)	434	372		1810	2064		
v/s Ratio Prot	c0.07				c0.28		
v/s Ratio Perm		0.03		0.21			
v/c Ratio	0.27	0.10		0.32	0.43		
Uniform Delay, d1	34.8	33.1		8.5	9.3		
Progression Factor	1.00	1.00		0.20	1.00		
Incremental Delay, d2	1.6	0.5		0.4	0.6		
Delay (s)	36.4	33.7		2.1	10.0		
Level of Service	D	С		Α	Α		
Approach Delay (s)	34.9			2.1	10.0		
Approach LOS	С			Α	Α		
• •							
Intersection Summary			11 1	1.1.	CN 4 2000	Lavel of Camilan	
HCM 2000 Control Delay	- 11 11 -		11.1	H	CIVI 2000	Level of Service	
HCM 2000 Volume to Capac	city ratio		0.38	_	um of last	time o (o)	
Actuated Cycle Length (s)	tion.		120.0		um of lost		
Intersection Capacity Utilizat	uon		67.5%	IC	CU Level c	oi Service	
Analysis Period (min)			15				
c Critical Lane Group							

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	7	†	7	7	†	7	Ŋ	∱ ∱		ሻ	↑ Ъ	
Volume (vph)	140	265	255	105	340	75	295	360	55	175	485	80
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Lane Width	12	10	10	10	10	11	11	10	11	12	12	12
Total Lost time (s)	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5		4.5	4.5	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95		1.00	0.95	
Frpb, ped/bikes	1.00	1.00	0.97	1.00	1.00	0.94	1.00	0.99		1.00	0.99	
Flpb, ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00		1.00	1.00	
Frt	1.00	1.00	0.85	1.00	1.00	0.85	1.00	0.98		1.00	0.98	
Flt Protected	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00		0.95	1.00	
Satd. Flow (prot)	1646	1601	1330	1492	1617	1326	1560	2922		1607	3179	
Flt Permitted	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00		0.95	1.00	
Satd. Flow (perm)	1646	1601	1330	1492	1617	1326	1560	2922		1607	3179	
Peak-hour factor, PHF	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Adj. Flow (vph)	146	276	266	109	354	78	307	375	57	182	505	83
RTOR Reduction (vph)	0	0	61	0	0	59	0	9	0	0	10	0
Lane Group Flow (vph)	146	276	205	109	354	19	307	423	0	182	578	0
Confl. Peds. (#/hr)	15		25	25		15	20		20	20		20
Confl. Bikes (#/hr)			15			25			5			15
Heavy Vehicles (%)	1%	2%	1%	4%	1%	2%	3%	3%	3%	1%	1%	1%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	10	6	0	0
Turn Type	Prot	NA	pm+ov	Prot	NA	Perm	Prot	NA		Prot	NA	
Protected Phases	5	2	3	1	6		3	8		7	4	
Permitted Phases			2			6						
Actuated Green, G (s)	13.2	30.9	57.1	11.3	29.0	29.0	26.2	42.4		17.4	33.6	
Effective Green, g (s)	13.2	30.9	57.1	11.3	29.0	29.0	26.2	42.4		17.4	33.6	
Actuated g/C Ratio	0.11	0.26	0.48	0.09	0.24	0.24	0.22	0.35		0.14	0.28	
Clearance Time (s)	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5		4.5	4.5	
Vehicle Extension (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0		2.0	2.0	
Lane Grp Cap (vph)	181	412	682	140	390	320	340	1032		233	890	
v/s Ratio Prot	c0.09	0.17	0.07	0.07	c0.22	0.04	c0.20	0.14		0.11	c0.18	
v/s Ratio Perm	0.04	0.77	0.09	0.70	0.01	0.01	0.00	0.14		0.70	0.45	
v/c Ratio	0.81	0.67	0.30	0.78	0.91	0.06	0.90	0.41		0.78	0.65	
Uniform Delay, d1	52.2	40.0	19.2	53.1	44.2	35.0	45.7	29.3		49.5	38.0	
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00		1.28	0.83	
Incremental Delay, d2	21.3	3.2	0.1	21.6	23.7	0.0	25.5	1.2		13.5	3.4	
Delay (s)	73.5	43.2	19.3	74.7	67.9	35.0	71.2	30.5		76.7	34.8	
Level of Service	E	D	В	E	E	D	E	C		E	C	
Approach LOS		40.4			64.5			47.4			44.7	
Approach LOS		D			E			D			D	
Intersection Summary												
HCM 2000 Control Delay			48.3	Н	CM 2000	Level of	Service		D			
HCM 2000 Volume to Capac	city ratio		0.81									
Actuated Cycle Length (s)			120.0	S	um of los	t time (s)			18.0			
Intersection Capacity Utiliza	tion		78.2%	IC	CU Level	of Service			D			
Analysis Period (min)			15									
c Critical Lane Group												

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Movement	WBL2	WBL	WBR	NBL	NBT	NBR	SBL	SBT	SBR	NEL	NER	NER2
Lane Configurations		M			4			4	7	M		
Volume (vph)	5	10	25	5	270	5	20	280	535	385	5	5
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Lane Width	12	12	12	12	14	12	12	12	12	12	12	12
Total Lost time (s)		4.0			4.0			4.0	5.0	5.0		
Lane Util. Factor		1.00			1.00			1.00	1.00	1.00		
Frpb, ped/bikes		0.97			1.00			1.00	0.98	1.00		
Flpb, ped/bikes		0.99			1.00			1.00	1.00	1.00		
Frt		0.92			1.00			1.00	0.85	1.00		
Flt Protected		0.98			1.00			1.00	1.00	0.95		
Satd. Flow (prot)		1497			1824			1726	1442	1637		
Flt Permitted		0.98			0.99			0.97	1.00	1.00		
Satd. Flow (perm)		1497			1814			1676	1442	1717		
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	5	11	27	5	293	5	22	304	582	418	5	5
RTOR Reduction (vph)	0	31	0	0	1	0	0	0	0	10	0	0
Lane Group Flow (vph)	0	12	0	0	302	0	0	326	582	418	0	0
Confl. Peds. (#/hr)	10	12	10	10	002	10	10	020	10	10		10
Confl. Bikes (#/hr)	10		10	10		10	10		10	10		10
Heavy Vehicles (%)	1%	1%	1%	1%	2%	1%	1%	1%	1%	1%	2%	1%
Turn Type	Perm	NA	170	Perm	NA	170	Perm	NA	custom	NA	270	170
Protected Phases	I CIIII	8		I CIIII	2		I CIIII	6	cusioni	INA		
Permitted Phases	8	U		2	Z		6	U	3 6	3		
Actuated Green, G (s)	U	10.7			14.6		U	14.6	38.7	15.1		
Effective Green, g (s)		10.7			14.6			14.6	34.7	15.1		
Actuated g/C Ratio		0.28			0.38			0.38	0.90	0.39		
Clearance Time (s)		4.0			4.0			4.0	0.70	5.0		
Vehicle Extension (s)		2.5			2.5			2.5		2.0		
Lane Grp Cap (vph)		413			684			632	1292	669		
v/s Ratio Prot		413			004			032	1292	009		
v/s Ratio Prot v/s Ratio Perm		0.01			0.17			0.19	c0.40	c0.24		
v/c Ratio		0.01			0.17			0.19	0.45	0.62		
Uniform Delay, d1		10.2			9.0			9.3	0.45	9.5		
J .												
Progression Factor		1.00			1.00			1.00	1.00	1.00		
Incremental Delay, d2		0.0			0.3 9.3			0.5	0.1	1.3		
Delay (s)		10.2						9.8	0.4	10.8		
Level of Service		B			A			A	Α	B		
Approach LOS		10.2			9.3			3.8		10.8		
Approach LOS		В			А			А		В		
Intersection Summary												
HCM 2000 Control Delay			6.8	Н	CM 2000	Level of S	Service		Α			
HCM 2000 Volume to Capac	ity ratio		0.62									
Actuated Cycle Length (s)			38.7		um of lost				11.0			
Intersection Capacity Utilizat	ion		65.9%	IC	CU Level o	of Service	;		С			
Analysis Period (min)			15									
c Critical Lane Group												

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4	7	۲	f)		ř	4î	
Volume (vph)	15	55	15	165	55	35	5	455	155	35	630	15
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Lane Width	12	14	12	12	12	12	12	12	12	12	12	12
Total Lost time (s)		4.5			4.5	4.5	4.5	4.5		4.5	4.5	
Lane Util. Factor		1.00			1.00	1.00	1.00	1.00		1.00	1.00	
Frpb, ped/bikes		0.99			1.00	0.94	1.00	0.99		1.00	1.00	
Flpb, ped/bikes		1.00			0.99	1.00	0.99	1.00		0.99	1.00	
Frt		0.98			1.00	0.85	1.00	0.96		1.00	1.00	
Flt Protected		0.99			0.96	1.00	0.95	1.00		0.95	1.00	
Satd. Flow (prot)		1761			1648	1388	1632	1643		1638	1724	
Flt Permitted		0.93			0.75	1.00	0.26	1.00		0.29	1.00	
Satd. Flow (perm)		1650			1291	1388	449	1643		494	1724	
Peak-hour factor, PHF	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89
Adj. Flow (vph)	17	62	17	185	62	39	6	511	174	39	708	17
RTOR Reduction (vph)	0	11	0	0	0	29	0	15	0	0	1	0
Lane Group Flow (vph)	0	85	0	0	247	10	6	670	0	39	724	0
Confl. Peds. (#/hr)	15		10	10		15	20		10	10		20
Confl. Bikes (#/hr)			20			5			25			20
Heavy Vehicles (%)	1%	1%	1%	1%	1%	1%	1%	1%	1%	1%	1%	1%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	4	0	0	0
Turn Type	Perm	NA		Perm	NA	Perm	Perm	NA		Perm	NA	
Protected Phases		2			6			8			4	
Permitted Phases	2			6		6	8			4		
Actuated Green, G (s)		20.1			20.1	20.1	45.9	45.9		45.9	45.9	
Effective Green, g (s)		20.1			20.1	20.1	45.9	45.9		45.9	45.9	
Actuated g/C Ratio		0.27			0.27	0.27	0.61	0.61		0.61	0.61	
Clearance Time (s)		4.5			4.5	4.5	4.5	4.5		4.5	4.5	
Vehicle Extension (s)		3.0			3.0	3.0	2.0	2.0		2.0	2.0	
Lane Grp Cap (vph)		442			345	371	274	1005		302	1055	
v/s Ratio Prot								0.41			c0.42	
v/s Ratio Perm		0.05			c0.19	0.01	0.01	0.47		0.08	0.40	
v/c Ratio		0.19			0.72	0.03	0.02	0.67		0.13	0.69	
Uniform Delay, d1		21.2			24.9	20.2	5.7	9.5		6.1	9.7	
Progression Factor		1.00			1.00	1.00	1.78	1.99		1.00	1.00	
Incremental Delay, d2		0.2			6.9	0.0	0.1	3.0		0.9	3.6	
Delay (s)		21.4			31.8	20.3	10.3	22.0		7.0	13.4	
Level of Service		C			C	С	В	C		А	B	
Approach LOS		21.4			30.2			21.9			13.0	
Approach LOS		С			С			С			В	
Intersection Summary												
HCM 2000 Control Delay			19.5	Н	CM 2000	Level of	Service		В			
HCM 2000 Volume to Capac	ity ratio		0.69									
Actuated Cycle Length (s)			75.0		um of los				9.0			
Intersection Capacity Utilizat	on		77.8%	IC	U Level	of Service)		D			
Analysis Period (min)			15									
c Critical Lane Group												

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4		ሻ	1>		ሻ	f)	
Volume (vph)	30	10	15	20	10	15	15	580	10	15	845	25
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Lane Width	12	14	12	12	14	12	12	12	12	12	12	12
Total Lost time (s)		4.5			4.5		4.5	4.5		4.5	4.5	
Lane Util. Factor		1.00			1.00		1.00	1.00		1.00	1.00	
Frpb, ped/bikes		0.98			0.98		1.00	1.00		1.00	1.00	
Flpb, ped/bikes		0.98			0.99		1.00	1.00		0.99	1.00	
Frt		0.96			0.95		1.00	1.00		1.00	1.00	
Flt Protected		0.97			0.98		0.95	1.00		0.95	1.00	
Satd. Flow (prot)		1680			1675		1646	1699		1630	1695	
Flt Permitted		0.86			0.89		0.16	1.00		0.34	1.00	
Satd. Flow (perm)		1478			1524		279	1699		580	1695	
Peak-hour factor, PHF	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Adj. Flow (vph)	32	11	16	21	11	16	16	617	11	16	899	27
RTOR Reduction (vph)	0	12	0	0	12	0	0	1	0	0	1	0
Lane Group Flow (vph)	0	47	0	0	36	0	16	627	0	16	925	0
Confl. Peds. (#/hr)	15		10	10		15	10		15	15		10
Confl. Bikes (#/hr)			10			10			15			15
Heavy Vehicles (%)	1%	1%	1%	1%	1%	1%	1%	1%	1%	1%	1%	1%
Bus Blockages (#/hr)	0	0	4	0	0	4	0	4	0	0	4	0
Turn Type	Perm	NA		Perm	NA		Perm	NA		Perm	NA	
Protected Phases		2			6			4			8	
Permitted Phases	2			6			4			8		
Actuated Green, G (s)		18.0			18.0		48.0	48.0		48.0	48.0	
Effective Green, g (s)		18.0			18.0		48.0	48.0		48.0	48.0	
Actuated g/C Ratio		0.24			0.24		0.64	0.64		0.64	0.64	
Clearance Time (s)		4.5			4.5		4.5	4.5		4.5	4.5	
Vehicle Extension (s)		3.0			3.0		2.0	2.0		2.0	2.0	
Lane Grp Cap (vph)		354			365		178	1087		371	1084	
v/s Ratio Prot								0.37			c0.55	
v/s Ratio Perm		c0.03			0.02		0.06			0.03		
v/c Ratio		0.13			0.10		0.09	0.58		0.04	0.85	
Uniform Delay, d1		22.4			22.2		5.2	7.7		5.0	10.7	
Progression Factor		1.00			1.00		1.77	1.95		0.73	0.83	
Incremental Delay, d2		0.2			0.1		8.0	1.8		0.2	6.7	
Delay (s)		22.5			22.3		9.9	16.8		3.8	15.5	
Level of Service		С			С		Α	В		Α	В	
Approach Delay (s)		22.5			22.3			16.7			15.3	
Approach LOS		С			С			В			В	
Intersection Summary												
HCM 2000 Control Delay			16.3	Н	CM 2000	Level of	Service		В			
HCM 2000 Volume to Capacity	ratio		0.66									
Actuated Cycle Length (s)			75.0	Sı	um of lost	time (s)			9.0			
Intersection Capacity Utilization	1		72.5%		U Level o				С			
Analysis Period (min)			15									
c Critical Lane Group												

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ħ	f)		۲	₽		ř	₽		ሻ	f)	
Volume (vph)	20	25	45	120	65	40	35	575	30	35	835	30
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Total Lost time (s)	4.5	4.5		4.5	4.5		4.5	4.5		4.5	4.5	
Lane Util. Factor	1.00	1.00		1.00	1.00		1.00	1.00		1.00	1.00	
Frpb, ped/bikes	1.00	0.95		1.00	0.97		1.00	1.00		1.00	1.00	
Flpb, ped/bikes	0.96	1.00		0.96	1.00		1.00	1.00		0.99	1.00	
Frt	1.00	0.90		1.00	0.94		1.00	0.99		1.00	0.99	
Flt Protected	0.95	1.00		0.95	1.00		0.95	1.00		0.95	1.00	
Satd. Flow (prot)	1572	1474		1584	1584		1440	1450		1431	1453	
Flt Permitted	0.68	1.00		0.71	1.00		0.18	1.00		0.34	1.00	
Satd. Flow (perm)	1133	1474		1181	1584		279	1450		514	1453	
Peak-hour factor, PHF	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Adj. Flow (vph)	21	27	48	128	69	43	37	612	32	37	888	32
RTOR Reduction (vph)	0	38	0	0	30	0	0	3	0	0	2	0
Lane Group Flow (vph)	21	37	0	128	82	0	37	641	0	37	918	0
Confl. Peds. (#/hr)	20		20	20		20	15		10	10		15
Confl. Bikes (#/hr)			10			10			10			10
Heavy Vehicles (%)	2%	2%	2%	1%	1%	1%	1%	3%	1%	1%	3%	1%
Bus Blockages (#/hr)	0	0	4	0	0	4	0	4	0	0	4	0
Parking (#/hr)							5	5	5	5	5	5
Turn Type	Perm	NA		Perm	NA		Perm	NA		Perm	NA	
Protected Phases		2			6			4			8	
Permitted Phases	2			6			4			8		
Actuated Green, G (s)	16.0	16.0		16.0	16.0		50.0	50.0		50.0	50.0	
Effective Green, g (s)	16.0	16.0		16.0	16.0		50.0	50.0		50.0	50.0	
Actuated g/C Ratio	0.21	0.21		0.21	0.21		0.67	0.67		0.67	0.67	
Clearance Time (s)	4.5	4.5		4.5	4.5		4.5	4.5		4.5	4.5	
Vehicle Extension (s)	3.0	3.0		3.0	3.0		2.0	2.0		2.0	2.0	
Lane Grp Cap (vph)	241	314		251	337		186	966		342	968	
v/s Ratio Prot		0.03			0.05			0.44			c0.63	
v/s Ratio Perm	0.02			c0.11			0.13			0.07		
v/c Ratio	0.09	0.12		0.51	0.24		0.20	0.66		0.11	0.95	
Uniform Delay, d1	23.6	23.8		26.0	24.5		4.8	7.5		4.5	11.3	
Progression Factor	1.00	1.00		1.00	1.00		1.00	1.00		0.15	0.33	
Incremental Delay, d2	0.2	0.2		1.6	0.4		2.4	3.6		0.4	12.6	
Delay (s)	23.8	24.0		27.7	24.9		7.2	11.1		1.0	16.3	
Level of Service	С	C		С	C		А	B		Α	B	
Approach Delay (s)		23.9			26.4			10.9			15.7	
Approach LOS		С			С			В			В	
Intersection Summary												
HCM 2000 Control Delay			15.7	H	CM 2000	Level of S	Service		В			
HCM 2000 Volume to Capacity ratio			0.84									
Actuated Cycle Length (s)		75.0		um of lost				9.0				
Intersection Capacity Utilization			72.2%	IC	U Level o	of Service			С			
Analysis Period (min)			15									
c Critical Lane Group												

Intersection Capacity Utilization 54.0% ICU Level of Service		•	•	•	†	↓	✓	
Lane Configurations	Movement	EBL	EBR	NBL	NBT	SBT	SBR	
Volume (vph)		*	7	ሻ	†	∱ Ъ		
Ideal Flow (vphpl)		115	135	40	520		160	
Total Lost time (s)								
Lane Utili. Factor 1.00 1.00 1.00 1.00 0.95 Frpb. ped/bikes 1.00 1.00 1.00 1.00 0.98 Frpb. ped/bikes 1.00 1.00 1.00 1.00 1.00 Frt 1.00 0.85 1.00 1.00 0.97 Fil Protected 0.95 1.00 0.95 1.00 0.97 Fil Protected 0.95 1.00 0.95 1.00 1.00 Satd. Flow (prot) 1630 1395 1646 1705 3096 Fil Permitted 0.95 1.00 0.95 1.00 1.00 Satd. Flow (perm) 1630 1395 1646 1705 3096 Fil Permitted 0.95 1.00 0.95 1.00 1.00 Satd. Flow (perm) 1630 1395 1646 1705 3096 Fil Permitted 0.95 1.00 0.95 1.00 1.00 Satd. Flow (perm) 1630 1395 1646 1705 3096 Fil Permitted 0.95 1.00 0.95 1.00 1.00 Satd. Flow (perm) 1630 1395 1646 1705 3096 Fil Permitted 0.95 1.00 0.95 1.00 1.00 Satd. Flow (perm) 1630 1395 1646 1705 3096 Fil Permitted 0.95 1.00 0.95 1.00 1.00 Satd. Flow (perm) 1630 1395 1646 1705 3096 Fil Permitted 0.95 1.00 0.95 1.00 1.00 Satd. Flow (perm) 1630 1395 1646 1705 3096 Fil Permitted 0.95 1.00 0.95 1.00 1.00 Satd. Flow (perm) 10 0.97 0.97 0.97 0.97 0.97 Fil Protected Place 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.0								
Frpb, ped/bikes								
Flipb, ped/bikes 1.00 1.00 1.00 1.00 1.00 7Ft 1.00 1.00 0.85 1.00 1.00 0.97 Ftl Protected 0.95 1.00 0.95 1.00 1.00 0.97 Ftl Protected 0.95 1.00 0.95 1.00 1.00 0.97 Ftl Protected 0.95 1.00 0.95 1.00 1.00 0.95 1.00 1.00 Satd. Flow (prot) 1630 1395 1646 1705 3096 Ftl Permitted 0.95 1.00 0.95 1.00 1.00 Satd. Flow (prom) 1630 1395 1646 1705 3096 Peak-hour factor, PHF 0.97 0.97 0.97 0.97 0.97 0.97 0.97 0.97								
Fit Protected 0.95 1.00 0.85 1.00 1.00 0.97 Fit Protected 0.95 1.00 0.95 1.00 1.00 1.00 Satd. Flow (prot) 1630 1395 1646 1705 3096 Fit Permitted 0.95 1.00 0.95 1.00 1.00 Satd. Flow (perm) 1630 1395 1646 1705 3096 Peak-hour factor, PHF 0.97 0.97 0.97 0.97 0.97 0.97 Adj. Flow (vph) 119 139 41 536 732 165 RTOR Reduction (vph) 0 108 0 0 15 0 Lane Group Flow (vph) 119 31 41 536 882 0 Confl. Peds. (#/hr) 10 10 30 30 30 Confl. Bikes (#/hr) 10 10 30 30 Confl. Bikes (#/hr) 0 0 0 4 4 4 0 Turn Type NA Perm Prot NA NA Protected Phases 2 3 8 4 Permitted Phases 2 Rctuated Green, G (s) 27.0 27.0 7.2 85.0 73.8 Effective Green, g (s) 27.0 27.0 7.2 85.0 73.8 Effective Green, g (s) 27.0 27.0 7.2 85.0 73.8 Effective Green, g (s) 3.0 3.0 3.0 3.0 3.0 Lane Grp Cap (vph) 366 313 98 1207 1904 Vsh Ratio Prot C.0.07 0.20 Vsh Ratio Prot C.0.07 0.02 Vsh Ratio Prot C.0.07 0.02 Vs Ratio Prot C.0.07 0.03 Vs Ratio Prot C.0.07 0.03 Vs Ratio Prot C.0.07 0.04 Vs Ratio Prot C.0.07 0.05 Vs Ratio Prot C.								
Fit Protected 0.95 1.00 0.95 1.00 1.00 Satd. Flow (prot) 1630 1395 1646 1705 3096 Fit Permitted 0.95 1.00 0.95 1.00 1.00 Satd. Flow (perm) 1630 1395 1646 1705 3096 Satd. Flow (perm) 1963 1646 1705 3096 Satd. Flow (ph) 119 139 41 536 732 165 Satd. Flow (ph) 119 139 41 536 732 165 Satd. Flow (ph) 119 31 41 536 882 0 Satd. Flow (ph) 119 31 41 536 882 0 Satd. Flow (ph) 119 31 41 536 882 0 Satd. Flow (ph) 110 10 10 10 10 Satd. (#/hr) 10 10 10 Satd. (#/hr) 10 10 Satd. (#/hr) 10								
Satd. Flow (prot) 1630 1395 1646 1705 3096 FIL Permitted 0.95 1.00 0.95 1.00 1.00 Satd. Flow (perm) 1630 1395 1646 1705 3096 Peak-hour factor, PHF 0.97 0.97 0.97 0.97 0.97 0.97 Adj. Flow (yph) 119 139 41 536 732 165 RTOR Reduction (yph) 0 108 0 0 15 0 Lane Group Flow (yph) 119 31 41 536 882 0 Confl. Peds. (#/hr) 10 10 30 30 30 Confl. Bikes (#/hr) 10 10 30 30 30 Confl. Bikes (#/hr) 10 10 30 4 4 4 0 Heavy Vehicles (%) 2% 2% 1% 1% 1% 1% 1% Bus Blockages (#/hr) 0 0 0 4 4 0 Turn Type NA Perm Prot NA NA Protected Phases 2 3 8 4 Permitted Phases 2 3 8 4 Permitted Process 2 3 8 4 Permitted Process 2 3 8 4 Retuated Green, G (s) 27.0 27.0 7.2 85.0 73.8 Fifective Green, g (s) 27.0 27.0 7.2 85.0 73.8 Actuated Green, G (s) 4.0 4.0 4.0 4.0 4.0 Vehicle Extension (s) 3.0 3.0 3.0 3.0 3.0 Vehicle Extension (s) 3.0 3.0 3.0 3.0 3.0 Vehicle Extension (s) 3.0 3.0 3.0 3.0 3.0 Verice Protected Process 2 3 8 8 4 Progression Factor 1.00 1.00 1.50 0.12 1.00 Incremental Delay, d1 38.9 36.9 54.4 7.4 12.4 Progression Factor 1.00 1.00 1.50 0.12 1.00 Incremental Delay, d2 2.4 0.6 2.1 0.8 0.8 Delay (s) 41.2 37.5 83.7 1.7 13.2 Level of Service D D F A B Approach LOS D A B Intersection Summary HCM 2000 Control Delay 15.2 HCM 2000 Level of Service B HCM 2000 Volume to Capacity ratio Intersection Capacity (tillization 54.0% ICU Level of Service A								
Fit Permitted 0.95 1.00 0.95 1.00 1.00 Satd. Flow (perm) 1630 1395 1646 1705 3096 Peak-hour factor, PHF 0.97 0.97 0.97 0.97 0.97 0.97 0.97 RTOR Reduction (vph) 119 139 41 536 732 165 RTOR Reduction (vph) 10 108 0 0 15 0 Lane Group Flow (vph) 119 31 41 536 882 0 Confl. Peds. (#/hr) 10 10 30 30 30 Confl. Peds. (#/hr) 10 10 30 30 Heavy Vehicles (%) 2% 2% 1% 1% 1% 1% 1% 1% 1% 1% 1% 1% 1% 1% 1%								
Satd. Flow (perm) 1630 1395 1646 1705 3096 Peak-hour factor, PHF 0.97 0.97 0.97 0.97 0.97 Adj. Flow (vph) 119 139 41 536 732 165 RTOR Reduction (vph) 0 108 0 0 15 0 Lane Group Flow (vph) 119 31 41 536 882 0 Confl. Peds. (#/hr) 10 10 30 30 30 Confl. Bikes (#/hr) 10 10 10 10 Heavy Vehicles (%) 2% 2% 1% 1% 1% Bus Blockages (#/hr) 0 0 0 4 4 0 Turn Type NA Perm Prot NA NA Perm Prot NA NA Permitted Phases 2 3 8 4 Permitted Phases 2 3 8 4 Permitted Phases 2 73.8 Setutated Green, G (s)								
Peak-hour factor, PHF 0.97 0.97 0.97 0.97 0.97 0.97 Adj. Flow (vph) 119 139 41 536 732 165 RTOR Reduction (vph) 0 108 0 0 15 0 Lane Group Flow (vph) 119 31 41 536 882 0 Confl. Peds. (#/hr) 10 10 30 30 Confl. Bikes (#/hr) 10 10 10 10 Heavy Vehicles (%) 2% 2% 1% 1% 1% 1% Bus Blockages (#/hr) 0 0 4 4 0 1 Turn Type NA Perm Prot NA NA NA Permitted Phases 2 3 8 4 9 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 0 1 1 1 <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td>								
Adj. Flow (vph)							0.97	
RTOR Reduction (vph)								
Lane Group Flow (vph) 119 31 41 536 882 0 Confl. Peds. (#/hr) 10 10 30 30 Confl. Bikes (#/hr) 10 10 10 Heavy Vehicles (%) 2% 2% 1% 1% 1% 1% 1% Bus Blockages (#/hr) 0 0 0 0 4 4 0 Turn Type NA Perm Prot NA NA Protected Phases 2 3 8 4 Permitted Phases 2 3 8 4 Permitted Phases 2 7.0 27.0 7.2 85.0 73.8 Effective Green, G (s) 27.0 27.0 7.2 85.0 73.8 Effective Green, G (s) 27.0 27.0 7.2 85.0 73.8 Effective Green, G (s) 27.0 27.0 7.2 85.0 73.8 Actuated g/C Ratio 0.22 0.22 0.06 0.71 0.61 Clearance Time (s) 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0								
Confl. Peds. (#/hr) 10 10 30 30 30	· · · ·							
Confl. Bikes (#/hr) 10 10 10 10 10 10 10 10 10 10 10 10 10					300	302		
Heavy Vehicles (%)		10						
Bus Blockages (#/hr) 0 0 0 4 4 0 Turn Type NA Perm Prot NA NA Protected Phases 2 3 8 4 Permitted Phases 2 Actuated Green, G (s) 27.0 27.2 85.0 73.8 Effective Green, g (s) 27.0 27.0 7.2 85.0 73.8 Effective Green, g (s) 27.0 27.0 7.2 85.0 73.8 Effective Green, g (s) 27.0 27.0 7.2 85.0 73.8 Effective Green, g (s) 27.0 27.0 7.2 85.0 73.8 Effective Green, g (s) 27.0 27.0 7.2 85.0 73.8 Effective Green, g (s) 20.0 0.02 0.06 0.71 0.61 Clearance Time (s) 4.0 4.0 4.0 4.0 4.0 4.0 Vehicle Extension (s) 3.0 3.0 3.0 3.0 3.0 3.0 3.0 <t< td=""><td></td><td>2%</td><td></td><td>1%</td><td>1%</td><td>1%</td><td></td><td></td></t<>		2%		1%	1%	1%		
Turn Type	· · ·							
Protected Phases 2 3 8 4 Permitted Phases 2 Actuated Green, G (s) 27.0 27.0 7.2 85.0 73.8 Effective Green, g (s) 27.0 27.0 7.2 85.0 73.8 Actuated g/C Ratio 0.22 0.22 0.06 0.71 0.61 Clearance Time (s) 4.0 4.0 4.0 4.0 4.0 Vehicle Extension (s) 3.0 3.0 3.0 3.0 3.0 Lane Grp Cap (vph) 366 313 98 1207 1904 v/s Ratio Prot c0.07 0.02 c0.31 c0.28 v/c Ratio 0 0.33 0.10 0.42 0.44 0.46 Uniform Delay, d1 38.9 36.9 54.4 7.4 12.4 Progression Factor 1.00 1.00 1.50 0.12 1.00 Incremental Delay, d2 2.4 0.6 2.1 0.8 0.8 Delay (s) 41.2 37.5 83.7 1.7 13.2 Level of Service D D F A B Approach Delay (s) 39.2 7.6 13.2 Approach LOS D A B Intersection Summary HCM 2000 Control Delay 15.2 HCM 2000 Level of Service B HCM 2000 Volume to Capacity ratio 0.43 Actuated Cycle Length (s) 12.0 Incressection Capacity Utilization 54.0% ICU Level of Service A								
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Analysis Daried (min)		zation			IC	CU Level o	of Service	А
	Analysis Period (min)			15				
c Critical Lane Group	c Critical Lane Group							

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	†	7	ř	†	7	ሻ	f÷		Ŋ	↑ Ъ	
Volume (vph)	140	265	255	105	340	75	295	360	55	175	485	80
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Lane Width	12	10	10	10	10	11	11	10	11	12	12	12
Total Lost time (s)	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5		4.5	4.5	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00		1.00	0.95	
Frpb, ped/bikes	1.00	1.00	0.93	1.00	1.00	0.90	1.00	0.99		1.00	0.99	
Flpb, ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00		1.00	1.00	
Frt	1.00	1.00	0.85	1.00	1.00	0.85	1.00	0.98		1.00	0.98	
Flt Protected	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00		0.95	1.00	
Satd. Flow (prot)	1646	1601	1283	1492	1617	1270	1560	1538		1607	3180	
Flt Permitted	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00		0.95	1.00	
Satd. Flow (perm)	1646	1601	1283	1492	1617	1270	1560	1538		1607	3180	
Peak-hour factor, PHF	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Adj. Flow (vph)	146	276	266	109	354	78	307	375	57	182	505	83
RTOR Reduction (vph)	0	0	166	0	0	59	0	4	0	0	11	0
Lane Group Flow (vph)	146	276	100	109	354	19	307	428	0	182	577	0
Confl. Peds. (#/hr)	15		25	25		15	20		20	20		20
Confl. Bikes (#/hr)			15			25			5			15
Heavy Vehicles (%)	1%	2%	1%	4%	1%	2%	3%	3%	3%	1%	1%	1%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	10	6	0	0
Turn Type	Prot	NA	Perm	Prot	NA	Perm	Prot	NA		Prot	NA	
Protected Phases	5	2		1	6		3	8		7	4	
Permitted Phases			2			6						
Actuated Green, G (s)	12.1	29.6	29.6	11.3	28.8	28.8	25.4	45.8		15.3	35.7	
Effective Green, g (s)	12.1	29.6	29.6	11.3	28.8	28.8	25.4	45.8		15.3	35.7	
Actuated g/C Ratio	0.10	0.25	0.25	0.09	0.24	0.24	0.21	0.38		0.13	0.30	
Clearance Time (s)	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5		4.5	4.5	
Vehicle Extension (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0		2.0	2.0	
Lane Grp Cap (vph)	165	394	316	140	388	304	330	587		204	946	
v/s Ratio Prot	c0.09	0.17		0.07	c0.22		c0.20	c0.28		0.11	0.18	
v/s Ratio Perm			0.08			0.01						
v/c Ratio	0.88	0.70	0.32	0.78	0.91	0.06	0.93	0.73		0.89	0.61	
Uniform Delay, d1	53.3	41.2	36.9	53.1	44.4	35.2	46.4	31.8		51.5	36.2	
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00		1.11	0.82	
Incremental Delay, d2	37.9	4.5	0.2	21.6	24.8	0.0	31.7	7.7		32.2	2.7	
Delay (s)	91.2	45.7	37.2	74.7	69.2	35.2	78.1	39.5		89.4	32.5	
Level of Service	F	D	D	Е	Е	D	Е	D		F	С	
Approach Delay (s)		52.1			65.4			55.5			46.0	
Approach LOS		D			Е			E			D	
Intersection Summary												
HCM 2000 Control Delay		53.9	Н	CM 2000	Level of	Service		D				
HCM 2000 Volume to Capac		0.87										
Actuated Cycle Length (s)		120.0	S	um of los	t time (s)			18.0				
Intersection Capacity Utilizat		78.2%	IC	CU Level	of Service)		D				
Analysis Period (min)		15										
c Critical Lane Group												

	•	/	•	A)	†	<i>></i>	\	+	لِر	•	<i>></i>	4
Movement	WBL2	WBL	WBR	NBL	NBT	NBR	SBL	SBT	SBR	NEL	NER	NER2
Lane Configurations		M			4			4	7	M		
Volume (vph)	5	10	25	5	270	5	20	280	535	385	5	5
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Lane Width	12	12	12	12	14	12	12	12	12	12	12	12
Total Lost time (s)		4.0			4.0			4.0	5.0	5.0		
Lane Util. Factor		1.00			1.00			1.00	1.00	1.00		
Frpb, ped/bikes		0.97			1.00			1.00	0.98	1.00		
Flpb, ped/bikes		0.99			1.00			1.00	1.00	1.00		
Frt		0.92			1.00			1.00	0.85	1.00		
Flt Protected		0.98			1.00			1.00	1.00	0.95		
Satd. Flow (prot)		1497			1824			1726	1442	1637		
Flt Permitted		0.98			0.99			0.97	1.00	1.00		
Satd. Flow (perm)		1497			1814			1676	1442	1717		
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	5	11	27	5	293	5	22	304	582	418	5	5
RTOR Reduction (vph)	0	31	0	0	1	0	0	0	0	10	0	0
Lane Group Flow (vph)	0	12	0	0	302	0	0	326	582	418	0	0
Confl. Peds. (#/hr)	10	12	10	10	302	10	10	320	10	10	U	10
Confl. Bikes (#/hr)	10		10	10		10	10		10	10		10
Heavy Vehicles (%)	1%	1%	1%	1%	2%	1%	1%	1%	1%	1%	2%	1%
Turn Type	Perm	NA	1 70	Perm	NA	1 70	Perm		custom	NA	270	1 70
Protected Phases	reiiii	8		reiiii	2		reiiii	6	Custom	IVA		
Permitted Phases	8	Ü		2	Z		6	U	3 6	3		
Actuated Green, G (s)	O .	10.7			14.6		U	14.6	38.7	15.1		
Effective Green, g (s)		10.7			14.6			14.6	34.7	15.1		
Actuated g/C Ratio		0.28			0.38			0.38	0.90	0.39		
Clearance Time (s)		4.0			4.0			4.0	0.70	5.0		
Vehicle Extension (s)		2.5			2.5			2.5		2.0		
		413			684			632	1292	669		
Lane Grp Cap (vph) v/s Ratio Prot		413			004			032	1292	009		
v/s Ratio Perm		0.01			0.17			0.19	c0.40	c0.24		
v/c Ratio		0.01			0.17			0.19	0.45	0.62		
		10.2			9.0			9.3	0.45			
Uniform Delay, d1		1.00			1.00			1.00	1.00	9.5 1.00		
Progression Factor		0.0			0.3			0.5	0.1	1.00		
Incremental Delay, d2		10.2			9.3			9.8	0.1	10.8		
Delay (s) Level of Service		10.2 B								10.6 B		
Approach Delay (s)		10.2			A 9.3			A 3.8	А	10.8		
11		10.2 B								10.8 B		
Approach LOS		Б			A			А		Б		
Intersection Summary												
HCM 2000 Control Delay			6.8	H	CM 2000	Level of S	Service		Α			
HCM 2000 Volume to Capaci	ity ratio		0.62									
Actuated Cycle Length (s)			38.7		um of lost				11.0			
Intersection Capacity Utilizati	on		65.9%	IC	U Level o	of Service			С			
Analysis Period (min)			15									
c Critical Lane Group												

Appendix

- Simulation Results: Vehicle Queuing Summary

Estimated Vehicle Queuing Comparison for Alternatives - Future Year 2018

Interception	Movement/	Alterna	ative 1	Alternati	ve 3 or 5
Intersection	Direction	Average	95 th	Average	95 th
	SB (Thru/Right)	130	230	250	430
	SB (Left)*	180	290	80	280
	NB (Thru/Right)**	40	80	180	260
Willamette Street/ 24 th Avenue	NB (Thru/Left)**	100	170	30	110
	EB (All)	40	70	40	80
	WB (Right)	40	110	40	110
	WB (Thru/Left)	140	240	140	240
	EB (All)	30	70	30	70
	WB (All)	30	60	30	70
Willamette Street/	NB (Thru/Right)	70	150	220	370
25 th Avenue	NB (Left)*	100	190	20	80
	SB (Thru/Right)	140	230	290	570
	SB (Left)*	140	220	10	20
	EB (Left)	20	60	20	60
	EB (Thru/Right)	50	90	55	100
	WB (Left)	70	120	80	130
Willamette Street/	WB (Thru/Right)	80	160	80	160
27 th Avenue	SB (Thru/Right)	100	190	230	560
	SB (Left)*	80	160	30	100
	NB (Thru/Right)	90	160	160	290
	NB (Left)*	100	160	30	100
	EB (Left)	140	320	140	320
	EB (Right)	110	240	120	240
Willamette Street/	NB (Left)*	50	110	40	80
Willamette Plaza Driveway	NB (Thru)	30	90	40	100
	SB (Thru)	160	270	230	320
	SB (Thru/Right)	180	290	120	140
Willamette Street/	SB (Thru/Right)	220	300	220	300
29 th Avenue***	SB (Left)	160	240	130	190

Estimated Vehicle Queuing Comparison for Alternatives - Future Year 2018

	NB (Thru/Right)	140	300	310	530
	NB (Left)	190	270	220	300
	WB (Thru)	390	660	350	630
	WB (Left)	150	240	150	250
	WB (Right)	60	160	60	140
	EB (Thru)	350	670	420	760
	EB (Left)	140	230	140	240
	EB (Right)	120	190	130	180
	WB (32 nd Ave.)	20	40	20	40
	NB (Donald Street)	90	150	90	160
Willamette Street/ 32 nd Avenue	SB (Thru/Left)	170	280	170	290
	SB (Right)	10	80	10	70
	EB (Willamette Street)	120	200	120	190

Average = Average simulation queue length (feet) 95th = Ninety fifth percentile (highest five percent) simulation queue length (feet)

^{*} Shared left/through lane in Alternative 1, dedicated left turn lane in Alternatives 3 and 5.

^{**}Shared left/through lane and right lane in Alternative 1, shared right/through lane and left lane in Alternatives 3 and 5.

Appendix

– Traffic Signal Warrant @ Willamette St and Woodfield Station Driveway

10/3/2012

10/3/2012

Count Date (am)

Count Date (pm)

Warrants

Warrants Weight

OREGON DEPARTMENT OF TRANSPORTATION TRAFFIC SIGNAL WARRANT COMPARISON Willamette St Plaza Driveway

at M.P. 0

0

Analysis by: DKS Associates (DAM)

503-391-8773 Phone:

Eugene

Major Street has two approaching lanes Minor Street has one approaching lane 100% Warrants

100 % Reduction in minor st right turn volume

												0.00 % growth per year for			
	VOLUME DATA														
Time 14 Hours	6:00 - 7:00	7:00 - 8:00	8:00 - 9:00	9:00 - 10:00	10:00 - 11:00	11:00 - 12:00	12:00 - 13:00	13:00 - 14:00	14:00 - 15:00	15:00 - 16:00	16:00 - 17:00	17:00 - 18:00	18:00 - 19:00	19:00 20:00	
Hourly Volumes on Major Street	249	738	994	934	979	1098	1152	1110	1179	1338	1385	1378	1138	856	
Hourly Volumes on Minor Street	71	89	121	151	147	104	136	100	125	98	99	80	51	27	
Hourly Volumes other Minor St.	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
Pedestrians Xing Major Street only	0	2	6	4	0	0	0	0	0	0	2	6	7	0	

Wallants	Volume Warrant	Warrant is Met	met	Value
1. Eight-Hour Vehicular Volume:	· · · · · · · · · · · · · · · · · · ·		YES	25
Condition A: Minimum Vehicular Volume				
Major Street (total of both approaches)	600			
Minor Street (one direction only)	150	1 (8 hours required)		
Condition B: Interruption of Continous Traffic				
Major Street (total of both approaches)	900			
Minor Street (one direction only)	75	(8 hours required)	*	
**Condition A: Minimum Vehicular Volume, 80%				
Major Street (total of both approaches)	480			
Minor Street (one direction only)	120	5 (8 hours required)		
**Condition B: Interuption of Continuous Traffic, 80%				
Major Street (total of both approaches)	720			
Minor Street (one direction only)	60	11 (8 hours required)		
2. Four-Hour Vehicular Volume:			YES	5
	Graph attached	4 (4 hours required)		
3. Peak Hour:			NO	
Condition A:				
1.) Total stopped time delay on one minor street approach		vehicle-hours		
2.) Minor Street (one direction only)	100	0 (1 hour required)		
3.) Total entering volume serviced during the hour	650	0 (1 hour required)		
Condition B:	Graph attached	0 (1 hour required)		
4. Pedestrian Volume:			NO	
Condition A:				
Pedestrian Volume for each of any four hours	100	0 (4 hours required)		
2.) Pedestrian Volume during any hour	190	0 (1 hour required)		
Condition B:				

Minimum

Number of Hours

OREGON DEPARTMENT OF TRANSPORTATION TRAFFIC SIGNAL WARRANT COMPARISON Willamette St Plaza Driveway at

0

M.P. 0 Eugene

Major Street has two approaching lanes Minor Street has one approaching lane 100% Warrants

Phone:

100 % Reduction in minor st right turn volume 0.00 % growth per year for

Analysis by: DKS Associates (DAM)

503-391-8773

(required) see attached analysi

1.) Number of gaps per hour of adequate length:

10/3/2012

10/3/2012

Condition C:

Count Date (am)

Count Date (pm)

1.) Distance to nearest traffic signal along major street (ft):

300

60

300

10/3/2012

10/3/2012

Count Date (am)

Count Date (pm)

OREGON DEPARTMENT OF TRANSPORTATION TRAFFIC SIGNAL WARRANT COMPARISON Willamette St Plaza Driveway at

0 M.P. 0

Eugene

Analysis by: DKS Associates (DAM)

503-391-8773 Phone:

Major Street has two approaching lanes Minor Street has one approaching lane 100% Warrants

100 % Reduction in minor st right turn volume 0.00 % growth per year for 0 years

Warrants		Minimum Volume Warrant	Number of Hours Warrant is Met	v	Warrants met	Weight Value
5. School Cros					N/A	
Condition A;	 Frequency and adeqacy of gaps is not acceptable: Students crossing during the highest hour: 	N/A				
	3.) Other remedial measures have been considered:					
	4.) Distance to nearest traffic signal along major street (ft):	N/A				
6. Coordinated Condition A:	Signal System:				N/A	
Outuit.o.	Existing traffic signals do not provide the necessary degree of platooning:					
Condition B:	Existing traffic signals and proposed traffic signal will collectively provide a progressive operation:					
	2.) Distance to nearest traffic signal along major street (ft):	N/A				
7. Accident Ex	perience:				NO	
Condition A:	Adequate trial of alternatives with satisfactory observance and enforcement has failed to reduce the crash frequence		NO	(required) see attached analysis		
Condition B:	1.) Preventable Crashes within a 12 month period:	5	0	(5 crashes required)		
Condition C:	1.) 80% Vehicular Volume: Condition A (SEE WARRANT 1 80% Vehicular Volume: Condition B (SEE WARRANT 1		<u>5</u> 11	(8 hours required) (8 hours required)	*	
	2.) 80% Pedestrian Volume:	80 152	0	(4 hours required) (1 hour required)		
8. Roadway Ne	etwork:				N/A	
Condition A:	1.) Entering Volume (Weekday Peak Hour):	N/A				
	2.) 5-year projected volumes: Warrant 1 Condition A: Minimum Vehicular Volume Major Street (total of both approaches): Minor Street (one direction only):	N/A N/A				
	Condition B: Interruption of Continous Traffic Major Street (total of both approaches): Minor Street (one direction only):	N/A N/A				
	**Condition A: Minimum Vehicular Volume, 80% Major Street (total of both approaches): Minor Street (one direction only):	N/A N/A				
	**Condition B: Interuption of Continuous Traffic, 80% Major Street (total of both approaches): Minor Street (one direction only):	N/A N/A				
	3.) 5-year projected volumes: Warrant 2					
	 4.) 5-year projected volumes: Warrant 3 Condition A: Total stopped time delay on one minor street approach Minor Street (one direction only): Total entering volume serviced during the hour: Condition B: 	h : N/A N/A N/A	<u> </u>			

OREGON DEPARTMENT OF TRANSPORTATION TRAFFIC SIGNAL WARRANT COMPARISON
Willamette St at Plaza Driveway
0 M.P. 0

Analysis by: DKS Associates (DAM)

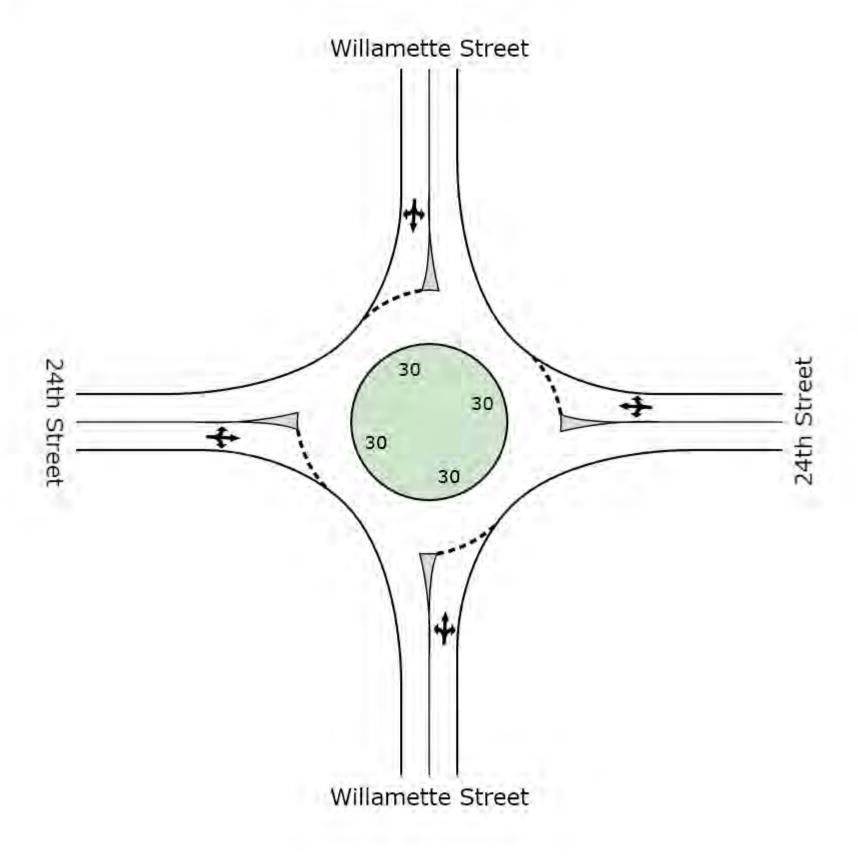
503-391-8773 Phone:

Count Date (am) Count Date (pm)	10/3/2012 10/3/2012		Eugene	м.р. о		Major Street has two approaching lanes Minor Street has one approaching lane 100% Warrants	
			K		,	100 % Reduction in minor st right tur 0.00 % growth per year for 0	rn volume years
Condition B:	intering Volume (No	on-normal busine	ss day):	N/A			
The hourly warrants are Control Devices for Str				ffic			Total Value
-		l warrants used. tandard warrants	used due to 85 perd	centile speed in	n excess of 40 MPH	H or isolated community with population less than 10,000.	

Appendix

- Roundabout Operations Analysis, 2018





Roundabout

Lane Use and	d Performa															
	L veh/h	Demand T veh/h	d Flows R veh/h	Total veh/h	HV %	Cap.	Deg. Satn v/c	Lane Util. %	Average Delay sec	Level of Service	95% Back of Vehicles veh	of Queue Distance ft	Lane Length ft	SL Type	Cap. Adj. %	Prob. Block. %
South: Willame		7011/11	V 311/11	VOI.,,11		V 31 I/11	•,,,	,,,			VO11	·			70	,,
Lane 1	5	457	212	674	1.0	1045	0.645	100	8.1	LOS A	7.9	198.0	1600	_	0.0	0.0
Approach	5	457	212	674	1.0		0.645		8.1	LOS A	7.9	198.0				
East: 24th Stree	et															
Lane 1	255	87	43	386	0.2	593	0.650	100	18.9	LOS B	7.4	185.7	1600	_	0.0	0.0
Approach	255	87	43	386	0.2		0.650		18.9	LOS B	7.4	185.7				
North: Willamet	tte Street															
Lane 1	54	761	16	832	0.9	716	1.162	100	97.8	LOS F	60.9	1533.2	1600	_	0.0	3.8
Approach	54	761	16	832	0.9		1.162		97.8	LOS F	60.9	1533.2				
West: 24th Stre	eet															
Lane 1	16	49	16	82	0.0	299	0.273	100	18.6	LOS B	2.0	50.5	1600	_	0.0	0.0
Approach	16	49	16	82	0.0		0.273		18.6	LOS B	2.0	50.5				
Intersection				1973	0.8		1.162		48.5	LOS D	60.9	1533.2				

Level of Service (Aver. Int. Delay): LOS D. Based on average delay for all lanes. LOS Method: Delay (HCM).

Level of Service (Worst Lane): LOS F. LOS Method for individual lanes: Delay (HCM).

Approach LOS values are based on the worst delay for any lane. Roundabout LOS Method: Same as Signalised Intersections.

Roundabout Capacity Model: SIDRA Standard.

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Roundabout

Lane Use	and Pe	rforma	ance													
	. [Deman	d Flows		HV	Cap.	Deg.	Lane	Average	Level of	95% Back		Lane	SL	Cap. F	
	L veh/h	l veh/h	R veh/h	Total veh/h	пv %	veh/h	Satn v/c	Util. %	Delay sec	Service	Vehicles veh	Distance ft	Length ft	Type	Adj. E %	Block. %
South: Willa			V () () ()	VOII/II	/0	VOII/II	V/ O	70	300		VO11		11		70	70
Lane 1	5	451	209	665	1.0	1122	0.593	100	7.7	LOS A	5.0	126.5	1600	_	0.0	0.0
Approach	5	451	209	665	1.0		0.593		7.7	LOS A	5.0	126.5				
East: 24th S	treet															
Lane 1	242	88	44	374	0.2	699	0.535	100	15.0	LOS B	4.6	114.0	1600	_	0.0	0.0
Approach	242	88	44	374	0.2		0.535		15.0	LOS B	4.6	114.0				
North: Willar	mette St	reet														
Lane 1	60	758	16	835	0.9	864	0.966	100	29.2	LOS C	27.4	689.6	1600	_	0.0	0.0
Approach	60	758	16	835	0.9		0.966		29.2	LOS C	27.4	689.6				
West: 24th S	Street															
Lane 1	16	49	16	82	0.0	388	0.212	100	22.0	LOS C	1.3	32.1	1600	_	0.0	0.0
Approach	16	49	16	82	0.0		0.212		22.0	LOS C	1.3	32.1				
Intersection				1956	0.8		0.966		18.9	LOS B	27.4	689.6				

Level of Service (Aver. Int. Delay): LOS B. Based on average delay for all lanes. LOS Method: Delay (HCM).

Level of Service (Worst Lane): LOS C. LOS Method for individual lanes: Delay (HCM).

Approach LOS values are based on the worst delay for any lane.

Roundabout LOS Method: Same as Signalised Intersections.

Roundabout Capacity Model: US NCHRP 572.

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Roundabout

Lane Use a	Lane Use and Performance															
	C	eman	d Flows		1.15.7		Deg.	Lane	Average	Level of	95% Back	of Queue	Lane	SL	Сар.	
	L	Τ.	R	Total	HV	Cap.	Satn	Util.	Delay	Service	Vehicles	Distance	Length	Type		Block.
Courthy Willow	veh/h		veh/h	veh/h	%	veh/h	v/c	%	sec		veh	ft	ft		%	%
South: Willamette Street																
Lane 1	16	602	16	634	0.9	1313	0.483	100	6.6	LOS A	3.5	88.4	1600	_	0.0	0.0
Approach	16	602	16	634	0.9		0.483		6.6	LOSA	3.5	88.4				
East: 25th Street																
Lane 1	27	5	16	48	0.0	585	0.083	100	13.9	LOS B	0.5	11.5	1600	_	0.0	0.0
Approach	27	5	16	48	0.0		0.083		13.9	LOS B	0.5	11.5				
North: Willar	nette St	reet														
Lane 1	5	962	27	995	1.0	1344	0.740	100	6.8	LOS A	8.8	221.1	1600	_	0.0	0.0
Approach	5	962	27	995	1.0		0.740		6.8	LOSA	8.8	221.1				
West: 25th S	Street															
Lane 1	32	5	11	48	2.0	406	0.119	100	21.2	LOS C	0.7	17.4	1600	_	0.0	0.0
Approach	32	5	11	48	2.0		0.119		21.2	LOS C	0.7	17.4				
Intersection				1726	1.0		0.740		7.3	LOSA	8.8	221.1				

Level of Service (Aver. Int. Delay): LOS A. Based on average delay for all lanes. LOS Method: Delay (HCM).

Level of Service (Worst Lane): LOS C. LOS Method for individual lanes: Delay (HCM).

Approach LOS values are based on the worst delay for any lane.

Roundabout LOS Method: Same as Signalised Intersections.

Roundabout Capacity Model: US NCHRP 572.

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Roundabout

Lane Use	Lane Use and Performance															
	L veh/h	Т	d Flows R veh/h	Total veh/h	HV %	Cap.	Deg. Satn v/c	Lane Util. %	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance ft	Lane Length ft	SL Type	Cap. Adj. %	Prob. Block. %
South: Willamette Street																
Lane 1	31	557	41	629	2.0	1123	0.560	100	7.5	LOS A	4.4	112.6	1600	_	0.0	0.0
Approach	31	557	41	629	2.0		0.560		7.5	LOS A	4.4	112.6				
East: 27th S	East: 27th Street															
Lane 1	129	103	36	268	1.6	592	0.453	100	15.8	LOS B	3.4	85.2	1600	_	0.0	0.0
Approach	129	103	36	268	1.6		0.453		15.8	LOS B	3.4	85.2				
North: Willan	mette St	treet														
Lane 1	31	835	57	923	1.1	947	0.974	100	27.2	LOS C	30.1	757.7	1600	_	0.0	0.0
Approach	31	835	57	923	1.1		0.974		27.2	LOS C	30.1	757.7				
West: 27th S	Street															
Lane 1	31	52	46	129	1.2	408	0.316	100	21.3	LOS C	2.0	50.0	1600	_	0.0	0.0
Approach	31	52	46	129	1.2		0.316		21.3	LOS C	2.0	50.0				
Intersection				1948	1.4		0.974		18.9	LOS B	30.1	757.7				

Level of Service (Aver. Int. Delay): LOS B. Based on average delay for all lanes. LOS Method: Delay (HCM).

Level of Service (Worst Lane): LOS C. LOS Method for individual lanes: Delay (HCM).

Approach LOS values are based on the worst delay for any lane.

Roundabout LOS Method: Same as Signalised Intersections.

Roundabout Capacity Model: US NCHRP 572.

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Roundabout

Lane Use	Lane Use and Performance															
	L	Deman T	d Flows R	Total	HV	Сар.	Deg. Satn	Lane Util.	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Lane Length	SL Type	Cap. Adj. l	Prob. Block.
	veh/h		veh/h	veh/h	%	veh/h	v/c	%	sec		veh	ft	ft		%	%
South: Willamette Street																
Lane 1	36	479	0	515	2.1	1077	0.479	100	7.5	LOS A	3.4	85.6	1600	_	0.0	0.0
Approach	36	479	0	515	2.1		0.479		7.5	LOSA	3.4	85.6				
North: Willa	mette S	treet														
Lane 1	0	758	175	933	0.2	1390	0.671	100	6.7	LOS A	7.0	174.5	1600	_	0.0	0.0
Approach	0	758	175	933	0.2		0.671		6.7	LOSA	7.0	174.5				
West: Plaza	Drivew	ay														
Lane 1	129	0	155	284	1.0	524	0.541	100	20.7	LOS C	4.7	118.5	1600	_	0.0	0.0
Approach	129	0	155	284	1.0		0.541		20.7	LOS C	4.7	118.5				
Intersection				1732	0.9		0.671		9.2	LOSA	7.0	174.5				

Level of Service (Aver. Int. Delay): LOS A. Based on average delay for all lanes. LOS Method: Delay (HCM).

Level of Service (Worst Lane): LOS C. LOS Method for individual lanes: Delay (HCM).

Approach LOS values are based on the worst delay for any lane.

Roundabout LOS Method: Same as Signalised Intersections.

Roundabout Capacity Model: US NCHRP 572.

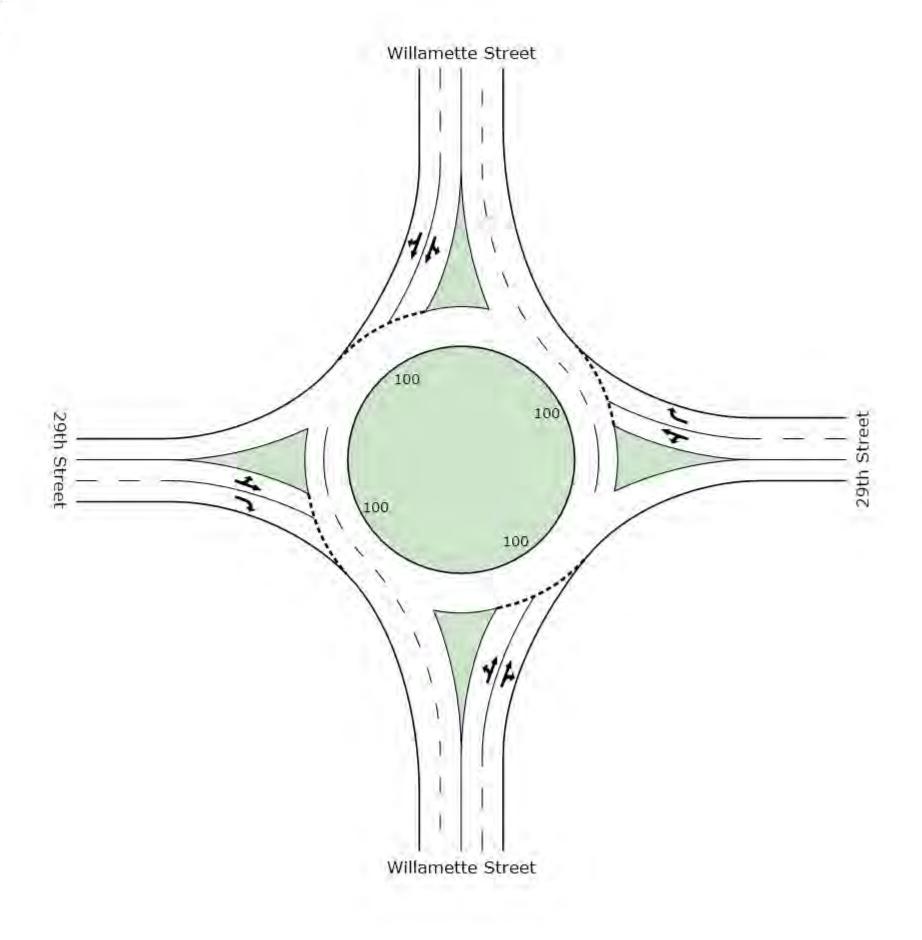
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Roundabout with 2 entering lanes on all legs, 2 exiting lanes on north/south legs, and 1 exiting leg on east/west legs Roundabout

Lane Use	Lane Use and Performance															
	ا	Deman	d Flows		1.157	0	Deg.	Lane	Average	Level of	95% Back		Lane	SL	Cap. I	
	L	T	R	Total	HV	Cap.	Satn	Util.	Delay	Service	Vehicles	Distance	Length	Type		Block.
South: Willa	veh/h		veh/h	veh/h	%	veh/h	v/c	%	sec		veh	ft	ft		%	%
Lane 1	289	45	0	335	1.3	612	0.547	100	17.9	LOS B	4.7	118.6	1600	_	0.0	0.0
Lane 2	203	281	47	328	3.3	600	0.547	100	12.2	LOS B	4.6	117.4	1600	_	0.0	0.0
	289	326				000		100		LOS B		118.6	1000		0.0	0.0
Approach	209	320	47	663	2.3		0.547		15.1	LUS B	4.7	110.0				
East: 29th S	East: 29th Street															
Lane 1	126	389	0	516	1.7	654	0.789	100	15.3	LOS B	7.6	192.4	1600	_	0.0	0.0
Lane 2	0	0	74	74	2.0	628	0.117	100	8.3	LOS A	0.5	12.3	1600	_	0.0	0.0
Approach	126	389	74	589	1.8		0.789		14.4	LOS B	7.6	192.4				
North: Willa	mette S	treet														
Lane 1	163	276	0	439	1.0	493	0.889	100	41.2	LOS D	15.7	395.7	1600	_	0.0	0.0
Lane 2	0	329	111	440	0.7	495	0.889	100	39.0	LOS D	15.8	396.2	1600	_	0.0	0.0
Approach	163	605	111	879	0.9		0.889		40.1	LOS D	15.8	396.2				
West: 29th	Street															
Lane 1	126	295	0	421	1.4	590	0.713	100	15.6	LOS B	5.8	146.6	1600	_	0.0	0.0
Lane 2	0	0	347	347	1.0	566	0.613	100	13.6	LOS B	4.4	110.2	1600	_	0.0	0.0
Approach	126	295	347	768	1.2		0.713		14.7	LOS B	5.8	146.6				
Intersection				2900	1.5		0.889		22.4	LOS C	15.8	396.2				

Level of Service (Aver. Int. Delay): LOS C. Based on average delay for all lanes. LOS Method: Delay (HCM).

Approach LOS values are based on the worst delay for any lane.

Roundabout LOS Method: Same as Signalised Intersections.

Roundabout Capacity Model: US NCHRP 572.

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Roundabout

Lane Use a	Lane Use and Performance															
	D	emano	d Flows				Deg.	Lane	Average	Level of	95% Back	of Queue	Lane	SL	Cap.	Prob.
	L	Т	R	Total	HV	Cap.	Satn	Util.	Delay	Service	Vehicles	Distance	Length	Type		Block.
	veh/h \		veh/h	veh/h	%	veh/h	v/c	%	sec		veh	ft	ft		%	%
South East: Donald Street																
Lane 1	5	0	263	268	1.9	725	0.370	100	11.4	LOS B	2.3	59.1	1600	_	0.0	0.0
Approach	5	0	263	268	1.9		0.370		11.4	LOS B	2.3	59.1				
East: 32nd Avenue																
Lane 1	16	0	21	37	3.4	567	0.065	100	14.1	LOS B	0.3	8.9	1600	_	0.0	0.0
Approach	16	0	21	37	3.4		0.065		14.1	LOS B	0.3	8.9				
North: Willar	nette Str	eet														
Lane 1	368	0	647	1016	1.0	1458	0.696	100	8.9	LOS A	7.5	190.1	1600	_	0.0	0.0
Approach	368	0	647	1016	1.0		0.696		8.9	LOSA	7.5	190.1				
South West:	Willame	tte Str	eet													
Lane 1	384	0	11	395	1.0	767	0.514	100	14.9	LOS B	4.1	102.7	1600	_	0.0	0.0
Approach	384	0	11	395	1.0		0.514		14.9	LOS B	4.1	102.7				
Intersection				1716	1.2		0.696		10.8	LOS B	7.5	190.1				

Level of Service (Aver. Int. Delay): LOS B. Based on average delay for all lanes. LOS Method: Delay (HCM).

Level of Service (Worst Lane): LOS B. LOS Method for individual lanes: Delay (HCM).

Approach LOS values are based on the worst delay for any lane.

Roundabout LOS Method: Same as Signalised Intersections.

Roundabout Capacity Model: US NCHRP 572.

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